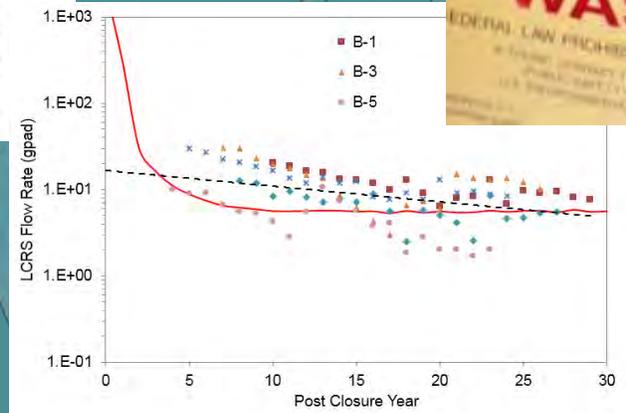
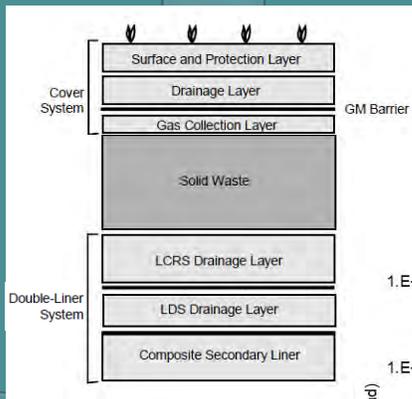


Post-Closure Performance of Liner Systems at RCRA Subtitle C Landfills

Final Report



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Post-Closure Performance of Liner Systems at RCRA Subtitle C Landfills

Final Report

Materials Management Branch
Land and Materials Management Division
National Risk Management Research Laboratory
Office of Research and Development
Cincinnati, OH

Foreword

The US Environmental Protection Agency (US EPA) is charged by Congress with protecting **the Nation's land, air, and water resources. Under the mandate of national** environmental laws, the Agency strives to formulate and implement actions leading to a compatible balance between human activities and the ability of natural systems to support and nurture life. To meet this mandate, US EPA's research program is providing data and technical support for solving environmental problems today and building the scientific knowledge base necessary to manage our ecological resources wisely, understand how pollutants affect our health, and prevent or reduce environmental risks in the future.

The National Risk Management Research Laboratory (NRMRL) is the Agency's center for investigating technological and management approaches for preventing and reducing risks from pollution that threaten human health and the environment. The focus of the Laboratory's research program is on methods and their cost-effectiveness for preventing and controlling pollution of air, land, water, and subsurface resources; protecting water quality in public water systems; remediating contaminated sites, sediments, and ground water; preventing and controlling indoor air pollution; and restoring ecosystems. NRMRL collaborates with public and private sector partners to foster technologies that reduce the cost of compliance and anticipate emerging problems. NRMRL's research provides solutions to environmental problems by developing and promoting technologies that protect and improve the environment; advancing scientific and engineering information to support regulatory and policy decisions; and providing the technical support and information transfer to ensure implementation of environmental regulations and strategies at the national, state, and community levels.

This publication has been produced as part of the Laboratory's strategic long-term research plan. It is published and made available by US EPA's Office of Research and Development to assist the user community and to link researchers with their clients.

**Cynthia Sonich-Mullin, Director
National Risk Management Research Laboratory**

Executive Summary

Generation, transportation, treatment, storage, and disposal of hazardous waste are regulated under the Resource Conservation and Recovery Act of 1976 (RCRA), an act of Congress that gives the U.S. Environmental Protection Agency (EPA) authority to control **hazardous waste from the “cradle-to-grave.” Specifically, Subtitle C of RCRA pertains to** management of hazardous waste.¹ This document is specifically focused on the long-term performance of **landfill containment facilities (the “grave” in the above analogy) at RCRA Subtitle C facilities.** Landfills are used for the environmentally protective disposal of hazardous waste, regulation of which is codified at **40 CFR Part 264, “Standards for Owners and Operators of Hazardous Waste Treatment, Storage, and Disposal Facilities,”** as published in various editions of the Federal Register since 1980. Sections of the regulation of relevance to this document are provided under Subpart N – Landfills (40 CFR §264.300 through .317) and Subpart G – Closure and Post-Closure (40 CFR §264.110 through .120). Post-closure care (PCC) requirements for Subtitle C landfills involve monitoring and maintaining the waste containment systems for a presumptive period of 30 years (per 40 CFR §264.117), or an extended or reduced period based on the demonstration that such adjustment is necessary or sufficient, respectively, for the protection of human health and the environment. Hazardous waste landfills have been permitted under RCRA Subtitle C since 1984, over 30 years ago, thus an increasing number of facilities around the country are approaching the end of their presumptive 30-year PCC period. Stakeholders requested the EPA to provide guidance on how and when it may be appropriate to make such certifications or other decisions regarding the ongoing status of their site. For its part, in **2016 EPA issued “Guidelines for Evaluating the Post-Closure Care Period for Hazardous Waste Disposal Facilities under Subtitle C of RCRA.”**

The aim of the study is to facilitate the discussion and decision-making processes by illustrating what data are needed, highlighting categories of useful data that are typically **lacking, and recommending techniques and tools to complement the EPA’s “Guidelines for Evaluating the Post-Closure Care Period for Hazardous Waste Disposal Facilities under Subtitle C of RCRA”.** The study investigates the field performance of engineered double-liner systems based on data from 9 Subtitle C landfills sites that have completed several years of PCC. It is noted that the document is not intended to address policy issues (such as how

¹ Subtitle D of RCRA sets forth a framework for the management of non-hazardous solid wastes; however, management of non-hazardous solid waste is only of passing interest in this document.

landfills may be managed, controlled, or regulated after PCC has ended) or to provide generic answers to defining conditions for ending PCC.

Furthermore, also provides a follow-up for a broader EPA study published in 2002 entitled **"Assessment and Recommendations for Improving the Performance of Waste Containment Systems."** The 2002 study reported on the performance of active and closed hazardous and non-hazardous waste landfill units around the country using data collected in the 1980s and 1990s. In updating that study, EPA is specifically interested in supplementing the previous dataset with a further 10–15 years of performance data from closed Subtitle C landfill units.

Overall, the nine landfills yielded 45 individual double-lined closed units ranging in size from 1.4 to 11 acres, although most units were less than 5 acres in area. The oldest units in the study have been closed for over 29 years, while the newest are only 6 years into a PCC program. The thickness of waste in place above the liner ranged from 40 feet to 110 feet (average 70 to 80 feet). Amongst the 45 case study units, 11 different liner system designs and a further 11 different cover system designs are represented. These are combined into 13 unique containment system design configurations featuring commonality through the entire thickness of the unit from the top of the cover to the bottom of the liner. The discussion is interested in addressing the five broad research questions presented next.

1. *How much leachate is generated in closed Subtitle C landfills and what are the effects of site location (climatic region), cover system design, or waste type on leachate generation rates?*

In general, field data showed a decline in leachate flow from the LCRS and LDS. In all cases, placement of cover led to a reduction in the LCRS flow rate, including where only 12 inches of intermediate cover soil had been placed. Rainfall has an effect on leachate generation, with higher LCRS flows recorded at the four wet sites and very low or negligible flows recorded at three dry sites. The incidence of precipitation as rainfall versus snowfall does not appear to affect leachate generation at the wet sites.

An increasing trend in leachate generation was observed at some sites for a duration of time and was attributable to known operation and maintenance (O&M) issues affecting cover system performance. Erosion damage to cover systems was identified as a key issue affecting landfill performance in the post-closure period, with higher costs and effort associated with repairs needed during initial years of PCC before cover vegetation is fully established and the cover stabilized. Breaches in the cover system, particularly in the early years of PCC, could result in relatively long-term setbacks in terms of returning LCRS flow rates to expected levels once the cover is repaired. Routine cover inspection is essential for

identifying issues related to erosion damage, water ponding on the cover system, or other issues, as this facilitates timely maintenance and repair to reduce leachate flow volumes.

Data from this study suggest the rate at which LCRS flow rate declines post-closure may be three to five times slower than reported in 2002 study or approximately an order of magnitude decrease in flow every 15–20 years. However, more field studies, preferably under a random selection procedure, are needed to validate this finding before recommendations for adjusting current industry projections and accruals for leachate management can be made. Furthermore, the rate of decrease in the leachate generation correlates with the maximum leachate generation at closure. In another word, cells that were wetter and had higher leachate flow at the time of closure continued to have relatively high flow well into their PCC period. This emphasizes the importance of good storm water control during the period of landfill operation and competent cover design and construction performed under strict COA procedures so as to minimize peak leachate generation immediately after closure.

2. *What conclusions can be drawn regarding the hydraulic efficiencies of double-liner systems (i.e., leakage rates through primary liners) at Subtitle C landfills based on available leachate collection and removal system (LCRS) and leakage detection system (LDS) data?*

The “**apparent**” hydraulic efficiency (E_A) of the primary liner can be calculated as the flow in the LDS relative to the flow in the LCRS. If the only source of flow into the LDS sump is primary liner leakage, then E_A provides the true measure of the effectiveness of a particular liner in limiting or preventing advective transport across the liner. Overall, calculated E_A values from this study generally fall significantly short of the one suggested by the 2002 study (99%). Furthermore, as would be expected, the apparent liner efficiencies are significantly higher at dry sites than at wet sites.

A significant number of calculated E_A values in this study fall below zero (i.e., have negative E_A values). Negative values are interesting in that they indicate that flow in the LDS exceeded that in the LCRS. This may be the result of major defects in the primary liner system (which should have been identified during the construction of the liner system, or that the volume of liquids in the LDS cannot be attributed to liner leakage alone and may be caused by groundwater intrusion. It is possible that by distinguishing liquids with similar chemical signatures, leachate chemistry data can be used to quantify the portion of liquids comprising total LDS flow that could be attributable to primary liner leakage as opposed to other sources, thereby correcting the extent to which the LCRS and LDS are hydraulically connected. Using chlorides for chemical signature, the liner efficiencies increased drastically.

It is noted that this correction may grossly overestimate the collection efficiency as it does not take into account chemical attenuation of the liner system.

An interesting observation is that the thickness of material specified does not appear to affect the hydraulic performance of the barrier GM in either the cover or liner system for the units included in this study when compared across a common material type, although high-density polyethylene (HDPE) appeared to outperform polyvinyl chloride (PVC). Four sites featured a GM-only barrier in the primary liner, while all other designs feature a composite GM/CCL barrier. However, the primary liner design also does not appear to affect LDS flows; therefore it appears that construction of a composite primary liner may not be necessary as long as a composite secondary liner is constructed if one was only considering the limited number of units included in this study. More data are needed to evaluate this observation.

3. *How do predictions of leachate generation using the EPA's Hydrologic Evaluation of Landfill Performance (HELP) Model compare to observed generation rates at these sites?*

Based on the landfill cover designs and leachate flow rates from the sites evaluated in this study, the HELP Model appeared to be better suited to predicting long-term LCRS flow at wet rather than dry sites, which is consistent with published findings. The model predicts zero or near-zero LCRS flows at dry sites, whereas higher LCRS flow was observed at all four dry case study landfills. Based on our findings, it appears that it may be unreasonable to achieve a leachate flow rate of zero or near zero for a landfill site within the 30 years PCC period. Therefore, it is prudent to demonstrate that the absence of care for the leachate collection system would not pose a threat to water quality and human health and the environment. Such demonstrations may be made if enough data is available on leachate flow and chemical concentrations having reached quasi-steady state, predictable, and non-impacting conditions, albeit at a non-zero flow rate.

4. *What is the leachate chemistry at these sites, and does it exhibit asymptotic behavioral trends over the long term?*

Thirty chemical parameters were selected to represent leachate constituents of interest, based on those investigated in the 2002 study. These included water quality indicator parameters, macro indicators of dissolved organic matter, major inorganic cations and anions, trace metals, and trace volatile organic compounds (VOCs) frequently observed to be present in landfill leachate. Concentration trends in these data were examined to assess whether the asymptotic behavior was evident or could be predicted. Where available, leachate concentrations were compared to published data as well as EPA water quality standards. However, rather than directly comparing source leachate concentrations to a limit value (which defeats the performance-based intention of the regulation), a universal

dilution/attenuation factor (DAF) of 20 was applied to represent a concentration decrease that would be expected prior to detection at a point of compliance (POC) monitoring well. **This DAF value is equal to the default specified in the EPA's Soil Screening Guidance (EPA, 1996);** however, use of a DAF of 20 for illustrative purposes in this study should not be misconstrued as a suggestion that this value has universal applicability nor that EPA has endorsed use of this value in lieu of site-specific analysis. In any evaluation of leachate chemistry and threat potential, a site-specific DAF should be calculated.

Significant variability was evident in the data for many constituents, particularly in the LCRS where differences between maximum and minimum observed values often span six or more orders of magnitude for cations/anions, trace metals, and VOCs. The general water quality characteristics of liquids from the LCRS and LDS drainage layers are also significantly different, again by multiple orders of magnitude in many cases. Although, contaminant transport through a liner can occur due to advective or diffusive flux, or both, given the order of magnitude differences in concentration of the chemical constituents in LCRS and LDS, advective flux may be considered as a primary driving mechanism for this variability.

The long-term outlook for leachate management based on observations of behavioral trends amongst selected leachate data from this study is mixed. Water quality indicators and major cations/anions suggest that the materials contained in Subtitle C landfills may not degrade under landfill conditions, or only degrade very slowly, such that observations based on similar behavioral trends from non-hazardous Subtitle D landfills cannot be extrapolated to characterize the expected performance of Subtitle C landfills.

5. How could current monitoring, reporting, and recordkeeping requirements be improved to better ensure that the data necessary for performance demonstrations are collected?

The unavailability of some critical site information and monitoring data limited the extent to which evaluations of leachate flow and chemistry could be completed or even performed in this study. Obvious data gaps and their effects are identified throughout this document. Fuller LCRS and LDS datasets, for example, would have expanded the level of detail to which cover and liner system performance could be evaluated, while a longer data record would have enabled a clearer picture of long-term stable leachate generation to be gathered. The focus and goal of this discussion is to reiterate some key data limitations and discuss their effect on limiting the study, in so doing providing some guidance to site operators and regulators as to what data that are not routinely collected would be valuable in demonstrating that one or more components of PCC at Subtitle C landfills could be modified over the long term. This is intended to provide motivation, rather than an obligation, for additional data collection.

Overall, significant variability existed between the case study units, which is beneficial to a study of this nature. In terms of variability in construction details, 11 different liner system designs and a further 11 different cover system designs were featured amongst the study units featured, combining to provide 13 unique containment system design configurations. Major variables in cover system design were represented. However, primary liner designs essentially comprised only two variables: GM/CCL and GM only. No case study units were constructed having a GM/GCL composite primary liner, although one site has a GCL secondary liner. As such, the efficacy of a GM/GCL primary liner design cannot be evaluated, an important limitation given the widespread use of GCLs in liner systems. In terms of facility operations, seven of the nine landfills are commercial facilities accepting hazardous waste from a wide range of generators. As such, an original research question from this study (does waste type affect leachate concentrations?) could not be addressed, as the commercial facilities accepted waste from multiple sources thereby making it difficult to compare waste chemistry for these sites, while there were insufficient data from non-commercial facilities against which to gauge variability between commercial and non-commercial sites. Further, waste manifests were not available, which meant that although some findings appeared to support the hypothesis that facility/waste type would affect leaching behavior and leachate characteristics, this could not be confirmed.

With regard to leachate chemistry data, many targeted leachate constituents are poorly represented in the LCRS dataset, while LDS chemistry is not monitored at all at many sites. Leachate chemistry data are most commonly collected semi-annually or annually, thereby limiting the overall size of the dataset available for analysis. An issue of importance identified in the process of collecting leachate chemistry data for this study is that site operators are reportedly only required to keep records for 3 years; as such, many older data are no longer available. If this is broadly representative of Subtitle C facilities, it represents an important limitation on assessing the long-term performance of containment systems and potential modifications to existing PCC programs. Given the focus on containment and leachate minimization at Subtitle C landfills, data availability may be reflective of the low level of concern operators have on managing leachate treatment and disposal costs (low leachate flows attract little interest because disposal costs are modest for small volumes). Many of the data used to support the performance demonstrations made in this study rely on non-compliance data collected only to meet influent limits imposed by a receiving facility for leachate treatment and are not routinely submitted to the state. This has implications in terms of being able to independently assess site performance, which would be important if an operator was unable to continue providing care.

Notice

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Abbreviations

ASTSWMO	Association of State and Territorial Solid Waste Management Officials
BOD	biochemical oxygen demand
BTEX	benzene, toluene, ethylbenzene and xylene
CAMU	corrective action management unit
CCL	compacted clay liner
CF	correction factor
CERCLA	Comprehensive Environmental Response, Compensation, and Liability Act
CFR	Code of Federal Regulations
COD	chemical oxygen demand
COA	construction quality assurance
DAF	dilution/attenuation factor
DOE	U.S. Department of Energy
DOM	dissolved organic matter
E_A	apparent liner efficiency
E_C	corrected liner efficiency
Eh	oxidation-reduction potential
E_M	modeled liner efficiency
EPA	U.S. Environmental Protection Agency
E_T	true liner efficiency
ET	evapotranspiration; evapotranspirative
FA	financial assurance
GCL	geosynthetic clay liner
GC	geocomposite
GM	geomembrane
GN	geonet
gpad	gallons per acre per day
GT	geotextile
HDPE	High-density polyethylene
HELP	Hydrologic Evaluation of Landfill [Model]
HHE	human health and environment
HW	hazardous waste
ITRC	Interstate Technology and Regulatory Council
LCRS	leachate collection and removal system
LDS	the leakage detection system
LFG	landfill gas
LLDPE	linear low-density polyethylene
LLRW	low-level radioactive waste
lphd	liter per hectare per day
MCL	maximum contaminant level
MSW	municipal solid waste
MSWI	municipal solid waste incineration

ND	non-detect
NRML	National Risk Management Research Laboratory (EPA)
O&M	operation and maintenance
OIG	Office of Inspector General (EPA)
ORCR	Office of Resource Conservation and Recovery (EPA)
ORD	Office of Research and Development (EPA)
OSDC	on-site disposal cell
OSDF	on-site disposal facility
PCC	post-closure care
POC	the point of compliance
PVC	polyvinyl chloride
QA	quality assurance
QAPP	quality assurance project plan
QC	quality control
RCRA	Resource Conservation and Recovery Act
SCS	Soil Conservation Service (U.S. Dept. of Agriculture)
SMCL	secondary maximum contaminant level
TDS	total dissolved solids
TOC	total organic carbon
TSDF	treatment, storage, and disposal facility
VOC	volatile organic compound
WWTP	wastewater treatment plant

1. INTRODUCTION

1.1 Overview and Terms of Reference

A research field of significant interest to the U.S. Environmental Protection Agency (EPA) is the long-term behavior of hazardous waste disposal facilities regulated under Subtitle C of the Resource Conservation and Recovery Act of 1976 (RCRA). The regulation is codified under 40 CFR Part 264, as published in various editions of the Federal Register since 1980 (EPA, 2014a). Sections of the regulation of relevance to this research field are provided under Subpart N – Landfills (40 CFR §264.300 through .317) and Subpart G – Closure and Post-Closure (40 CFR §264.110 through .120). Post-closure care (PCC) requirements for Subtitle C landfills involve monitoring and maintaining the waste containment systems for a default period of 30 years (per 40 CFR §264.117), or an extended or reduced period based on demonstration that such adjustment is necessary or sufficient, respectively, for protection of human health and the environment (HHE). Adjusting the PCC period, or certifying that PCC is completed such that HHE remains protected in the absence of PCC, is at the discretion of the permitting authority (EPA Regional Administrator or Director of an authorized state program).

Hazardous waste landfills have been permitted under RCRA Subtitle C since 1984, over 30 years ago, which means that an increasing number of facilities around the country may soon have completed 30 years of PCC. Both the regulated and regulatory communities are faced with addressing the situation in which a decision is needed regarding the ongoing status of the site. A 2015 study by the EPA Office of Inspector General (OIG) estimated that **over 1,500 hazardous waste disposal units across the nation had been “closed with waste in place” as of October 2014, although not all are under a permitted PCC program (EPA, 2015)**. The number of units for which a decision regarding the end of PCC will be needed was estimated at between 15 and 45 annually through 2030, with over half of these decisions falling in the next 10 years. The need for technical and procedural guidance has been identified by the regulated and regulatory communities alike. Notably, the Association of State and Territorial Solid Waste Management Officials (ASTSWMO) issued a position paper calling for guidance from EPA on evaluating PCC criteria and modifying the PCC period (ASTSWMO, 2013). In response to these and other calls for increased clarity on the subject, **EPA recently issued “Guidelines for Evaluating the Post-Closure Care Period for Hazardous Waste Disposal Facilities under Subtitle C of RCRA” (EPA, 2016)**. While the guidance does not provide specific details on how to conduct an evaluation, it does provide a framework that recommends the use of monitoring, modeling, and/or statistical analysis to determine if

landfill contaminants (primarily leachate) would pose a threat to HHE at compliance or exposure points outside of the waste mass. This study provides updated information on the post-closure performance of Subtitle C landfill liner systems and, therefore, supports EPA's guidance on using performance-based demonstrations as the basis for extending, reducing, or ending PCC.

1.2 Purpose, Scope, and Limitations of the Study

The primary objective of this study is to investigate the field performance of engineered double-liner systems based on data from Subtitle C landfills that have completed several years of PCC and to quantify actual leachate generation rates, liner performance (i.e., **leakage**), and **leachate chemistry during PCC in relation to current industry "norms" and expectations**. It is anticipated that this study will help the EPA assess and update expectations for field performance of Subtitle C landfills in the PCC period, specifically as reported and discussed in Chapter 5 and Appendix E of the previous study prepared for EPA titled "**Assessment and Recommendations for Improving the Performance of Waste Containment Systems**" (EPA, 2002). This should be beneficial to both the regulatory and regulated communities in terms of making decisions regarding the long-term data collection and performance demonstrations necessary to evaluate, and ultimately adjust, PCC at Subtitle C landfills. As such, the intended audience of this document is state and EPA regional regulators, private industry, commercial facility owners/operators, landfill design engineers, and other hazardous waste professionals.

EPA (2002) reported on the performance of active and closed hazardous and non-hazardous waste landfill units around the country using data collected in the 1980s and 1990s. In updating that study with respect to hazardous waste landfills, EPA is specifically interested in supplementing the previous dataset with a further 10–15 years of performance data from closed Subtitle C landfill units in the interests of addressing the following four broad research questions:

1. What conclusions can be drawn regarding the hydraulic efficiencies of double-liner systems (i.e., leakage rates through primary liners) at Subtitle C landfills based on available leachate collection and removal system (LCRS) and leakage detection system (LDS) data?
2. How much leachate is generated in closed Subtitle C landfills and what are the effects of site location (climatic region), cover system design, or waste type on leachate generation rates?
3. **How do predictions of leachate generation using the EPA's Hydrologic Evaluation of Landfill Performance (HELP) Model compare to observed generation rates at these sites?**

4. What is the leachate chemistry at these sites, and do certain constituents exhibit asymptotic or other notable behavioral trends over the long-term?

This study aims to provide answers to these questions. However, it is important to clarify that the focus is on understanding the performance of liner systems rather than cover systems. This is because Subtitle C landfills routinely install double liners such that the performance (leakage) of the primary liner can be directly measured, while the performance of the cover system can only be indirectly estimated based on overall leachate generation. Similarly, it should be clarified that it is not the intent of this study to directly address PCC policy issues (such as how landfills may be managed, controlled, or regulated after PCC has ended) or to provide generic answers to defining conditions for ending PCC. These should be agreed via a site-specific discussion between the regulator and owner/operator, based on the application of the guidance issued by EPA (2016). This study aims to facilitate the discussion and decision-making processes by illustrating what data are needed, highlighting categories of useful data that are typically lacking, and recommending techniques and tools for evaluation of data.

Other important questions extend beyond the main study goals but can help further the understanding of long-term landfill performance and development of guidance to both evaluate and improve performance. These include:

1. How does leachate chemistry at Subtitle C landfills compare with water quality standards?
2. What models are routinely used to analyze leachate data and predict leachate generation and potential transport of hazardous waste constituents?
3. What are the expected or observed effects of extreme weather or seismic induced events (e.g., flood, excessive rainfall, or tsunami) on the performance of waste containment systems, particularly the cover?
4. Can performance data from studies such as this be used to assign risk-based evaluation criteria and procedures for demonstrating long-term protection of HHE?
5. How could current monitoring, reporting, and recordkeeping requirements be improved to better ensure that the data necessary for performance demonstrations are collected?

Although these additional questions were considered during this research study, they serve primarily to establish longer-term research goals for EPA.

Important limitations pertain to this study, mainly with regard to the relatively small number of contributory datasets available to evaluate the long-term performance of composite liner systems comprising both geosynthetic and clay barrier layers (liner system components are described in detail in Section 1.4.2). For inclusion in the study, therefore, a Subtitle C landfill disposal unit (i.e., discrete set of one or more cells, phases, modules, or areas) needed to be final capped (or have been inactive at final grade for an extended

period under intermediate cover) and have a double-liner system with separate unit-specific measurement of liquid flow rate in the LCRS and LDS. The total number of Subtitle C landfills in the country with at least one doubled-lined closed unit is limited to a few dozen sites, such that the pool of candidate sites was small. Furthermore, to promote the free exchange of site performance data and operational experiences, participation in the study was fully voluntary on the part of site operators. As a result, the selected case studies cannot be assumed to be representative of a random subset of Subtitle C facilities.

1.3 Overview of Waste Regulations and Guidance

The Resource Conservation and Recovery Act (RCRA) is the public law that creates the framework for the proper management of hazardous and non-hazardous solid waste to protect communities and natural resources in the country. Specifically, the law describes the mandate and authority given to EPA by Congress to develop the RCRA program, which comprises regulations, guidance, and policies to ensure the safe management and cleanup of solid and hazardous waste as well as encourage source reduction and beneficial reuse. The term RCRA is often used interchangeably to refer to the law, regulations, and EPA policy and guidance. However, **in the context of this study, the term refers specifically to EPA's** regulation of hazardous and non-hazardous solid waste management under RCRA Subtitle C and D, respectively.

Hazardous waste is defined under RCRA as a solid waste that is not excluded from regulation as a hazardous waste under 40 CFR §261.4(b) and meets the criteria listed in 40 CFR §261.3. Hazardous waste management requires treatment, storage, and disposal as appropriate under 40 CFR Part 264, with specific restrictions on landfill disposal of hazardous waste controlled under 40 CFR Part 268. These intersecting regulations are complex, with detailed discussion beyond the scope of this document. However, for the purposes of this study, it is important to emphasize that waste in liquid form cannot be directly landfilled but must be encapsulated, solidified, and/or stabilized before disposal. This general restriction extends to hazardous waste containerized in drums. As such, disposal of free-draining liquids in the landfill waste stream should not be an appreciable source of leachate at Subtitle C landfills.

This study is not specifically concerned with non-hazardous solid waste landfills regulated under RCRA Subtitle D as codified under 40 CFR Parts 257 and 258. Subtitle D establishes minimum technical standards and guidelines for state solid waste plans (EPA, 1993). Non-hazardous waste materials regulated under Subtitle D include (1) household refuse, also known as municipal solid waste (MSW); (2) sludges from wastewater treatment plants or

pollution control facilities; (3) non-hazardous industrial wastes (e.g., manufacturing process wastewaters and non-wastewater sludges and solids); and (4) other discarded materials resulting from industrial and commercial activities, (e.g., mining waste, oil and gas waste, construction and demolition debris, medical waste, agricultural waste, household hazardous waste, and conditionally exempt small quantity generator waste).

1.3.1 Regulation of Hazardous Waste Landfills

EPA has developed a comprehensive program to provide safe management of hazardous **waste from the moment it is generated to its final disposal (a “cradle-to-grave” approach)**. RCRA Subtitle C regulations set criteria for hazardous waste generators and transporters, and for treatment, storage, and disposal facilities. This includes permitting requirements, enforcement, and corrective action or cleanup. The regulations governing hazardous waste identification, classification, generation, management, and disposal are found in 40 CFR 260 through 273 (EPA, 2014a). EPA typically authorizes states to implement and regulate hazardous waste programs in lieu of the federal government. If a state program does not exist, EPA has the authority to directly implement hazardous waste requirements in that state.

This study is most concerned with 40 CFR Part 264: Standards for Owners and Operators of Hazardous Waste Treatment, Storage, and Disposal Facilities. Sections of the regulation of particular relevance are provided under Subpart N – Landfills (40 CFR §264.300 through .317) and Subpart G – Closure and Post-Closure (40 CFR §264.110 through .120). Most of the hazardous disposal facilities regulated under RCRA Subtitle C that were included in this study are commercial waste treatment, storage, and disposal facilities (TSDFs); however, two industrial TSDFs are also included in the study. TSDFs are used for treatment, storage, and disposal of hazardous waste produced by general industrial and manufacturing activities. Commercial facilities are owned and operated by waste management companies and accept waste from multiple industries whereas industrial facilities are owned by and contain waste from the industrial process run by the owner. Corrective action management units (CAMUs) are special units created under RCRA to facilitate treatment, storage, and disposal of hazardous wastes managed for implementing site cleanups, and to remove disincentives to cleanup that the application of RCRA can sometimes impose. Requirements for CAMUs are provided under Subpart S – Special Provisions for Cleanup (40 CFR §264.550 through .555).

1.3.2 Post-Closure Monitoring and Maintenance

The guidelines for post-closure monitoring and maintenance systems required at a hazardous waste landfill are listed in §264.310(b) and require the owner and operator to comply with all post-closure requirements contained in §264.117 through .120, including maintenance and monitoring throughout the post-closure care period (as specified in the permit under §264.117). Financial assurance requirements for post-closure care are specified under §264.145(i). The owner or operator of a hazardous waste disposal unit must have a written post-closure plan providing a description of the planned monitoring and maintenance activities, the function of the monitoring equipment, and frequencies at which monitoring and maintenance activities will be performed. Specifically, the owner or operator must:

1. Maintain the integrity and effectiveness of the final cover, including making repairs to the cap as necessary to correct the effects of settling, subsidence, erosion, or other events;
2. Continue to operate the leachate collection and removal system until leachate is no longer detected;
3. Maintain and monitor the leak detection system in accordance with 264.301(c)(3)(iv) and (4) and 264.303(c), and comply with all other applicable leak detection system requirements of this part;
4. Maintain and monitor the groundwater monitoring system and comply with all other applicable requirements of Subpart F (§264.90 through .101);
5. Prevent run-on and run-off from eroding or otherwise damaging the final cover; and
6. Protect and maintain surveyed benchmarks used in complying with §264.309 (Surveying and Recordkeeping).

Monitoring and inspection requirements for closed Subtitle C landfills specified under §264.303(c) include recording the volume of liquids removed from each leak detection system sump at least weekly during the closure period. After installation of the final cover has been certified, the volume of liquids removed from each leak detection system sump must initially be recorded at least monthly. If the liquid level in the sump stays below the pump operating level for two consecutive months, the amount of liquids in the sumps must be recorded at least quarterly. If the liquid level in the sump stays below the pump operating level for two consecutive quarters, the amount of liquids in the sumps must be recorded at least semi-annually. If at any time during the post-closure care period, the pump operating level is exceeded at units on quarterly or semi-annual recording schedules, the owner/operator must revert to monthly recording of the amounts of liquids removed from each sump until the liquid level again stays below the pump operating level for two consecutive months. In this regard, “**pump operating level**” refers to a liquid level based on

pump activation level, sump dimensions, and other considerations that will avoid backup into the drainage layer and minimize buildup of hydraulic head. Pump operating levels must be proposed by owner/operators and approved by the EPA Regional Administrator or Director of an authorized state program.

1.4 Landfills as Waste Containment Systems

1.4.1 Waste Containment Goals

Landfills are land-based waste management units that contain solid waste as well as byproducts of waste decomposition (conversion of solids to more mobile liquid and gaseous phases). Gaseous phase byproducts are primarily associated with biological degradation processes that form landfill gas (LFG) and are less important in RCRA Subtitle C landfills than in Subtitle D landfills regulated under 40 CFR Part 258, as the latter generally contains significantly more organic materials disposed of as part of an MSW stream. Although some Subtitle C landfills feature LFG control systems, the performance of containment systems in controlling gaseous phase byproducts is not considered in this study. This study is concerned with the control of liquid phase byproducts (leachate), which are primarily comprised of liquid that has percolated through or emerged from the solid waste and contains soluble or suspended materials removed from the waste (Pohland & Harper, 1985).

Waste containment systems for landfills consist of liner systems that underlay the wastes placed on them and final cover systems constructed over the wastes. The primary function of the liner system is to minimize, to the extent achievable, the subsurface migration of waste constituents and degradation byproducts (i.e., leachate and gases) out of the landfill. The primary functions of the final cover system are threefold: contain and isolate the waste from the surrounding environment; minimize, to the extent achievable, the percolation of water into the waste body; and control the atmospheric emission of gases, if any, from the landfill. To achieve their performance objective of protecting the environment, **multiple systems acting together are employed throughout the landfill's life (i.e., operation, closure, and post-closure)**. The performance objective for these systems is the protection of potential environmental receptors (groundwater, surface water, unsaturated soil, and air).

1.4.2 Containment System Components

Typical components of containment systems at Subtitle C landfills (Figure 1-1) and their role in meeting performance standards can be briefly described as follows:

- Liner system, typically a double-liner system with a composite clay/geosynthetic system, which provides containment of waste and waste byproducts (Rowe, 2005);

- Leachate management system, which collects leachate to minimize buildup of hydrostatic head above the liner and removes it for treatment and disposal (Rowe, 1998); and
- Final cover system (installed after closure), which provides long-term containment (Bonaparte et al., 2004), controls the rate of water entering the landfill from rainfall or snowmelt, provides storm water management, protects surface water quality, and can also provide a suitable platform for beneficial reuse options (Crest et al., 2010).

Active monitoring of landfill containment and control systems and potential receiving media is required (EPA, 2014a). Such monitoring is not only an important compliance tool to evaluate whether component systems are functioning as designed but also measures system performance over time.

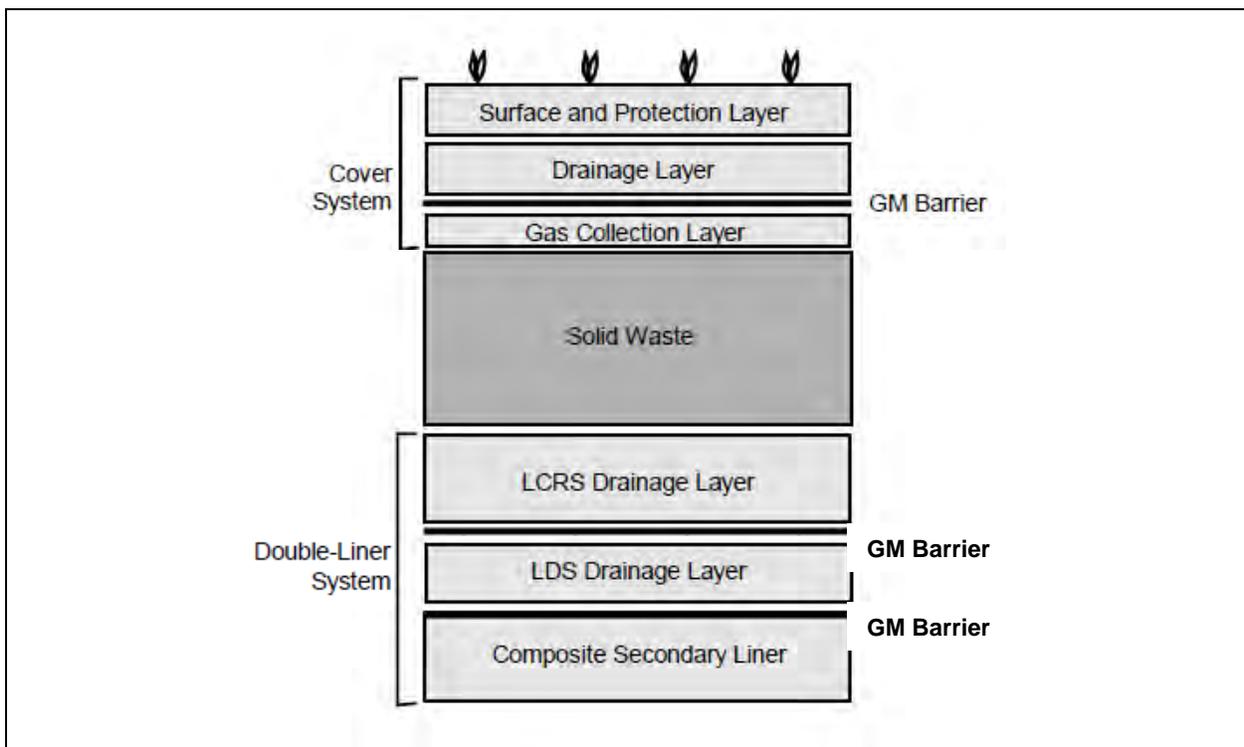


Figure 1-1. Schematic of typical cover and double-liner system for a Subtitle C landfill²

Liner and Leachate Management Systems

A liner system is a low-permeability barrier used to contain the waste and impede subsurface liquid or gas flow, primarily leachate. Liner systems are typically installed in accordance with an independent construction quality assurance (CQA) program, which provides third-party inspection, testing, documentation, and certification that liner

² Redrawn from Figure 5-1 of EPA (2002). Note that not all components will be present as shown in all Subtitle C containment system designs; in particular, a gas collection layer may not be specified and materials used in drainage layers may vary between granular soils and geosynthetic products.

components were installed in accordance with design specifications and regulatory requirements. Nevertheless, no liner material can be manufactured or installed to be perfectly impermeable (Giroud and Bonaparte, 1989; Giroud et al. 1997; EPA, 2002; Rowe, 2005). Therefore, the competent leachate containment design provides for a combination of barrier and drainage layers performing complementary functions. Barrier layers impede leachate percolation out of the landfill and improve the performance of overlying drainage layers, which serve to rapidly remove leachate and limit the buildup of hydraulic head on underlying barrier layers. Drainage layers collect and convey leachate from above the uppermost liner material towards controlled low-point collection points (sumps) on the liner where the liquids can be removed, thereby minimizing the hydraulic head on the liner and advective flux through the liner. Combinations of liners and drainage layers are collectively referred to as liner systems.

A double-liner system consists of a primary liner and a secondary liner, each overlain with a dedicated drainage layer. The drainage layer above the primary liner serves to remove leachate before it can develop a significant hydraulic head on the primary liner; as such, this drainage layer is referred to as the leachate collection and removal system (LCRS). The drainage layer between the primary and secondary liners serves to collect leachate that may leak through the primary liner; this drainage layer is thus referred to as the leak detection system (LDS). A schematic showing various components of a typical double-liner system is shown in Figure 1-1.

All double-liner systems being constructed at Subtitle C landfill facilities today have primary and secondary liners that include geomembranes (GMs). Due to its resistance to degradation by a wide range of chemicals, among other factors, high-density polyethylene (HDPE) is the most common type of GM barrier used in landfill liners. However, other GM materials include polyvinyl chloride (PVC), butyl rubber, polypropylene (PP), and Hypalon. Primary and/or secondary liners can consist of a GM alone, although this is rare, or a GM on top of a compacted clay liner (CCL) or geosynthetic clay liner (GCL). The latter two cases, which are referred to in this study as GM/CCL and GM/GCL liners, respectively, are known **as “composite” liners and significantly outperform single liners because the properties of the two different barrier materials work synergistically to maximize the performance characteristics of the other** (Rowe, 2011). Only double-lined landfills with GM, GM/CCL composite, or GM/GCL composite primary and secondary liners were included in this study. Sites with liner systems constructed with single CCL primary liners were not considered. The specific double-liner system types in service at the various landfill units included in this study are discussed in Chapter 3.

The LCRS and LDS drainage layers overlying the low permeability primary and secondary liners, respectively, which traditionally comprise a 12- to 24-inch layer of granular soil (sand or gravel), the main purpose of which is to collect and remove liquids to prevent buildup of hydraulic head on the liner, although the granular soil layer also serves to protect the liner system from damage during initial waste placement. Increasingly common, a specifically designed open-weave plastic mesh product termed a geonet (GN) or geocomposite (GC) is installed above the liner in conjunction with, or instead of, the granular soil layer to improve LCRS and/or LDS drainage performance and/or economics. Granular soil drainage layers are usually overlain by a geotextile (GT) fabric or similar permeable barrier to minimize intermixing of overlying waste and protective soil/gravel layers. A protective soil layer is installed above the LCRS, and/or the first two to four feet of waste is carefully selected and **placed to form a protective “fluff layer” above the LCRS**. A schematic illustration showing the locations of LCRS and LDS drainage layers in a double-liner system is shown in Figure 1-1. The specific LCRS and LDS designs and material types in service at the various landfill units included in this study are discussed in Chapter 3.

Cover System

Landfills require daily covers, intermediate covers, and final cover systems, depending on their stage of development. At most landfills, a daily cover (soil, select waste, or other material such as foam or fabric) is applied to waste at the end of each working day to provide temporary control of vectors and erosion of waste by wind and surface water runoff. The daily cover was not a primary focus of this study. The intermediate cover is often placed on open portions of landfill areas on which waste placement has ceased, either permanently or for an extended period. The intermediate cover serves the same purposes as daily cover, but at a higher performance level, and often comprises the subgrade foundation upon which a final cover is constructed. Intermediate cover usually consists of a thicker layer of soil or select waste than the daily cover and may include a temporary GM. As the active period of operation progresses, the landfill is filled with waste, and waste placement ceases. Depending on the method of operation, landfill units may be under intermediate cover for up to several years before a final cover system is constructed over the waste. The intermediate cover is of interest in this context because one of the case study landfills includes a unit in which waste has been placed to final grades but the final cover system has not yet been constructed.

The final cover system at a Subtitle C landfill typically consists of a barrier layer (GM, GM/GCL, or GM/ CCL) overlain by drainage and soil protection/vegetation layers. The barrier

layer is constructed on a subgrade foundation layer above the waste. A gas distribution/collection layer is included beneath the barrier layer at landfills with wastes that generate gases during decomposition (EPA, 1989, 1991; Bonaparte et al., 2004). Final covers are engineered systems constructed over the entire aboveground surface of the landfill (i.e., a top area and side slopes). Final covers are designed to minimize water infiltration into the waste (i.e., leachate generation), control the migration of gases produced by waste decomposition, prevent against inadvertent intrusion, and be aesthetically acceptable (Koerner and Daniel, 1997). Similar to liner systems, final cover systems are typically installed in accordance with an independent COA program, which provides third-party inspection, testing, documentation, and certification that cover system components were installed in accordance with design specifications, although (again similar to liners) no cover material can be manufactured or installed perfectly (EPA, 2002; Rowe, 2005). A schematic illustration of the various components of a typical cover system is shown in Figure 1-1. In some jurisdictions, the final cover system is often considered to replace the liner system as the primary means of environmental protection once it is installed at a site during closure construction. The specific cover system types in service at the various landfill units included in this study are discussed in Chapter 3.

1.5 Long-Term Performance of Landfill Containment Systems

1.5.1 *Liner-System Performance*

A number of textbooks and guidance documents have been developed to provide recommendations for design, permitting, operation, performance, and monitoring of hazardous waste landfills. However, most active research on design and performance of landfill containment systems predates the publication of the seminal study by EPA (2002) and was captured in the extensive literature review provided therein. That discussion is not reiterated here.

A report published by the National Academic Press (2007) on the performance of engineered barriers for containment of MSW, hazardous and toxic waste, and low-level radioactive waste (LLRW) focused on answering two primary questions: How well are engineered barrier systems working? How long are they likely to work effectively? Based on 20 years of data, **the report concluded that** “most engineered waste containment barrier systems that have been designed, constructed, operated, and maintained in accordance with current statutory regulations and requirements have thus far provided environmental protection at or above specified levels. Extrapolations of long-term performance can be made from existing data and models, but they will have high uncertainties until field data are accumulated for longer

periods, perhaps 100 years or more. We will never have all the long-term observations and data that we would like.” **The report concluded that significant failures have been rare and, in general, repair or limited reconstruction has been possible when needed.**

Since the late 1990s, a number of on-site disposal facilities (OSDFs), which are doubled-lined landfills for containment of mixed LLRW and RCRA wastes, have been constructed as part of the decommissioning and remediation of the U.S. Department of Energy (DOE) facilities such as the Feed Material Production Center in Fernald, Ohio and Idaho CERCLA Disposal Facility in Idaho Falls, Idaho (Koslow, 2015). Liquids management data (i.e., leachate collection system and leakage detection system flow rate and liquid chemical constituent data) from the operational and post-closure periods have been reported for a number of these facilities (e.g., Benson et al., 2007; Bonaparte et al., 2011; Bonaparte et al., 2016). In the latter study, performance data from three facilities were analyzed to calculate hydraulic containment efficiencies of the liner systems and to draw conclusions as to whether the liner systems are performing as expected. Based on the data presented, all facilities were found to be “performing very well in providing containment and collection of leachate.” Performance metrics for the facilities were consistent with those presented in EPA (2002). However, the authors noted that the public availability of data for several of the facilities was limited and recommended more intensive liner system monitoring and information dissemination by DOE.

Although some recent research has focused on the design and hydraulic performance of CCLs exposed to high strength leachate (Safari et al., 2012), most geotechnical research on landfill containment systems since 2002 has focused on long-term material integrity and aging and their expected effect on the service life of HDPE geomembranes and other geosynthetics used in liner applications (e.g., Sangam and Rowe, 2002; Hsuan et al., 2005; Rowe and Rimal, 2008; Rowe et al., 2009; Rowe and Hoor, 2009; Rowe et al., 2010), including in applications at landfills for containment of mixed LLRW and RCRA wastes (Jo et al., 2005; Tian, 2015). The most significant aging mechanism in HDPE geomembranes used in landfill liners is chemical aging, with the extraction of antioxidants and then oxidation being the main degradation mechanisms. Eventually, the geomembrane will likely become brittle to the extent that it is considered to have reached the end of its service life. In addition to the in-service stresses imposed on a GM, Rowe, and Sangam (2002) highlighted that the real service life of a GM depends on its mechanical, hydraulic, and diffusive properties. Thus, a GM may lose strength while still performing satisfactorily as a barrier. Accordingly, the true hydraulic and diffusive service life of a GM may significantly exceed

the service life determined based on the degradation of the physical and mechanical properties, especially if the tensile stresses are minimal.

Many researchers (cit. in Rowe, 2007 and 2011) have focused on the importance of manufacturing and construction practices in minimizing long-term field leakage through composite liners. **Rowe (2011) concluded that** “composite liners have performed extremely well in field applications for a couple of decades and that recent research both helps understand why they have worked so well, but also provides new insight into issues that need to be considered to ensure excellent long-term liner performance of composite liners.” Factors potentially affecting the field performance of barrier materials in liners include avoiding excessive wrinkles in GMs, moisture loss and shrinkage of GCL panels (particularly overlaps), and desiccation of CCLs. These can be mitigated by imposing strict quality control (QC) protocols on manufacturers and COA procedures on installation, particularly minimizing exposure to the sun and the wind (Rowe and Hosney, 2010).

The potential for LCRS clogging and malfunction has also been considered by several researchers (e.g., Fleming and Rowe, 2004; VanGulck and Rowe, 2004; Cooke et al., 2005; Rowe, 2005). Rowe (2005) reported that clog material forms by biologically induced processes that involve the removal of some of the organic leachate constituents and precipitation of some inorganic leachate constituents followed by an accumulation of inorganic particles originally suspended in leachate. Research suggests that the potential for clogging depends on the amount and composition of leachate and on the details of the design of the LCRS. However, most of the research has been performed on MSW landfills; as hazardous waste landfills generally contain significantly lower quantities of organic waste, which should reduce their clog potential. The lower the leachate generation rate, the lower the potential for clogging (other factors being equal). Given the significant decrease in leachate generation rates after landfill closure, the potential for biological clogging of the LCRS decreases after the landfill is closed with a final cover system.

1.5.2 Leachate Generation Modeling

Leachate generation can be estimated using water balance models, which are based on the principle of conservation of mass, in which water mass is conserved through the process. Typically, water in a landfill exists as an input, output, or in storage. The most important factor in the water balance equation is the storage in the cover soil and evapotranspiration. These two factors greatly affect surface runoff and the amount of precipitation that is allowed to infiltrate the cover system barrier layers (Kaushik et al., 2014). There are many variations in the assumptions and algorithms these models use. Generally, site-specific

geometry (e.g., waste thickness, base liner, side slope inclination) as well as climatic and soil data are input to predict flow over a period of time. A water balance may be performed for various periods, ranging from one month to several decades, depending on the amount of data necessary. All models are limited by availability and accuracy of site-specific data (Peyton and Schroeder, 1993; EPA, 2004).

Although many public and commercial water balance models are available, the HELP Model developed by the U.S. Army Corps of Engineers for the EPA (Schroeder et al., 1994a and b) is the most widely used hydrologic model in the landfill industry, both in the United States and internationally (Berger, 2003). The model can be used to evaluate percolation through cover and liner systems, hydraulic head on liners, and, by association, leakage of leachate to the subsurface (Yalcin and Demirer, 2002; Alslaibi et al., 2013). Three versions of the model are currently available: HELP 3.07, Visual HELP 2.2, and HELP 3.95 D. Plans for improving the model and developing a future version (HELP 4 D) based on ongoing validation results and requests from the practice are described by Berger (2015).

HELP is a quasi-2D model because it considers either vertical or horizontal flow in each layer, but not both simultaneously. The model is popular for its ease of use in comparing different cover and liner types; a broad database of climatic and material property default values; ability to provide daily, monthly, or annual output; consideration of lateral drainage; and ability to evaluate up to 20 layers. The HELP Model has widespread regulatory acceptance both in the United States and internationally. There have been several criticisms levied about the model; for example, that it overestimates leachate generation and percolation through covers (particularly in arid and semi-arid conditions) by underestimating evapotranspiration (Vorster, 2001). However, many findings from validation studies are contradictory: for example, the model is reported to either under-predict or over-predict surface runoff (Paige et al., 1996) and assign too high or too low default values for field capacity and hydraulic conductivity depending on geographic region (Uguccione and Zeiss, 1997). Some limitations are due to the maximum 1-day time resolution of the model and its input data. Any model using daily precipitation data can give only a rough estimate of surface runoff, especially in poorly vegetated regions experiencing large precipitation events. The empirically derived Soil Conservation Service (SCS) curve number method underlying the runoff sub-model should, strictly speaking, be calibrated for different regions and time periods of application. Similarly, using daily precipitation data cannot reproduce short-term peaks in lateral drainage from drainage layers in final covers (Berger, 2015).

Conventional soil mechanics theory used to describe hydraulic properties of solid waste such as field capacity assumes Darcian flow through a homogeneous porous matrix. However, the landfill waste body is, by its nature, highly heterogeneous. Observations (e.g., Bendz et al., 1997; Rosqvist, 1999; Fourie et al., 2001; Rosqvist et al., 2005) have shown significant channeling, or preferential flow, through waste layers, albeit generally measured at MSW landfills under high infiltration rates (open conditions). This results in lower practical field capacities, faster breakthrough times, and higher and non-uniform leachate discharge rates than obtained from the HELP Model, which assumes homogenous Darcian flow (Fellner and Brunner, 2010). Low infiltration rates (such as occur after application of landfill covers), however, are less likely to lead to pronounced channeling than high rates, because more time is allowed for absorption of water into waste particles, and capillary action in smaller pores redistributes moisture so that uniform flow through the waste layer may contribute more to overall moisture movement. As a result, channeling may only occur significantly in the initial phase of landfilling, or where high rates of liquids injection are attempted at bioreactor facilities (Bengtsson et al., 1994). This suggests that the HELP Model should be better at predicting leachate generation during PCC than during the operational period.

1.5.3 Potential Sources of Liquids Contributing to LCRS and LDS Flow

Leachate is produced when the field moisture-holding capacity of the landfill contents is exceeded. This occurs when the waste moisture deficit (the difference between the waste moisture content at placement and field capacity) is exceeded. Four principal factors govern leachate production at a landfill (Rees, 1980):

- The water content of the waste when placed;
- The volume of infiltrating rainfall;
- The volume and water content of sludges co-disposed with the waste; and
- Waste compaction and density.

Prior to closure, precipitation levels would be expected to be directly correlated to leachate generation (i.e., liquids volume recovered in the LCRS), and thus also have an indirect impact on LDS flow rates, since higher LCRS flow rates mean greater potential for primary liner leakage. However, leachate generation during the post-closure period is primarily controlled by the cover system, which limits infiltration. As such, LCRS and LDS flow rates would be expected to decrease significantly following completion of closure construction. Thereafter, any residual post-closure LDS flow is expected to be more dependent on imperfections in the liner and cover system construction (e.g., anchor trench tie-in and

welding of cover and liner system geosynthetics), cover erosion issues, and groundwater conditions than on precipitation.

The performance of primary liners at double-lined landfills in limiting leakage of leachate is generally inferred by comparing LCRS and LDS flow rate and chemical constituent data. Pioneering studies by Bonaparte and Gross (1990) and Gross et al. (1990) identified four main potential sources of liquids contributing to LDS flow (Figure 1-2). The potential sources of LDS liquids identified in the figure include:

- Primary liner leakage (Source A);
- Construction water and compression water (Source B), comprising water (mostly rainwater) that infiltrates the LDS during construction and continues to drain to the LDS sump after the start of facility operation;
- Consolidation water expelled into the LDS from the CCL/GCL components of a composite primary liner as a result of clay consolidation under the weight of the waste (Source C); and
- Groundwater that percolates vertically or laterally through the secondary liner from outside the landfill or other external sources of water (e.g., condensation of water vapor in any gases encapsulated within the landfill), which condenses and infiltrates the LDS (Source D).

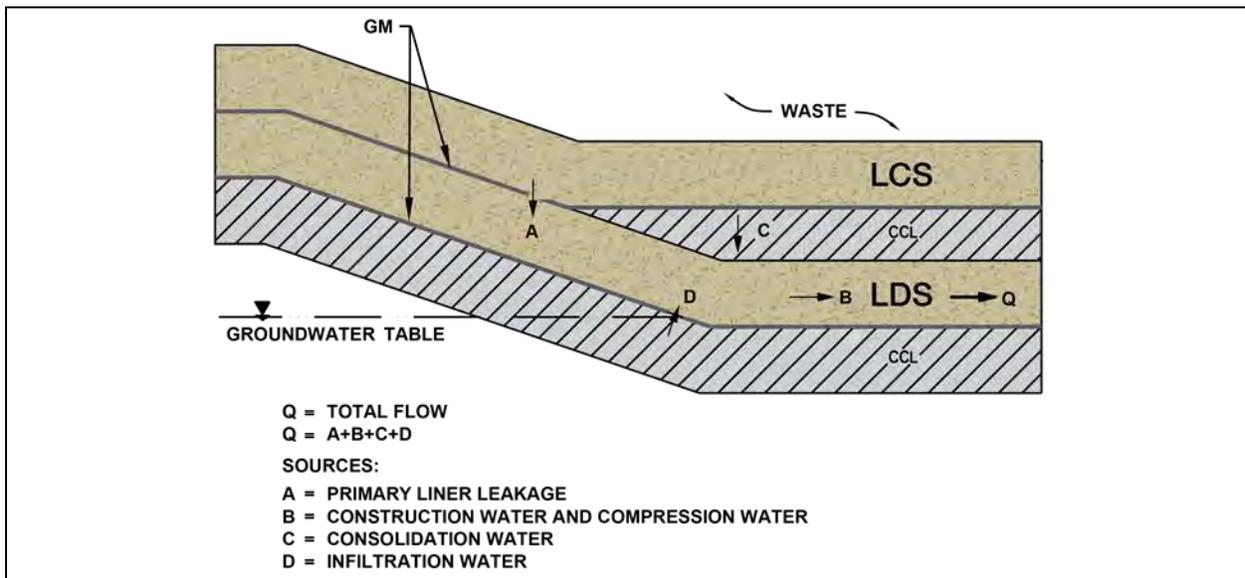


Figure 1-2. Potential sources of liquids contributing to LDS flow³

The contribution of Sources B and C is not insignificant, particularly at sites having granular drainage layers and thick CCL barrier layers (Bonaparte et al., 2011). However, such sources are expected to have less relevance when only post-closure performance of the liner is considered.

³ Based on Figure 3 of Bonaparte and Gross (1990), used with permission of the authors.

Gross et al. (1990) presented the following five-step approach for evaluating the sources of LDS liquid at a specific waste management unit:

1. Identify the potential sources of flow based on double-liner system design, climatic and hydrogeologic setting, and landfill operating history;
2. Calculate flow rates from each potential source;
3. Calculate the time frame for flow from each potential source;
4. Evaluate the potential sources of flow by comparing measured flow rates to calculated flow rates at specific points in time; and
5. Compare LCRS and LDS flow chemistry data to further establish the likely sources of liquid.

The approach outlined above was used to evaluate the performance of the primary liner systems in this study as discussed in Chapter 4. In addition to mechanisms identified above, the operators of participating case study sites were asked about other possible reasons for higher-than-expected flows in LDS sumps; where this occurred, it was mainly attributed to issues with the anchor trench tie-in design or cover erosion and repair. Selection of secondary liner system and LDS drainage layer materials on a side slope and base liners may also be important where groundwater is shallow and potentially in contact with the liner.

1.5.4 Leachate Chemistry

Leaching, which is the release of compounds from a solid to a solution, typically involves a number of interrelated physical, chemical, and biological processes (e.g., degradation, dissolution, desorption, complexation, or mineralization) and transport mechanisms, which can be grouped into those predominantly controlled by diffusion and those predominantly controlled by percolation and kinetics. Diffusion occurs where percolating liquids move mainly over the surface of a block of material rather than around individual particles or grains. Therefore, apart from initial wash-off effects, contaminant release is by diffusion through the interstitial spaces inside the block to the exposed surface. Contaminant release is thus related to the extent of this exposed surface. Percolation and kinetics refer to the situation where the liquid passes through a material, which comprises a mass of particles or granules and comes in direct contact with the surface of each particle. Contaminants transfer into solution by diffusion or dissolution and then migrate with the leachate. The factors controlling leaching also affect the composition of the resulting leachate, which in turn affect long-term management and treatment options (Renou et al., 2008). These factors include (Rees, 1980; Heasman, 1997; Morris, 2001):

- The source, nature, and physical properties of the waste material;

- The leaching mechanism;
- The chemistry of the contaminants concerned;
- The pH and redox potential of the leaching environment; and
- The nature and rate of movement of percolating liquids.

With regard to the last factor, hazardous waste materials may not degrade or only degrade very slowly, under landfill conditions. Reliable data on landfill leachate constituents collected over multiple years can support analyses of landfill processes and are needed to predict long-term trends in leachate chemistry with statistical confidence (Kylefors, 2003).

However, while considerable research into the long-term composition of leachate has been conducted (e.g., Kjeldsen et al, 2002; Statom et al., 2004; Öman and Junestedt, 2008; Gibbons et al., 2014) this has tended to focus on non-hazardous waste landfills. In light of this, Tian (2015) analyzed leachate composition from four landfills constructed for containment of LLRW and hazardous wastes in the United States and compared concentrations of dissolved organic matter (measured as total organic carbon [TOC]), inorganic macro-components (including major cations and anions), and trace metals to values reported in the literature for MSW leachate. The study concluded that:

- Dissolved organic matter concentrations were insignificant when compared with MSW leachate;
- Concentrations of inorganic macro-components were broadly similar to MSW leachate; and
- Trace metal concentrations were relatively lower than in MSW leachate.

For major cations, the concentrations of Ca and Mg were found to be similar to those in MSW leachate, while K and Na concentrations were higher in MSW leachate. For major anions, sulfate concentrations were much lower in MSW leachate. Interestingly, the concentrations of trace metals were found to be relatively constant over time at the four sites studied. Overall, if current expectations are that the time taken for concentrations of constituents of concern in leachate to decrease to asymptotic levels will be similar for hazardous and non-hazardous landfills, this may not be appropriate.

Most recent research into the properties of hazardous waste and its byproducts of degradation and decay has focused on landfill diversion of materials in support of **zero waste principles or the “circular economy”** (e.g., López- Delgado and Tayibi, 2012). An exception is municipal solid waste incineration (MSWI) bottom ash and fly ash, which contains a considerable amount of heavy metals, salts, organic pollutants, and other potentially toxic components. Investigation into recovery of metals with resale value (e.g.,

copper, zinc, lead, and cadmium) from MSWI by controlled leaching using different solutions (nitric acid, hydrochloric acid, and sulfuric acid) and optimized parameters (temperature, controlled pH value, leaching time, and liquid/solid ratio) was reported by Tang and Steenari (2016) with recovery rates varying from 68 to 92%, although it is doubtful such conditions for leaching could exist in situ. At many hazardous waste landfills, incoming waste is mixed with a bulk solidifier/stabilizer (cement kiln dust, fly ash, Portland cement, or activated carbon), which can make up as much as 40% of the volume in a given landfill unit, or stabilized by encapsulation using polymers (Kim et al., 2009; López et al., 2015). The stabilization process can immobilize hazardous organic materials such as phenol and reduce the potential for heavy metals to leach out of the waste (Rho et al., 2001; Reich et al., 2002).

Despite the shortfall in leachate characterization studies and need to establish a basis for expectations regarding trend behavior, hazardous waste landfill leachate data can help determine liner performance and identify groundwater contamination sources, depending on the signature relationship to landfill sections, waste type, and waste age. Of particular interest to this study, comparison of the concentrations of key chemical constituents in temporal LCRS and LDS chemistry datasets can be used to establish the extent of the hydraulic connection between these drainage layers (i.e., whether primary liner leakage had contributed significantly to observed LDS flows). Several factors need to be considered in identifying the constituents of interest, including common occurrence in leachate, high solubility in water and low octanol-water coefficient, and high resistance to hydrolyzation.

The parameters analyzed from sites used in this study included pH, specific conductance, total dissolved solids (TDS), chemical oxygen demand (COD), major cations and anions, trace metals, and volatile organic compounds (VOCs). The chemical signatures for these parameters in LDS vs. LCRS liquids serve as justification for proceeding with correcting liner efficiency calculations; this is discussed in Chapter 5. If sufficient data on major cations and anions are available, Stiff and Piper diagrams can be used to subjectively describe ionic solutions and distinguish leachates with similar chemistry into clusters (Tonjes, 2013). Further, with respect to the major ions, chloride is of particular interest since it serves as a conservative parameter that does not take part in biochemical reactions and is not physically altered during the processes of leaching (Rowe, 1991). Therefore, despite differences in volumes between LCRS and LDS flow rates, chloride concentrations should be approximately constant between these two drainage layers if the primary source of liquids in the LDS is leakage through the primary liner.

1.5.5 *Liner and Cover Stability*

Problems affecting liner stability generally occur as a result of short-term unstable conditions developing during operations (Blight, 2008; Mitchell, 2009). Potential causes of liner instability include inadequate design (e.g., inappropriate selection of smooth GM materials in contact with slick clay resulting in lower-than-expected interface friction), poor construction inspection and quality assurance, and/or inadequate control of liquid waste and moisture conditions. For the most part, such problems are avoidable. Liner issues not addressed during construction would be difficult to repair during operation and almost impossible to repair after the closure of the landfill. However, liner failures after closure have not been reported at modern RCRA landfills.

Most cover stability issues are identified based on visual observations during routine inspections. These are generally less likely to increase the potential for environmental impacts due to compromised waste containment system integrity than liner system failures. Cover issues related to erosion or storm water ponding may result in increased infiltration and leachate generation; however, most cover issues can be remedied in a relatively straightforward manner if detected and repairs are made in a timely fashion.

1.6 Assessment and Termination of Post-Closure Care

1.6.1 *Number of Closed Hazardous Waste Units*

The EPA Office of Inspector General recently issued a report (EPA, 2015) to evaluate whether the EPA and authorized states and territories have sufficient safeguards to control and mitigate long-term public health, environmental, and fiscal risks at hazardous waste disposal sites beyond the 30-year PCC period. The report identified over 1,500 hazardous **waste disposal units that had been assigned an operating code of “closed with waste in place” as of 9 October 2014** (Figure 1-3), although not all are reported to have entered into a permitted PCC program.

1.6.2 *Process for Assessing Completion of Post-Closure Care*

PCC for each hazardous waste management unit must begin after completion of closure and continue for 30 years thereafter according to 40 CFR §264.117(a), although discretion is provided to the permitting authority (EPA Regional Administrator or Director of an authorized state program) to adjust the post-closure period as necessary to protect HHE. Therefore, the presumption of a 30-year PCC period does not reflect a determination by EPA that 30 years is necessarily sufficient to eliminate potential threats to HHE in all cases. The regulations provide authority to conduct a case-by-case review of the PCC period and to

establish arrangements to adjust the length of the post-closure care period on a facility-specific basis, where the records support a determination that the revised post-closure period will protect human health and the environment (EPA, 2016). In other words, the duration of PCC is a performance standard rather than a prescriptive standard.

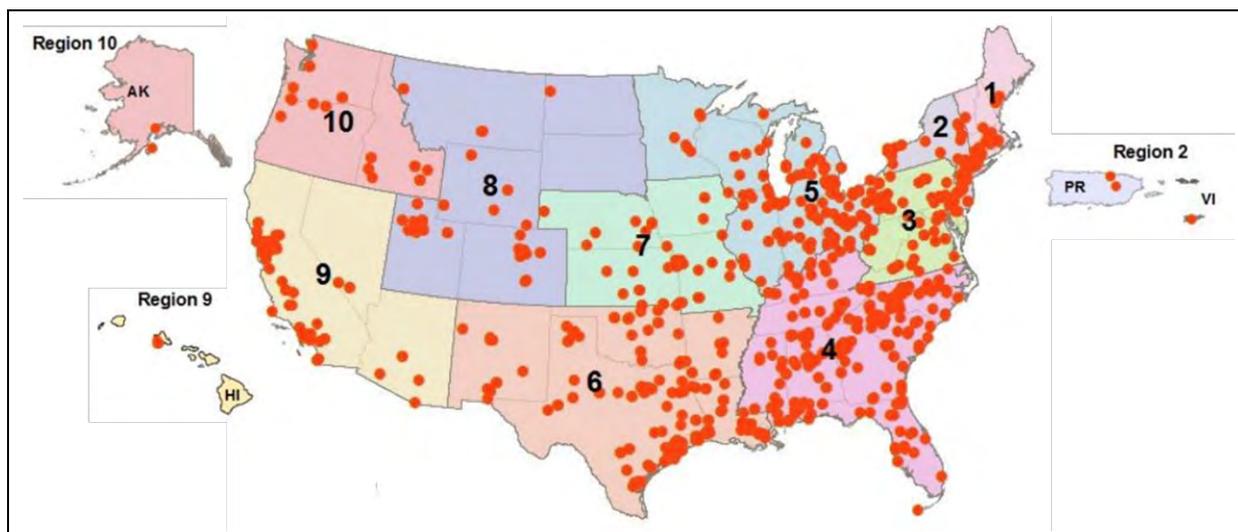


Figure 1-3. Distribution of closed hazardous waste disposal units in the United States⁴

In terms of the current state of the industry, EPA (2015) concluded that some important safeguards are in place, such as corrective action and other enforcement authorities that the EPA and authorized states can use to address cleanup needs at facilities undergoing PCC. States have exercised their authority, extending PCC and associated financial assurance when unacceptable risks remain. One state (Virginia) has also ended post-closure care at one facility and established other long-term care arrangements under an environmental covenant (see Section 1.6.3). If long-term problems arise after PCC, the implementing authority may be able to address these problems using its RCRA enforcement authority. Nevertheless, a number of challenges remain:

- In the absence of the finalized additional guidance from EPA, states have to make decisions on adjusting the PCC period;
- 18 states do not have environmental covenant statutes that strengthen controls for long-term protection of land use; and
- EPA and state hazardous waste programs will have an increased workload as more units reach the end of their expected 30-year PCC periods (the number of units for which a decision regarding the end of permitted PCC will be needed was estimated at between

⁴ Modified from the cover map of EPA (2015). Red dots show the locations of hazardous waste disposal units. Numbers and color shading groups indicate EPA regions.

15 and 45 annually through 2030, with over 50% of these decisions falling in the next 10 years).

Post-closure activities must be continued until the permitting authority (generally, the state) determines that the facility is performing acceptably and that the post-closure permit can be terminated. If the permittee (i.e., site owner/operator) is unable to continue providing PCC under these conditions, in accordance with applicable terms of the financial mechanism used to provide the financial assurances the state may provide PCC using an independent third party contractor. To cover the costs of such post-closure care under these circumstances, the state would exercise the financial assurances provided for PCC. For this reason, many states require routine assessments of the adequacy of funding and financial assurances for PCC at all facilities within their jurisdiction. Utah Senate Bill 24 of 2005, for example, requires an assessment every 5 years (Baird and Seiger-Webster, 2011).

A few states have used their authority under RCRA to extend PCC. For example, Maryland evaluated information on a disposal unit approaching the end of its 30-year PCC care period in 2012. Maryland identified continuing risk and required the owner to renew its PCC permit for another 10 years. Further, Maryland required the owner to maintain financial assurance to cover this extended care. The amount of financial assurance is to remain great enough to cover 10 years of care throughout the extended permit period (EPA, 2015).

1.6.3 Case Study Example of Post-Closure Permit Termination

This study found evidence of only one Subtitle C landfill facility at which PCC has been demonstrated complete and the PCC permit terminated (Romanchik, 2013; EPA, 2015). This is the Wheelabrator Corporation Landfill, a 2.7-acre landfill located on 13 acres of land near Bedford, Virginia. The landfill was used for disposal of furnace dust and furnace slag generated from secondary steel (scrap) smelting operations conducted at the adjacent Wheelabrator Abrasives foundry. The landfill was operated for 16 years through 1985 and then closed with waste in place on 21 December 1988. At the time of closure, the waste inventory was estimated as 122,900 cubic yards.

On 29 September 1992 Virginia Department of Environmental Quality issued Wheelabrator a Hazardous Waste Management Post-Closure Permit (Permit) that required monitoring of upgradient and downgradient groundwater at the closed landfill as well as maintenance of the landfill cap. On 17 July 2003, the Permit was renewed for a 10-year period through 16 August 2013 with groundwater monitoring requirements reduced from quarterly to semi-annual sampling. On 29 September 2008, the compliance period (16 years) for the regulated unit ended and the groundwater monitoring constituent list was significantly

reduced. Finally, on 9 August 2013, the department approved Class 3 Permit Modification to reduce the PCC period from 30 years to the time served to date. To comply with long-term stewardship goals, an environmental covenant was executed **under Virginia's Uniform Environmental Covenants Act** that allowed the site out of PCC in exchange for an annual certification by a certified professional engineer that required cap monitoring, maintenance, and site security obligations covered under the covenant are being met.

Based on a review of available documentation, it appears that termination of PCC at this site was approved based on demonstration of no unacceptable threat to HHE as a result of potential long-term leakage of leachate to groundwater. As such, the approval invokes the **department's authority under the performance standard implicit in §264.117(a)** rather than strict invocation of the prescriptive requirement under §264.310(b)(2) to continue to operate the leachate collection and removal system until leachate is no longer detected.

1.7 Long-Term Landfill Performance and Resilience

Some hazardous waste management units in place today may be under-designed for the future if conditions change significantly relative to recent historical patterns. This could have serious consequences for the integrity of hazardous waste disposal facilities, such as a cover system breach causing either subsidence or leaching of contaminants into the subsurface (Kelly and Winchester, 2005). Therefore, the vulnerability of existing and proposed hazardous waste disposal facilities should be evaluated with regard to long-term climatic hazards (e.g., inundation due to sea level rise, elevated temperatures, and/or groundwater elevation rise) as well as short-term hazards (e.g., possible increase in precipitation and associated flooding, increases in storm flooding/surges, potential changes in wave action and currents, king tides, seismic events such as earthquakes or tsunamis, and/or El Niño effects). Little research has been published on the long-term vulnerability of closed landfills to these events; as such, this represents a research need in terms of assessing the long-term performance of landfill containment systems.

Although very long-term design considerations and the effects of extreme climatic or other natural events are not routinely considered for RCRA Subtitle C landfills, very long-term performance requirements are considered for waste encapsulation designs (Reith and Caldwell, 1993) used for containment of extremely hazardous or radioactive wastes. For example, functional requirements for the design of an on-site disposal facility (OSDF) for mixed LLRW and RCRA waste containment based on 40 CFR §192.02(a) with a 1,000-year design life are described by Bonaparte et al. (2011 and 2016). These included potential mechanisms for performance failure such as long recurrence interval earthquake and storm

events. Changes in erosional stability over time were also considered. For design, the performance period was divided into three operating timeframes: initial period, which extends from construction until the end of the 30-year post-closure monitoring period; intermediate period, which begins 30 years after final closure and lasts for at least 200 years and up to 1,000 years; and final period, which does not occur for at least 200 years and possibly up to 1,000 years after final closure of the facility. During the final period, it is assumed that liner and final cover system geosynthetics are non-functional along with synthetic components of LCRS and LDS drainage systems and pipes.

According to the National Academic Press (2007), long-term containment designs that allow for lifetimes of thousands of years are likely infeasible and prohibitively expensive; therefore, designs that allow for recovery, repair, or replacement of the barrier system components for the landfill cover system should be encouraged. The potential effects of changes in temperature and precipitation, sea level rise, and related flooding, as well as other extreme events such as earthquakes, could be minimized by building resiliency in design for repair and replacement of the barrier system components. The appropriate continuing management strategy would include ending maintenance, continuing maintenance, repair, and rehabilitation.

2. DATA COLLECTION AND ANALYSIS

2.1 Data Collection

The EPA, in partnership with the States, biennially collects information regarding the generation, management, and final disposition of hazardous wastes regulated under RCRA and publishes a National Biennial Report to communicate the findings (EPA, 2011). This includes a list of reported RCRA sites in the United States, which helped in initial identification of candidate sites for this study.

Efforts were made to obtain data from a variety of facilities to represent the different waste generator and operator types regulated under Subtitle C, including commercial facilities and landfills dedicated to a specific industrial waste stream. Variability in geographic/climatic conditions was also sought, as discussed below. The cooperative participation of site managers and other operator personnel was also seen as key to success, in particular in understanding the nuances in the data (for example, many apparent anomalies can be readily explained from operational records: a 5-day spike in leachate flow may simply be the effect of a malfunctioning flowmeter that took a few days to notice, isolate, and replace). However, it is recognized that non-random selection of sites is likely to have biased the data set. Therefore, to gain some understanding of the extent of data that may be available without direct contact with the operator, site records and analytical data for one site (denoted Landfill F in the study) were obtained directly from the state.

2.1.1 Criteria for Case Study Site Selection

Landfill disposal units (i.e., cells, phases, modules, or areas) sought for inclusion in this study offered the following four main characteristics:

- Regulated under RCRA Subtitle C;
- Closed (i.e., final capped) or at final grade for an extended period under intermediate cover;
- Have a double-liner system with separate (unit-specific) measurement of liquid flow rate in the LCRS and LDS; and
- Collect leachate chemistry data independent of any active units in operation at the same facility.

2.1.2 Geographic and Climatic Distribution

For the purposes of this study, the continental United States was divided into four geographic regions: Northeast (NE), Southeast (SE), Northwest (NW), and Southwest (SW). These regions were constructed to broadly reflect climatic differences, as represented by

average annual rainfall and temperature (Figure 2-1). Regional representation was established as a goal in selecting case study landfills for inclusion in the study.

Generally, facilities in the SE experience higher rainfall and evapotranspiration, and fewer days below freezing annually than other regions. Compared to the SE, facilities in the NE receive slightly lower rainfall, have lower evapotranspiration, and experience a significant number of days below freezing annually. Both the SW and NW are relatively dry (ignoring the coastal Pacific Northwest), with relatively low precipitation and relatively high evapotranspiration. Facilities in the SW may not experience any days below freezing annually while facilities in the NW should expect a significant number of days below freezing annually.

The climatic differences between regions would be expected to have a direct impact on leachate generation (LCRS flow rates) as well as an indirect impact on LDS flow rates since higher LCRS flow rates mean greater potential for primary liner leakage. However, LDS flow rates may be more dependent on liner system construction, cover system construction (in particular, tie-in of to the liner system anchor trench by welding cover and liner geomembranes (GM) together), and groundwater conditions than on precipitation. Climatic and other influences on LDS flow are examined as part of this study.

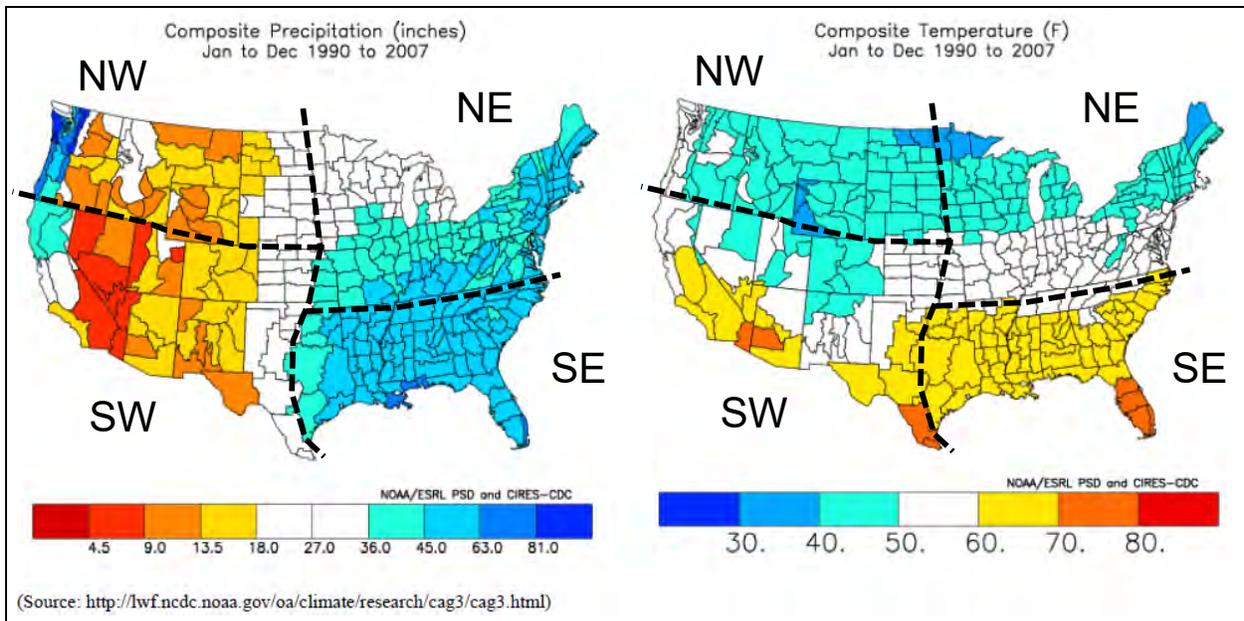


Figure 2-1. Geographical regions established for the study

2.1.3 Site Data Collection Protocol

The dataset available from closed landfill facilities reported in EPA (2002) included only 9 years of post-closure data from both MSW and hazardous waste units. A primary objective

of this research was to extend the dataset for hazardous waste units by 20+ years. Thus, in developing a data collection protocol, care was taken to review material used in the preparation of that study and interview a number of professionals that were involved in data collection and analysis for that study. A review of other relevant publications and guidance related to liner, cover, and LCRS design at Subtitle C landfills was conducted to develop a Data Requirements Checklist (Appendix I). Checklist data were broadly categorized in terms of (1) design and construction of the liner, LCRS, LDS, and cover; (2) waste placement schedule, characteristics, and in-place volume; (3) LCRS and LDS flow quantities; and (4) leachate chemistry data.

To obtain the data identified on the checklist, owner/operators, EPA regions, and state regulators of Subtitle C landfills were contacted to identify candidate sites with one or more closed units that meet the minimum data requirements specified in the checklist. Efforts were made to include facilities in a variety of regions and climatic conditions. Based on feedback received, the data requirement checklist and/or candidate list was modified and a shortlist of sites developed from the initial pool of candidates. Thereafter, the operators of shortlisted sites were contacted with a formal request for participation in the study. After reviewing responses, a final list of sites was selected.

Data for this study were collected by Geosyntec during 2015 and 2016. Initially, data were requested in electronic format. After reviewing the data received for completeness in relation to the checklist requirements, Geosyntec traveled to the **site or regulators' offices to** collect additional data needed for the completion of this task. During the data collection process, Geosyntec interviewed site personnel with regard to the nature and frequency of any issues encountered with the cover or LCRS and LDS operation (e.g., repair of cap erosion or replacement of flowmeters).

2.2 Site Data

2.2.1 Site Information

The site information, design details, LCRS and LDS flow data, and chemistry data presented in this study were obtained from engineering drawings, project specifications, as-built records, and/or operation records, supplemented with interviews with facility owner/operators, monitoring personnel, and/or regulatory agencies. Efforts were made to obtain as complete a record of data as possible, from completion of liner construction through the time of data collection. The presentation and structure of the information provided in this study purposefully mirror that of Appendix E in EPA (2002). In this way, this report is intended to serve as a limited extension of that study.

2.2.2 Leachate Flow

The leachate flow data from the LCRS and LDS drainage layers were collected from site records. For all landfills, the daily, weekly, monthly, or yearly data were normalized to gallons per acre per day (gpad) to provide a common unit for comparison of leachate flow for this study. Attempts were made to collect data from the date of unit closure (i.e., “time zero” for PCC) to the current date in order to obtain as complete a timeline of post-closure flow. Average and peak flow data are provided in full in Appendix II.

As described in Chapter 4, flow data were used to evaluate the trends between average LCRS flow rates and average annual rainfall for a given site. Consistent with EPA (2002), which this study seeks to complement, the data were first assessed using a methodology presented by Gross et al. (1990) using LCRS and LDS flow data to evaluate the performance of primary liner in terms of apparent leakage through the primary liner. The basic approach involved the evaluation of relative LDS to LCRS flow rate to quantify the hydraulic performance of primary liner system.

2.2.3 Leachate Chemistry

Leachate chemistry data for both the LCRS and LDS as reported in monthly, quarterly or annual reports were assembled for sites, as available. Data from 30 parameters were sought, where available. For consistency, these parameters mirrored those reported by EPA (2002). Major categories of parameters analyzed in this study included:

- pH;
- Specific conductance and total dissolved solids (TDS);
- Macro-indicators of leachate quality, including chemical oxygen demand (COD), biochemical oxygen demand (BOD), and total organic carbon (TOC);
- Major cations (calcium, magnesium, sodium, and potassium);
- Major anions (chloride, sulfate, and alkalinity);
- Trace metals, including arsenic, cadmium, chromium, lead, nickel; and
- Volatile organic compounds, including benzene, toluene, ethylbenzene, and xylene (BTEX).

The data were used to estimate the time that may be required for concentrations of constituents of interest in leachate to decrease to asymptotic levels during PCC (Section 6.2). As discussed in Section 5.3, leachate chemistry data were also used to quantify the portion of liquids comprising total LDS flow that should be attributable to primary liner leakage as opposed to other sources (i.e., by demonstrating a lack of hydraulic connection between the LCRS and LDS). In some cases, this allowed the apparent liner efficiency to be corrected based on relative concentrations of seven key cations and anions (particularly

chloride) as well as selected VOCs in the LCRS and LDS. Similar to the approach in EPA (2002), the presence of chemical constituents in the LDS was evaluated empirically (i.e., the concentrations of chemicals collected in the LDS were directly compared to concentrations of the same chemicals collected in LCRS). No fate and transport analysis were performed to account for attenuation of the LCRS chemicals migrating through the primary liner CCL. However, to minimize the effects of attenuation, the key chemical constituents were selected based on their high solubility in water, low octanol-water coefficient, high resistance to hydrolyzation, and high resistance to anaerobic biodegradation in soil.

The leachate chemistry database is too large and complex to meaningfully summarize here but is presented in full in Appendix III. It is noted that the database is limited in terms of its completeness and the duration of monitoring. Many key leachate constituents are poorly represented in the LCRS dataset (e.g., TDS, COD, and BOD), while LDS chemistry is not monitored at many sites. Further, site operators are only required to keep records for 3 years; as such, many older records are no longer available. If this lack of data exists across all other Subtitle C facilities (i.e., is not unique to the case studies), this represents an important limitation on assessing the long-term performance of Subtitle C containment systems.

2.2.4 Quality Assurance Project Plan

Collection and review of data for this study were performed in accordance with the quality assurance project plan (QAPP) developed for the Work Assignment. The QAPP was developed in accordance with **guidance provided in the EPA's National Risk Management Research Laboratory (NRMRL) quality assurance requirements for secondary data projects** (EPA, 2008). The QAPP was approved by EPA prior to the initiation of data gathering. The primary focus of the QAPP was to verify that the environmental and related data compiled for reference or use on this project are complete, accurate, and of the type, quantity, and quality required for their intended use.

In compiling information from secondary data sources for this report, every effort was made to identify and select data sources that have undergone peer and public review to varying degrees. Specific elements addressed by the QAPP include identifying the sources of secondary data and rationale for selecting the data sources used, presenting the hierarchy for data sources (Table 2-1), describing the review process and data quality criteria/metrics, discussing quality checks and procedures should errors be identified, and explaining how data will be managed, analyzed, and interpreted.

Table 2-1. Data quality assessment guide for source

Quality Ranking	Source
Highest	Federal, state and local government agencies
Second	Consultant reports for state and local government agencies
Third	Non-governmental organization (NGO) studies, peer-reviewed journal articles, and peer-reviewed conference proceedings
Fourth	Conference proceedings and other trade literature that are not peer-reviewed
Fifth	Individual estimates

With regard to data from the individual case studies presented in Chapter 3 and discussed in Chapters 4 to 6, a key focus of the QAPP was to ensure that the environmental and related data were complete, accurate, and of the type and quality required for their intended use. Potential data sources available for each site were reviewed to identify the level of quality assurance (QA) and quality control (QC) applied during collection and analysis of samples. Significant limitations on the use of data were documented prior to inclusion in this report in an effort to ensure that the data are appropriate for their intended use and representative of site conditions. Although it important to note that specific vetting of individual site data was not possible, data that were officially submitted to federal or state agencies were assumed to represent sources equivalent to the second tier in Table 2-1 (i.e., consultants reports for submission to state agencies or the relevant regulatory authority, for which data have undergone QA and QC procedures consistent with such submissions). These data were prioritized. Where data are included in the report that were received directly from the site operator or their consultants but that had not been officially submitted to the regulator, these sources are considered to represent fifth tier sources in the table. **Examples of this category of data include operators' anecdotal recollections regarding the locations and timing of localized cap repairs.** Use of such information is limited but included, where useful, to the case study discussions; in all cases, data limitations are clearly identified in the report text. The age of secondary data sources was also considered as a quality criterion per the scheme listed in Table 2-2.

Table 2-2. Data quality assessment guide for timeliness

Quality Ranking	Source
Highest	Data from sources dated 2010–2016
Second	Data from sources dated 2005–2009
Third	Data from sources dated 2000–2004
Fourth	Data from sources dated 1999 or prior

3. REVIEW OF CASE STUDY LANDFILLS

3.1 Overview

Nine doubled-lined Subtitle C landfills featuring one or more closed unit were selected as case studies. All data are blinded, with a single-letter alphabetic designation randomly assigned to each site. There are at least two case study sites from each of the four U.S. geographic regions (Figure 3-1). However, this study presents only a very small fraction of the total of over 1,500 hazardous waste sites closed Subtitle C sites nationwide. The nine sites do not represent a statistically significant number of sites to represent Subtitle C sites or to predict performance across different landfill design characteristics or climatic regions.

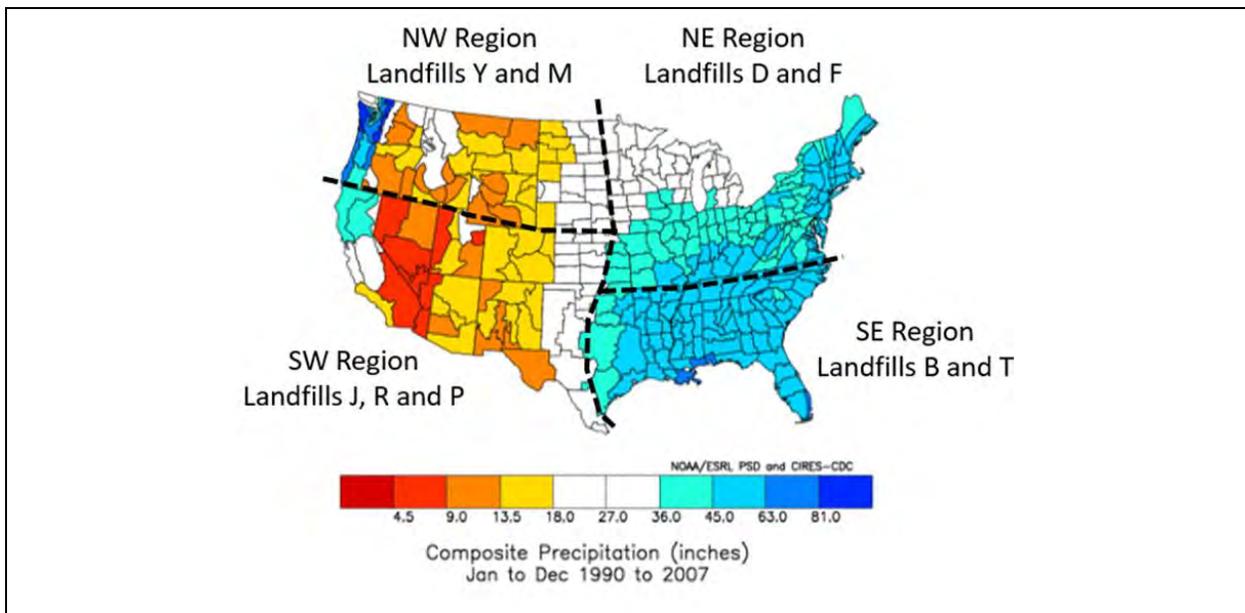


Figure 3-1. Distribution of landfill sites in U.S. geographical regions established for this study

The majority (7 of 9) of sites are commercial facilities that accepted hazardous waste from multiple sources. Two are industrial facilities that accepted waste from a single generator. One site (Landfill M) has a unit that is not formally closed but has been dormant under intermediate soil cover for several years. Facility information, including average annual rainfall, subsurface soil types, and nominal depth to groundwater below ground surface is summarized in Table 3-1. The nominal depth to groundwater affords an understanding of the separation distance between the base of the landfill and the water table, and thus the potential for the direct intersection between the landfill liner system and groundwater.

Table 3-1. General site information

Landfill designation	Geographic region	Average annual rainfall (inches) ⁽¹⁾	Average annual snowfall (inches) ⁽¹⁾	Nominal depth to groundwater (feet) ⁽²⁾	Subsurface soil type(s)
B	SE	47	--	<5	Claystone
T	SE	70	--	<5	Sandy silt and clay
J	SW	11	--	300	Clays and sandstone
R	SW	6.5	--	250	Sands and clays
P	SW	28	--	40	Clay
Y	NW	10	--	120	Gravelly sands to silty clays
M	NW	9.5	--	200	Clays
D	NE	42	17	10	Clay and gravelly sands
F	NE	40	60	90	Sands to silty loam

Notes:

1). The average annual rainfall and snowfall were computed based on nearest weather station data to the site.

2). The nominal depth to groundwater was based on design and/or groundwater monitoring reports made available by the operator.

Overall, the nine landfills yielded 45 individual double-lined closed units for investigation in this study. A database was developed based on reports made available by the operators that include design information and monitoring data collected at each individual unit. The data collected are summarized in Appendix II:

- General landfill construction information (Table II-1);
- Landfill liner system design details (Table II-2);
- Final cover system design details (Table II-3);
- LCRS and LDS annual average (Tables II-4A-C) and annual peak (Tables II-5A-C) flow, normalized as gallons per acre per day (gpad); and
- LCRS and LDS chemistry data (Table II-6).

Available LCRS and LDS flow and chemistry data are presented in full in Appendices III and IV, respectively.

3.2 General Description of Case Study Facilities

3.2.1 Landfill Construction and Operation

Overall, 45 individual double-lined units at nine separate landfill facilities are included in this study. Individual units at case study landfills ranged in size over an order of magnitude from 1.4 acres to 11.3 acres, although most units were less than 5 acres in area. The oldest units in the study have been closed for over 29 years, while the newest are only 6 years into a PCC program (the study unit at Landfill M is not closed with a final cover, but has been inactive with waste at final grade for 4 years). Individual units featured various LCRS and LDS flow measuring devices (Table II-1). In most cases, liquids in the LCRS and LDS were each drained to a single sump serving the entire unit; however, a few units featured

multiple sumps each equipped with individual flow measuring devices. The thickness of waste in place above a liner ranged from 40 feet to 110 feet, although the thickness of waste in most units was in the range of 70 to 80 feet. All liner and cover construction at the study units was performed under a construction quality assurance (COA) program.

3.2.2 Liner System and Cover System Design

Amongst the 45 case study units, 11 different liner system designs and a further 11 different cover system designs are represented. For discussion in this report, these are combined into 13 unique containment system design configurations featuring commonality through the entire thickness of the unit from cover to liner (Table 3-2).

Table 3-2. Commonality of liner system, LCRS and LDS drainage system, and cover system for different case study design configurations

Design Configuration	Landfill	Sub-group	Number of units	Primary Liner		LCRS		LDS		Cover system				
				Composite (GM/CCL)	Geomembrane	Granular soil ⁽¹⁾	Geosynthetic (GC or GN)	Granular soil ⁽¹⁾	Geosynthetic (GC or GN)	Composite (GM/CCL)	Composite (GM/GCL)	Composite (CCL/GM)	Geomembrane	Soil ⁽¹⁾
1	B	B-1 to B-6	6	✓		✓		✓				✓		
2		B-7 and B-8	2	✓		✓		✓		✓				
3	T	T-1 to T-18	18	✓		✓		✓		✓				
4	J	J-1 to J-3	3	✓		✓		✓	✓				✓	
5	R	R-1	1	✓			✓	✓	✓					
6		R-2 to R-5	4	✓			✓	✓	✓					
7	P	P-1	1		✓		✓		✓	✓				
8		P-2 to P-4	3		✓		✓		✓		✓			
9	Y	Y-1	1		✓	✓		✓			✓			
10		Y-2 and Y-3	2		✓	✓		✓	✓		✓			
11	M	M-1	1	✓		✓	✓		✓					✓
12	D	D-1 and D-2	2		✓		✓		✓		✓			
13	F	F-1	1		✓		✓		✓		✓			
TOTAL			45	35	10	32	13	27	18	27	8	6	3	1

Note 1). The granular soil is either coarse sand or gravel, Soil is general fill.

Inspection of Table 3-2 reveals the following commonalities and differences in containment system design traits amongst the case study units:

- Liner system: Most of the study units had a GM/CCL composite primary liner system (80%), with seven design configurations (1–6 plus 11) featuring this liner design. A single geomembrane primary liner was featured in 20% of study units (configurations 7–

10 and 12–13). No case study units were constructed having a GM/GCL composite primary liner, although one site (Landfill D) utilizes a GCL in the secondary liner.

- LCRS: Six design configurations (1–4 and 9–10), comprising 80% of the study units, had 12 inches of sand as the LCRS drainage layer while the other seven configurations (comprising 20% of study units) had a geocomposite (GC) or geonet (GN) drainage layer.
- LDS: A 12-inch sand drainage layer was in 60% of the study units (four design configurations, 1–3 and 9), while the remaining 40% (representing nine configurations) had a GC or GN drainage layer.
- Cover system: A GM/CCL composite cover design was used for 59% of the study units (six design configurations), while 18% (four configurations) were constructed having a GM/GCL composite cover system. Six study units (14% of the total, all design configuration 1 at Landfill B) were constructed in reverse with a CCL/GM cap. Two special-case cover design types exist:
 - Landfill J had an MSW overfill landfill constructed above it and thus its cover system acts as the liner system for the overlying landfill (design type 4), and
 - Landfill M currently has intermediate cover soil in place (design type 11) although the approved final cover design is an all-soil evapotranspiration cover system.

Landfill B

The site is located in the SE region and is fully closed with no active receipt of waste for treatment, storage, or disposal. The mean annual precipitation at the site is 47 inches with minimal depth to groundwater. The subsurface soil mainly consists of claystone with a few sand lenses. The eight study units at this landfill (B-1 to B-8) vary in size from 3.7 to 8.8 acres. Waste thicknesses above the liner range from about 45 feet to 110 feet (average 75 feet).

The liner and cover system details are as follows:

- Primary Liner (all): 80-mil HDPE GM overlying a 60-in CCL
- LCRS Drainage Layer (all): GT overlying a 12-in thick sand layer
- Secondary Liner (all): 80-mil HDPE GM overlying a 36-in CCL
- LDS Drainage Layer (all): GT overlying a 12-in thick sand layer
- Cover (B-1 to B-6): 24-in CCL overlying a 30-mil HDPE GM
- Cover (B-7 and B-8): 60-mil HDPE GM overlying a 24-in CCL
- Cover Drainage/Protective Layers (all): 18-in sand layer underlying 6-in protective soil

Note that the typical composite barrier configuration (geomembrane overlying soil layer) is reversed in the cover system for units B-1 to B-6, where the GM is placed under the CCL.

This has important ramifications on cover system performance and leachate flow as discussed in Chapter 4. The configurations of the liner and cover systems for units at Landfill B are shown in Figure 3-2.

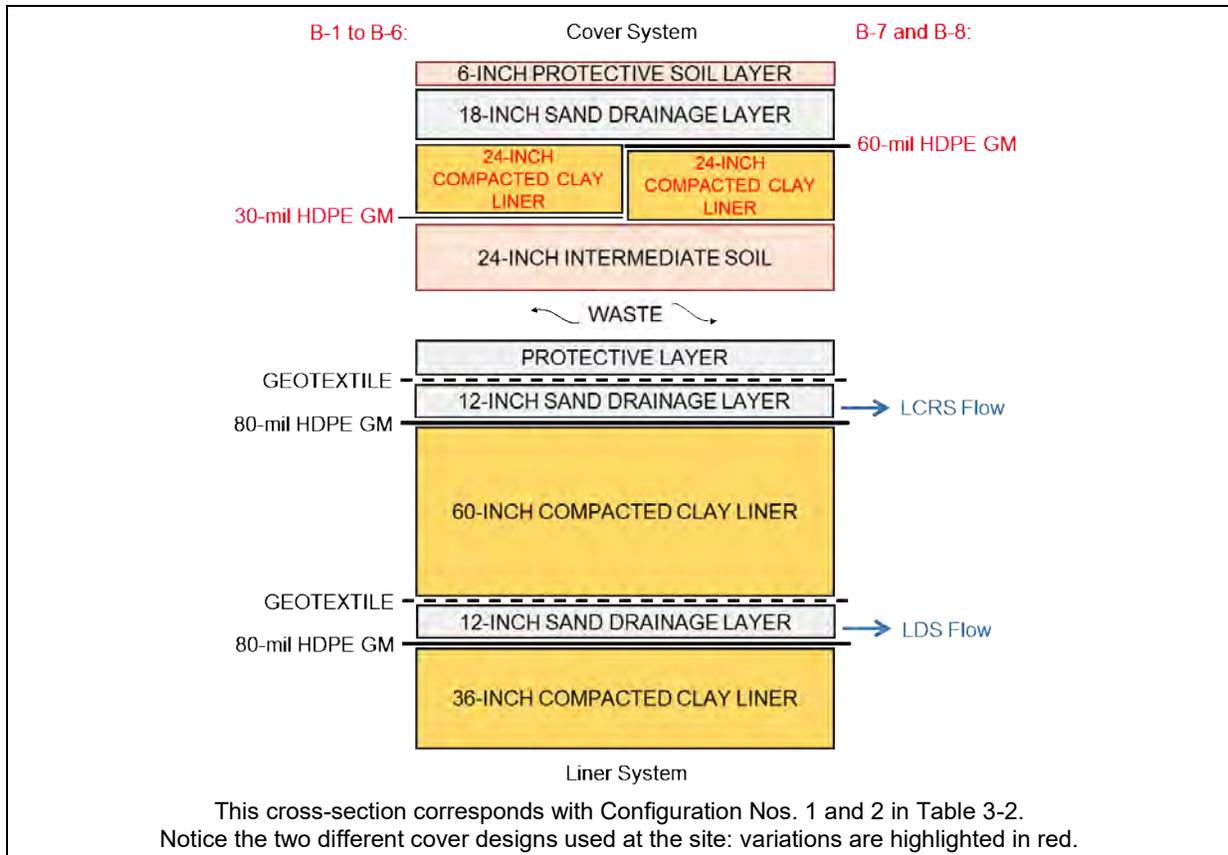


Figure 3-2. Liner and cover system cross-sections for Landfill B

Landfill T

The site is located in the SE region and is currently operational (i.e., some active units are accepting waste). The mean annual precipitation at the site is 70 inches with minimal depth to groundwater below ground level, which implies that groundwater is in contact with the liner system. The subsurface soil mainly consists of sandy silt and clay. Eighteen closed units (T-1 to T-18) are part of this study.

The closed units at Landfill T range in size from 1.4 to 4.2 acres with a waste thickness of 70 to 80 feet. All units have the same liner and cover system design, details of which are as follows (Figure 3-3):

- Primary Liner: 60-mil HDPE GM overlying a 36-in CCL
- LCRS Drainage Layer: 12-in thick sand layer overlain by 12-in protective soil layer with permeability greater than 1×10^{-2} cm/s
- Secondary Liner: 60-mil HDPE GM overlying a 36-in CCL
- LDS Drainage Layer: 12-in thick sand layer
- Cover: 60-mil HDPE GM overlying a 24-in CCL
- Cover Drainage/Protective Layers: GN underlying 24-in protective soil

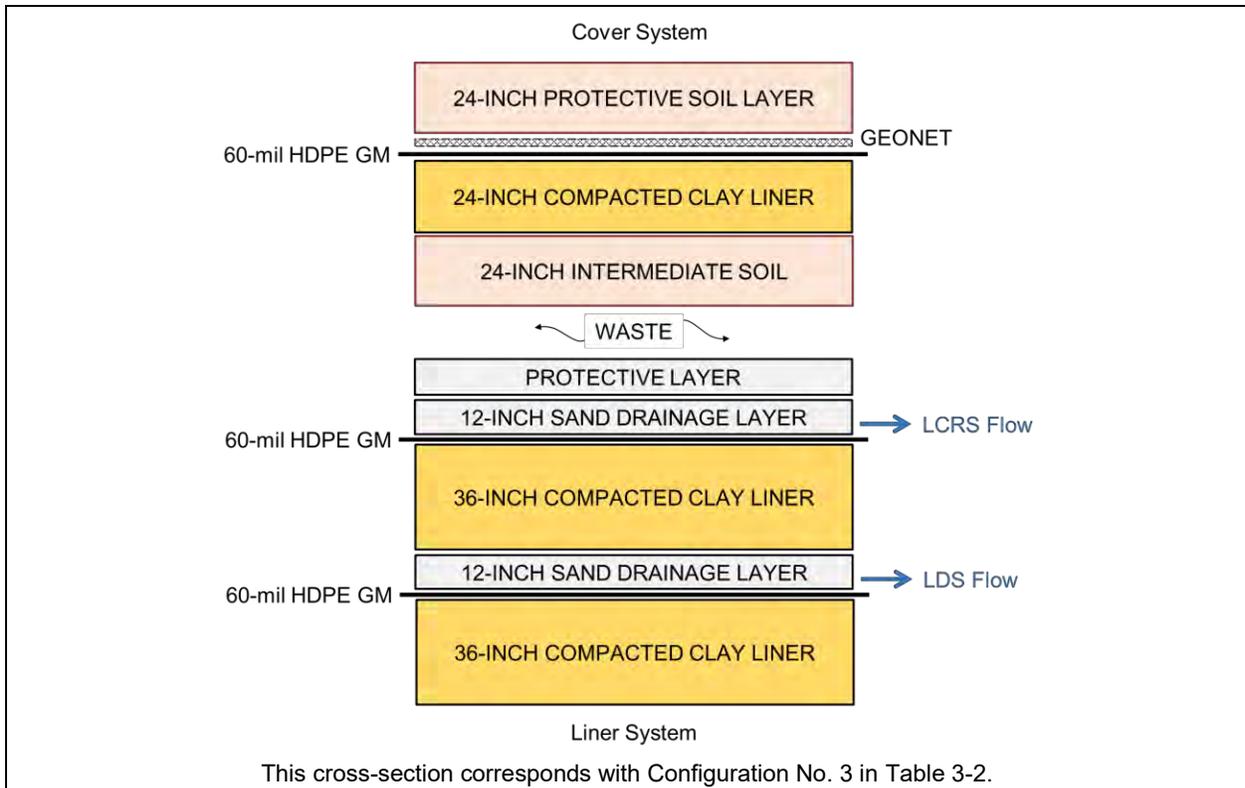


Figure 3-3. Liner and cover system cross-sections for Landfill T

Landfill J

This site is located in the SW region and is currently operational. The region is arid, with mean annual precipitation at the site only 11 inches. The depth to groundwater is 300 feet. Subsurface soils mainly consist of clay and sandstone. Three conjoined landfill units (J-1 to J-3) comprise a single closed landfill unit that is part of this study. These units have an MSW overfill landfill constructed above them such that the liner system for the overlying MSW landfill system is integrated with the cover system for Landfill J. The three units are all about 10 acres in area with waste thickness above the liner of 90 feet.

All units have same liner and cover system design, details of which are as follows (Figure 3-4):

- Primary Liner: 60-mil HDPE GM overlying an 18-in CCL
- LCRS Drainage Layer: 12-in thick sand layer overlain by 24-in soil layer with geotextile
- Secondary Liner: 60-mil HDPE GM overlying a 36-in CCL
- LDS Drainage Layer: 12-in thick gravel layer overlying a GN
- Cover: 60-mil HDPE GM overlying a 24-in foundation layer
- Cover Drainage/Protective Layers: GC underlying 24-in protective/drainage soil layer which also acts as LCRS for an overlying MSW cell

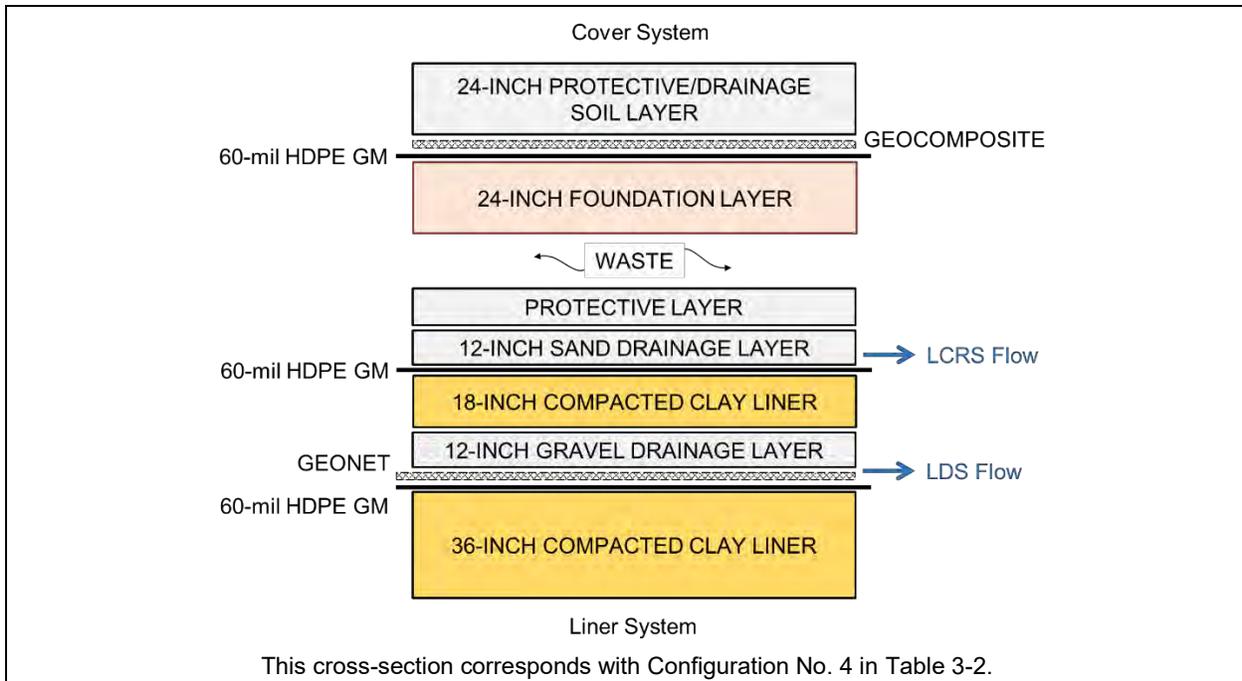


Figure 3-4. Liner and cover system cross-sections for Landfill J

Landfill R

The site is located in the SW region and is currently operational. The site is located in an arid region with mean annual precipitation at the site of 6.5 inches and depth to groundwater of 250 feet. The subsurface soils mainly consist of sand and clay. Five landfill units (R-1 to R-5) are part of this study. The units vary in size from 5.0 to 7.6 acres with the maximum thickness of waste in R-1 being about 70 feet and in R-2 to R-5 being about 100 feet. All units have a similar cover system; however, R-1 has a different liner system design to the other units.

The liner and cover system details are as follows (Figure 3-5):

- Primary Liner (R-1): 40-mil PVC GM overlying a 36-in CCL
- Primary Liner (R-2 to R-5): 80-mil HDPE GM overlying a 36-in CCL
- LCRS Drainage Layer (R-1): GN overlay by 18-in thick protective soil layer
- LCRS Drainage Layer (R-2 to R-5): GC
- Secondary Liner (R-1): 40-mil PVC GM overlying a 36-in CCL
- Secondary Liner (R-2 to R-5): 80-mil HDPE GM overlying a 36-in CCL
- LDS Drainage Layer (R-1): GN
- LDS Drainage Layer (R-2 to R-5): GC
- Cover (all): 80-mil HDPE GM overlying a 24-in CCL
- Cover Drainage/Protective Layers (all): GT underlying 24-in protective soil

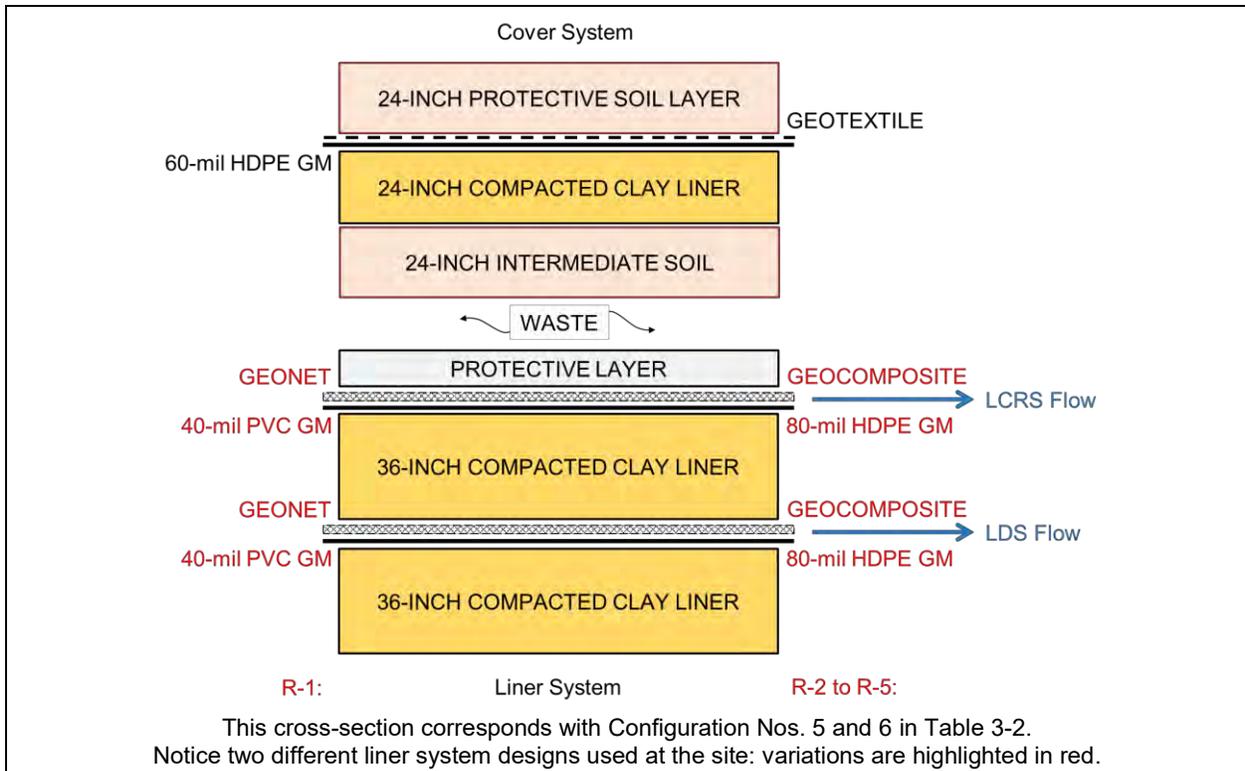


Figure 3-5. Liner and cover system cross-sections for Landfill R

Landfill P

The site is located in the SW region and is currently operational. The mean annual precipitation at the site is 28 inches and the average depth to groundwater is 40 feet below ground level. The subsurface soil mainly consists of clay. Four landfill units (P-1 to P-4) are part of this study, each with an area of 10 acres and a maximum thickness of waste above the liner of 42 feet.

The liner and cover system details are as follows (Figure 3-6):

- Primary Liner (all): 60-mil HDPE GM
- LCRS Drainage Layer (all): GN overlain by 24-in protective cover layer
- Secondary Liner (all): 80-mil HDPE GM overlying a 36-in CCL
- Lower LDS Drainage Layer (all): GN overlain by intermediate liner (60-mil HDPE GM)
- Upper LDS Drainage Layer (all): GN overlain by 24-in. the protective layer and underlain by intermediate liner
- Cover (P-1): 60-mil HDPE GM overlying a 24-in CCL
- Cover (P-2 to P-4): 60-mil HDPE GM overlying a GCL
- Cover Drainage/Protective Layers (all): GN underlying 36-in protective soil

This site is unique in that it has an intermediate liner system situated between the primary and secondary liners with an overlying GN drainage layer providing separate recovery and

recording of liquid flows. For ease of identification on the figure and throughout this report, the upper LDS drainage layer is denoted "LDS1" while the lower LDS drainage layer is denoted "LDS2." In terms of assessing the performance of the primary liner, the sum of flows in LDS1 and LDS2 is used to compare to flow in the LCRS.

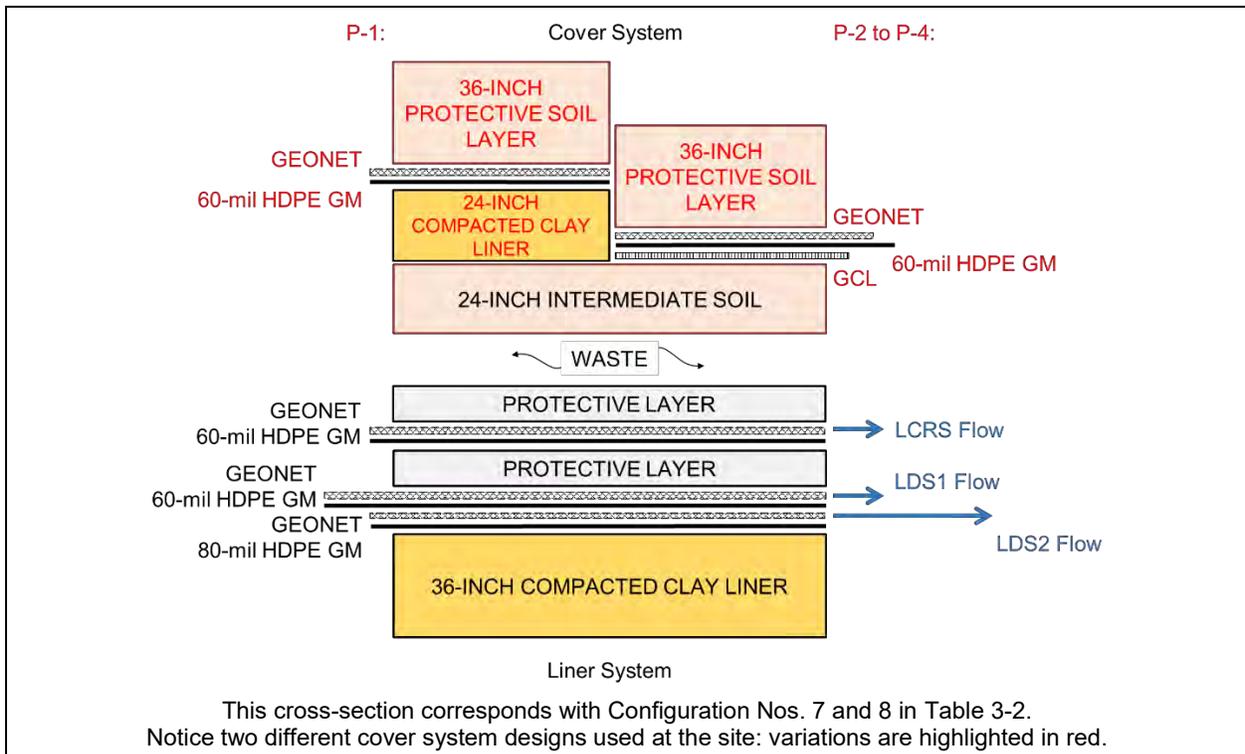


Figure 3-6. Liner and cover system cross-sections for Landfill P

Landfill Y

The site is located in the NW region and is currently operational. Mean annual precipitation at the site is 10 inches and depth to groundwater is 120 feet below ground level. Subsurface soils mainly consist of gravelly sand and silty clay. Three landfill units (Y-1 to Y-3) are part of this study, all of which have similar dimensions with an average area of 1.75 acres and maximum waste thickness of about 55 feet. All units have the same cover system; however, Y-1 has a different liner system from the other two units. The liner and cover system details are as follows (Figure 3-7):

- Primary Liner (Y-1): 60-mil HDPE GM
- Primary Liner (Y-2 and Y-3): 80-mil HDPE GM
- LCRS Drainage Layer (all): 12-in thick sand layer overlain by 6-in soil layer with geotextile
- Secondary Liner (Y-1): 40-mil HDPE GM overlying a 36-in CCL
- Secondary Liner (Y-2 and Y-3): 60-mil HDPE GM overlying a 36-in CCL

- LDS Drainage Layer (Y-1): 12-in thick sand layer
- LDS Drainage Layer (Y-2 and Y-3): 12-in thick sand layer overlying a GC
- Cover (all): 40-mil HDPE GM overlying a GCL
- Cover Drainage/Protective Layers (all): GC underlying 30-in protective soil

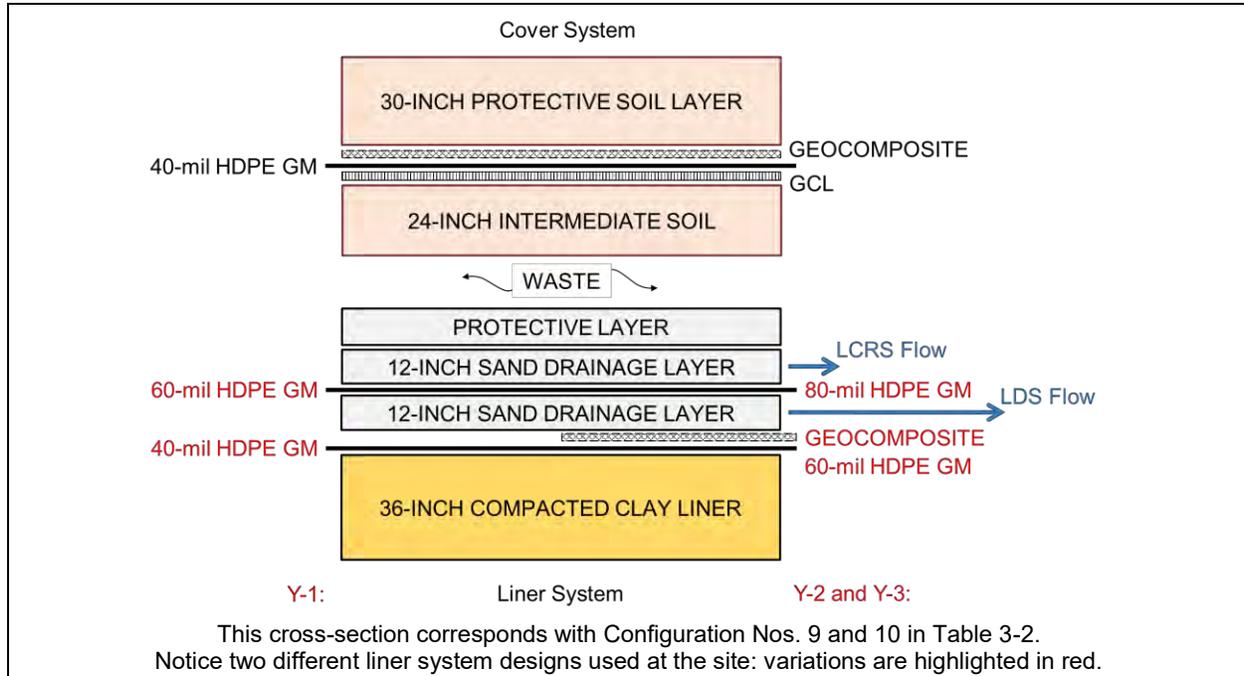


Figure 3-7. Liner and cover system cross-sections for Landfill Y

Landfill M

The site is located in the NW region and is currently operational. The mean annual precipitation at the site is 9.5 inches and depth to groundwater is 100 feet below ground level. The subsurface soil mainly consists of clay. A single landfill unit (M-1) is included in this study. The unit has an area of 9 acres with a maximum thickness of waste of 110 feet. The liner and cover system details for Landfill M are as follows (Figure 3-8):

- Primary Liner: 60-mil HDPE GM overlying an 18-in CCL
- LCRS Drainage Layer: 12-in thick sand layer with a GC overlain by 12-inch surface course material as a protective layer
- Secondary Liner: 60-mil HDPE GM overlying a 36-in CCL
- LDS Drainage Layer: GN
- Cover: Intermediate cover only, comprising 18-in select soil layer

The approved final cover design for the landfill consists of an evapotranspiration cover comprising a 3-foot soil layer. The waste in M-1 has been at final grades with the intermediate cover in place for 4 years.

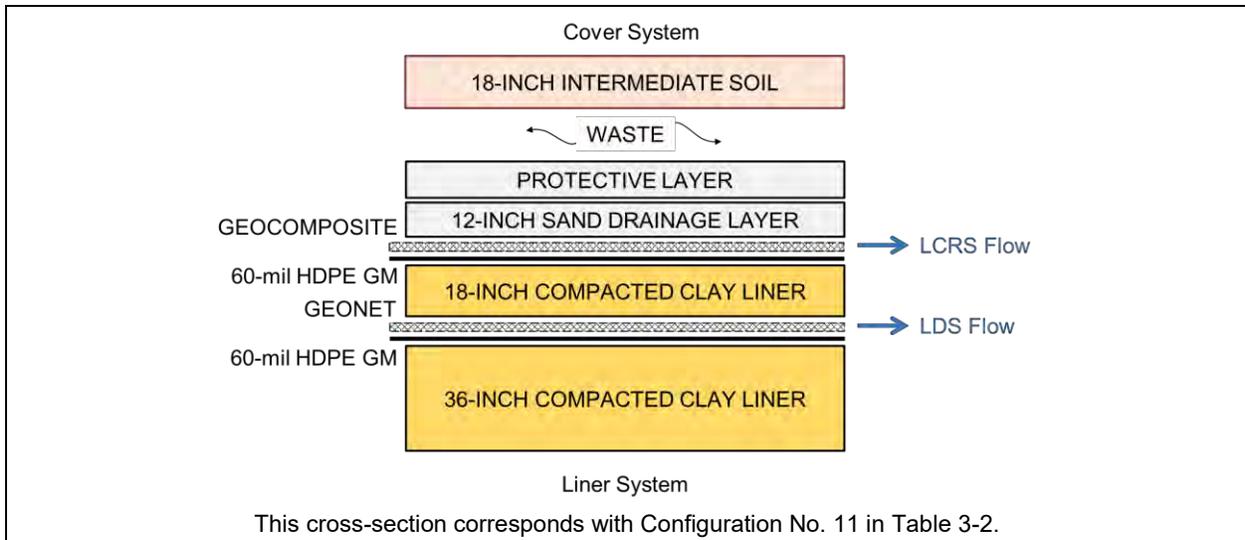


Figure 3-8. Liner and cover system cross-sections for Landfill M

Landfill D

Landfill D is located in the NE region and comprises an on-site disposal cell (OSDC) for hazardous waste generated from the closure of an industrial facility. The mean annual precipitation at the site is 40 inches (annual average snowfall is 17 inches) and groundwater is shallow at only 10 feet below ground level. The subsurface soils mainly consist of sands to silty loam. Two units (D-1 and D-2) are part of this study, D-1 having an area of 3.2 acres and D-2 an area of 5.8 acres. The maximum thickness of waste is 50 feet.

The liner and cover system details are identical for both units (Figure 3-9):

- Primary Liner: 60-mil HDPE GM
- LCRS Drainage Layer: GC overlain by 12-in protective soil layer with permeability greater than 1×10^{-3} cm/s
- Secondary Liner: 60-mil HDPE GM overlying a GCL above a 12-in foundation layer
- LDS Drainage Layer: GC
- Cover: 40-mil LLDPE GM overlying a GCL
- Cover Drainage/Protective Layers: GC underlying 24-in protective soil

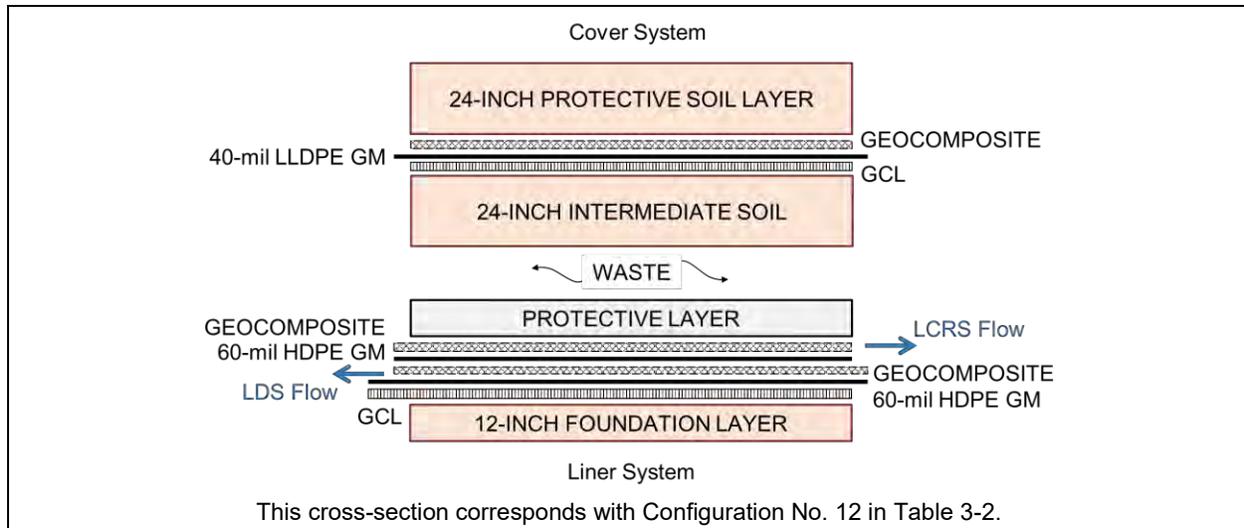


Figure 3-9. Liner and cover system cross-sections for Landfill D

Landfill F

The site is located in the NE region and is used for disposal of hazardous waste generated at an operational industrial facility. The mean annual precipitation at the site is 40 inches (annual average snowfall is 60 inches) and the average depth to groundwater is 90 feet below the ground surface. Subsurface soils mainly consist of sands to silty loam. A single landfill unit (F-1) is included in this study, with a liner area of 6 acres and a maximum thickness of waste of 50 feet.

The liner and cover system details (Figure 3-10) for F-1 are as follows:

- Primary Liner: 80-mil HDPE GM
- LCRS Drainage Layer: GN
- Secondary Liner: 80-mil HDPE GM overlying 36-in CCL
- LDS Drainage Layer: GN
- Cover: 40-mil LLDPE GM overlying a GCL
- Cover Drainage/Protective Layers: 12-in sand layer underlying 12-in protective soil

3.3 Post-Closure Monitoring and Maintenance

3.3.1 Leachate Management

The LCRS and LDS from each landfill unit drain to low points (i.e., sumps) on the primary and secondary liners, respectively, from where liquids are removed using pumps and side slope risers. Dedicated LCRS and LDS sumps are isolated from each other. The volume of liquids collected in each sump is recorded using a variety of devices, including automated accumulating flowmeters or periodic pumping based on liquid height exceeding an action threshold. To be included in the study, liquid flows in the LCRS and LDS had to be measured

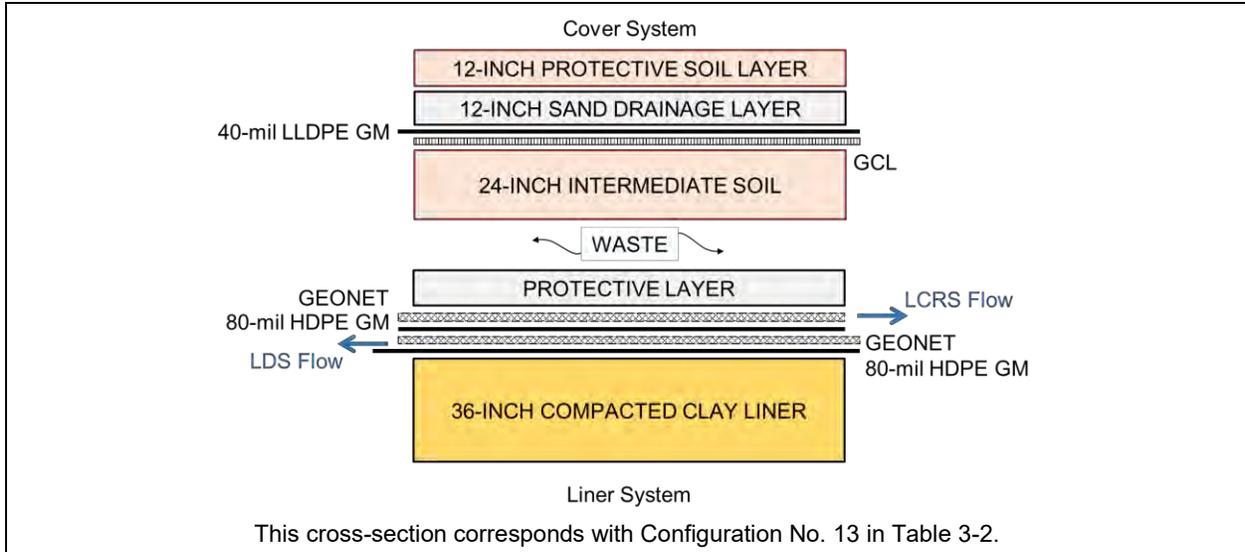


Figure 3-10. Liner and cover system cross-sections for Landfill F individually per landfill unit. In most cases, each landfill unit featured only one LCRS and LDS sump. Where a landfill unit had more than one sump, data were combined to represent total flow in the LCRS/LDS.

Leachate Flow

Leachate flow data from the LCRS and LDS drainage layers were collected from operators' site records. These were provided in the same format in which they are submitted in reports to regulators, and are assumed to have undergone QA and QC checks consistent with such submissions. The leachate flow database is included as Appendix III.

For each landfill unit, the daily, weekly, monthly, or yearly data were normalized to annual average and peak flow in terms of gallons per acre per day (gpad) to provide a common unit for comparison of leachate flow for this study. Attempts were made to collect data from the date of closure (i.e., time zero for PCC) through to the current time in order to obtain a complete timeline of post-closure flow from each unit. Average and peak flow data along with the sampling frequency and a total number of data points available from each unit are summarized in Tables II-4 and II-5 in Appendix II.

As described in Chapters 4 and 5, flow data were used to evaluate trends between average LCRS flow rates and average annual rainfall for a given site. Consistent with EPA (2002), which this study seeks to complement, the LCRS and LDS data were also comparatively assessed to evaluate the hydraulic performance of the primary liner in terms of apparent leakage.

Leachate Chemistry

Leachate chemistry data for both the LCRS and LDS as reported in monthly, quarterly or annual reports were assembled **from operators'** site records, as available. Data from 30 parameters were sought, where available; these parameters mirrored those reported by EPA (2002). Major categories of parameters targeted in this study included:

- pH;
- Specific conductance and TDS;
- Macro-indicators of leachate quality, including COD, BOD, and TOC;
- Major cations (calcium, magnesium, sodium, and potassium);
- Major anions (chloride, sulfate, and alkalinity);
- Trace metals, including arsenic, cadmium, chromium, lead, nickel; and
- Volatile organic compounds, including BTEX.

Leachate chemistry data from the LCRS and LDS drainage layers were collected from **operators' site records. These were provided in the same format in which they are** submitted in reports to regulators or to meet influent monitoring requirements of wastewater treatment facilities, and are assumed to have undergone QA and QC checks consistent with such submissions. The leachate chemistry database is too large and complex to meaningfully summarize in this report; however, the availability of leachate chemistry at each of the nine case study landfills is provided as Table II-7 in Appendix II, with the full database included as Appendix IV. Leachate data are discussed in Section 5.1 in the context of correcting apparent liner efficiency calculations and in Section 6.2 in terms of temporal trends observed in leachate quality parameters. Where possible, the latter serves to estimate the time that may be required for concentrations of constituents of interest in leachate to decrease to asymptotic levels during PCC.

It is noted that the leachate chemistry database is limited in terms of its completeness and the duration of monitoring although it is **important to acknowledge that "completeness" in** this context refers to the availability of the full suite of 30 parameters targeted in this study and not to data requirements specified for compliance. Many targeted leachate constituents are poorly represented in the LCRS dataset, while LDS chemistry is not monitored at all at many sites. In the latter case, it is noted that three sites (Landfills R, Y, and M) have zero flow in the LDS, which negates the ability to collect samples for analysis. In the context of this data assessment, therefore, this should not be construed as a data gap. An issue of importance identified in the process of collecting leachate chemistry data for this study is that many site operators reported only being required to keep records for 3 years, so older data are no longer available. As there are no specific requirements for monitoring and

retaining records of leachate quality under 40 CFR 264, this may reflect state-specific rules or site-specific agreements with receiving facilities for leachate treatment and disposal. Whatever the reason, if this lack of data at the case study landfills is representative of the majority of Subtitle C facilities, it will be an important limitation on assessing the long-term performance of Subtitle C containment systems and potential modifications to existing PCC programs.

Leachate Treatment and Disposal

The manner in which leachate from the case study landfills is treated and disposed of varies between the sites. For example:

- At Landfills B and P, leachate collected from the closed units is processed at an on-site leachate treatment plant that evaporates most of the effluent. The brine residuals from the evaporation process are sent off-site for incineration or disposal.
- Landfill J uses one of three options to dispose of leachate depending on constituents determined from chemical analysis of leachate, including evaporation in on-site ponds, stabilization prior to being disposed of in the landfill, or off-site transfer for treatment (VOC removal) and disposal.
- At Landfills R, T, and F, leachate is transported off-site for treatment and disposal.
- At Landfill D, leachate collected from D-1 is managed at an on-site water treatment plant, while leachate from D-2 is shipped off-site for disposal.
- Landfills Y and M dispose of leachate on-site using evaporation ponds, although some leachate is also sent to a wastewater treatment plant.

It is noted that only anecdotal information from site operators was provided with regard to compiling the above list. Operators did not complain of any significant problems related to leachate treatment and disposal. The strong focus on containment and reducing leachate flow volumes is likely a contributing factor. Operators interviewed for this study indicated that actual leachate disposal costs were generally in line with expectations.

3.3.2 Cover Monitoring and Maintenance

As part of this study, a small number of site operators were informally interviewed regarding site-specific conditions and the extent of cover monitoring and maintenance activities being conducted. In particular, whether the level of maintenance has increased or decreased noticeably over time, what the greatest challenges have been (e.g., extreme weather events) in maintaining and monitoring the containment systems, and noticeable trends in the type of recurring issues and whether these can be directly related to causal effects (e.g., leakage/scouring around the boot between cap geosynthetics and headwalls at stormwater drainage swales at the anchor trench, erosion and infiltration around appurtenances in the cover).

This small number of operators indicated that the general level of cover monitoring and maintenance performed and the costs associated with these activities had been relatively steady over the years, with higher costs associated with repairs needed during the initial years of PCC before cover vegetation was fully established and the cover stabilized. The biggest reported challenge has been erosion control and protection of the cover specifically due to high rainfall following a long spell of dry weather. This led one operator to focus on the timely and effective seeding of newer caps and to limit side slopes to 3H:1V to help prevent erosion issues. Cover penetrations at Subtitle C landfills are generally more limited than at Subtitle D landfills, as it is often not necessary to install LFG collection wells. Most penetrations were for vertical riser pipes at LCRS and LDS sumps. Maintenance and localized repair of these penetrations were not reported as being a significant issue. The low levels of biodegradable material disposed of within Subtitle C landfills relative to Subtitle D landfills also limit issues with the differential settlement of the cover.

3.3.3 *General Status of Post-Closure Care*

As part of this study, a small number of site operators were informally interviewed regarding the general status of the closed units at their facility and monitoring and maintenance conducted under the PCC program. Questions posed included whether progress has been made in improving the stability of the cover system, what the costs associated with PCC have been and how these compare with expectations, what their anticipation is for the total duration of PCC, and whether any steps have been employed or considered for implementing any sort of passive control systems that could potentially reduce the long-term PCC burden (e.g., alternative covers or on-site engineered wetlands). Operators were also asked about the adequacy of financial assurance (FA) requirements for PCC. Finally, operators were asked what challenges to innovation and creativity they face with regard to optimizing PCC, and how EPA could help incentivize action in this area.

Responses from operators are summarized below. It is noted that the information provided below is subjective in nature and should be taken as such:

- Closed units at the case study sites have mostly performed well during PCC with steady leachate generation rates in line with modeled predictions. Where significant deviations from expected flow rates have occurred, these have been traced to minor issues with the containment system, notably the anchor trench tie-in between the liner and cover systems that, once repaired, have rapidly returned to the expected level. Leachate chemistry has not deviated from acceptance criteria for treatment and disposal, such that leachate management has not been an issue or represented a higher-than-expected cost.
- Non-routine operation and maintenance (O&M) issues were mostly related to equipment repair and replacement due to clogging of pumps and flow meters. The latter is

particularly important as it has caused a number of sites concerns about apparently high or fluctuating leachate generation rates that were ultimately traced to faulty meters.

- The highest costs (or at least, most notably high costs from the perspective of operators) are related to PCC activities associated with a third-party engineer providing routine facility inspections and technicians conducting groundwater monitoring in the preparation of reports to be submitted to the overseeing agency. One operator opined that climate change has seemingly caused wider seasonal fluctuations in groundwater levels in recent years relative to historical data, but that has not significantly impacted the provision of PCC at the site to date.
- Most operators appear to assume that PCC will be conducted for 30 years with monitoring and maintenance activities being progressively reduced or scaled back based on facility performance before eventually being terminated. It is not clear whether the expectation is that the 30-year period applies to the time until the scaling back of activities would commence or the time at which activities will be terminated.
- In general, operators consider their FA provisions to be adequate for the assumed PCC program at the site but understand that the funds cannot last in perpetuity. As such, they suggested that some certainty or guidance on the process for scaling back and elimination of PCC activities is needed. Also, some operators noted that original FA estimates were done 20+ years ago and that adjustment factors for increases in costs and the complexity of PCC activities (e.g., as a result of improved analytical techniques and survey methods) may be needed.
- In general, operators have looked to remain with the prescriptive standards for design and operation of Subtitle C landfills. This is likely in the interest of improving cost certainty, as the perception is that alternative designs are riskier. Nevertheless, one operator is evaluating on-site leachate treatment using novel biological treatment technologies, and two operators are trying to permit an all-soil evapotranspiration (ET) final cover. It is noted that one case study site (Landfill M) already has an ET cover permitted.
- With regard to incentives that EPA could provide, one operator called for the option of recirculating leachate collected from closed units at their facility into active units. Another operator would like EPA to provide guidance on permitting ET covers at hazardous waste landfills and to provide guidance on how to scale back and end PCC activities based on an assessment of performance data.

4. ANALYSIS OF LEACHATE FLOW DATA

4.1 Temporal Trends in Leachate Flow Rates in the LCRS and LDS

This section focuses on understanding temporal trends in LCRS and LDS flow data. All the landfill units in the study were operated with the strategy of liquid removal from both LCRS and LDS in order to minimize potential head buildup and leakage through the primary and secondary liners. In accordance with 40 CFR §264.301(a)(2), the buildup of hydraulic head on the primary liner must be limited to less than 12 inches. Throughout this chapter, LCRS and LDS flows are normalized to gallons per acre per day (gpad) to facilitate comparison of results between different sites. Because the potential for leachate generation is closely tied to precipitation levels, the distinction **is made between sites in “wet” climates, at which average annual precipitation exceeds 25 inches and “dry” sites at which average annual precipitation falls well below this level.** Use of 25 inches of annual rainfall as the distinction **between wet and dry landfill conditions is consistent with the EPA’s approach to assigning decay factors for methane generation modeling at Subtitle D landfills (EPA, 1995).** As such, the discussion is grouped between four wet sites (Landfills B, T, D, and F), four dry sites (Landfills J, R, Y, and M), and one in-between (cusp) site with unique liner design (Landfill P). Within each category, data are presented for groups of units comprising each of the 13 design configurations listed in Table 3-2.

4.1.1 Wet Sites – Landfills B, T, D, and F

Landfill B receives an average of 47 inches of rainfall annually while Landfill T receives an average of 70 inches. Both Landfills B and T are in hot and wet climates with groundwater at or near the ground surface. The climate at Landfills D and F is also wet but colder: these landfills receive 42 and 40 inches of average annual rainfall and 17 and 60 inches of average annual snowfall, respectively. Groundwater is shallow at Landfill D (10 feet) and deep at Landfill F (90 feet).

Landfill B, Units B-1 to B-6 (Design Configuration No. 1)

Available leachate data for these units includes monthly LCRS and LDS flow volumes for up to 29 years of PCC (Figure 4-1, note the difference in y-axis scale between the two graphs).

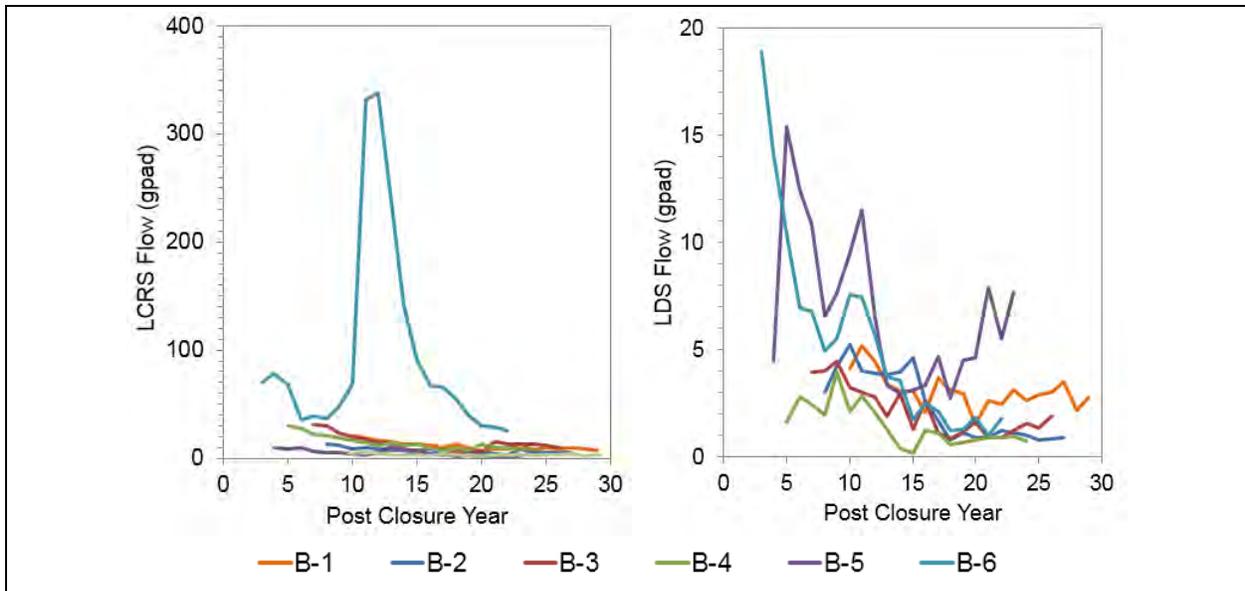


Figure 4-1. Annual average LCRS and LDS flow, B-1 to B-6

Of interest to this design is the fact that the typical composite barrier configuration (geomembrane overlying soil layer) is reversed in the cover system for units B-1 to B-6, where the GM is placed under the CCL. With the exception of B-6, LCRS flow rates for all units are below 100 gpad, with most below 10 gpad. Trends have been predictably downward or steady. Unit B-6 is not contiguous with B-1 through B-5 but forms part of a separate landfill mound with units B-7 and B-8. The sudden spike in LCRS flow in B-6 after 10 years was mainly attributed by the site operator to overtopping of an operational berm in B-8. In support of this, after the berm was repaired the LCRS flow rate in B-6 has trended down to similar levels recorded for the other five units.

Flow rates recorded in the LDS appear more erratic than in the LCRS, although this may simply reflect the low volumes of liquids recovered in the LDS. This behavior is most notable in B-5, although no specific causal factors were identified. With the exception of B-5, trends are steady or declining with most units exhibiting LDS flow below 5 gpad in recent years.

Landfill B, Units B-7 and B-8 (Design Configuration No. 2)

Available leachate data for these two units includes monthly LCRS and LDS flow volumes for up to 17 years of PCC. Annual average LCRS and LDS flow rates are presented in Figure 4-2 (note the difference in y-axis scale between the two graphs). As noted above, units B-7 and B-8 form part of a separate landfill mound to the other six closed units at Landfill B.

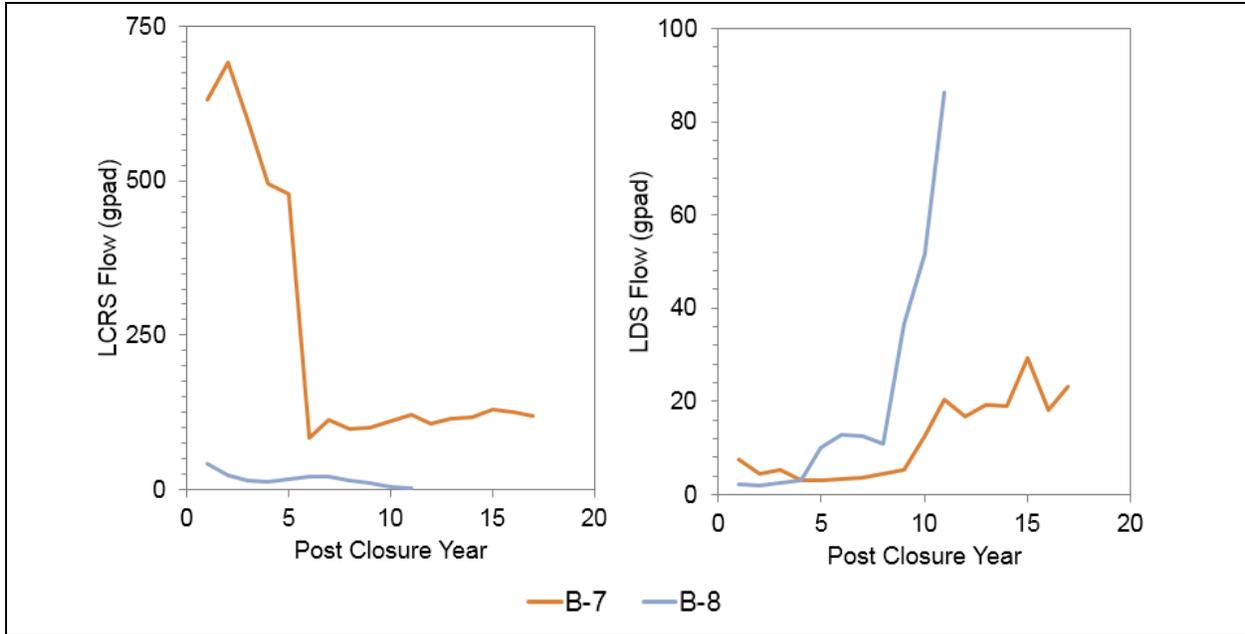


Figure 4-2. Annual average LCRS and LDS flow, B-7 and B-8

LCRS flow in B-8 is very low, trending downwards while the LDS flow rate exceeds that of the LCRS and is trending upward to 100 gpad. The LCRS flow rate in B-7 has decreased significantly since the closure of the unit and is relatively steady at a value of about 100 gpad. The LDS flow is increasing and currently is about 20 gpad.

Landfill T (Design Configuration No. 3)

Available leachate data for Landfill T in the PCC period includes annual total LCRS and LDS flow volumes for up to 7 years in T-1 to T-6 and daily flow volumes for T-7 to T-18 for years 10 through 23 of PCC. Earlier records for T-7 through T-18 are not available. Annual average LCRS and LDS flow rates for the 18 units are presented in Figure 4-3 (note the difference in y-axis scale between the two graphs). LCRS flow rates are generally below 100 gpad, with some units below 10 gpad; however, trending behavior is difficult to visualize in most cases. T-16 exhibited an increase in LCRS flow rate between years 7 and 16 of PCC that the site operator made several attempts at addressing with partial success (as denoted by the “saw tooth” shape of the graph during this period) and was finally able to trace to a localized cover system breach. The cover was repaired in year 16, which resulted in rapid reduction in LCRS flow to rates similar to other units.

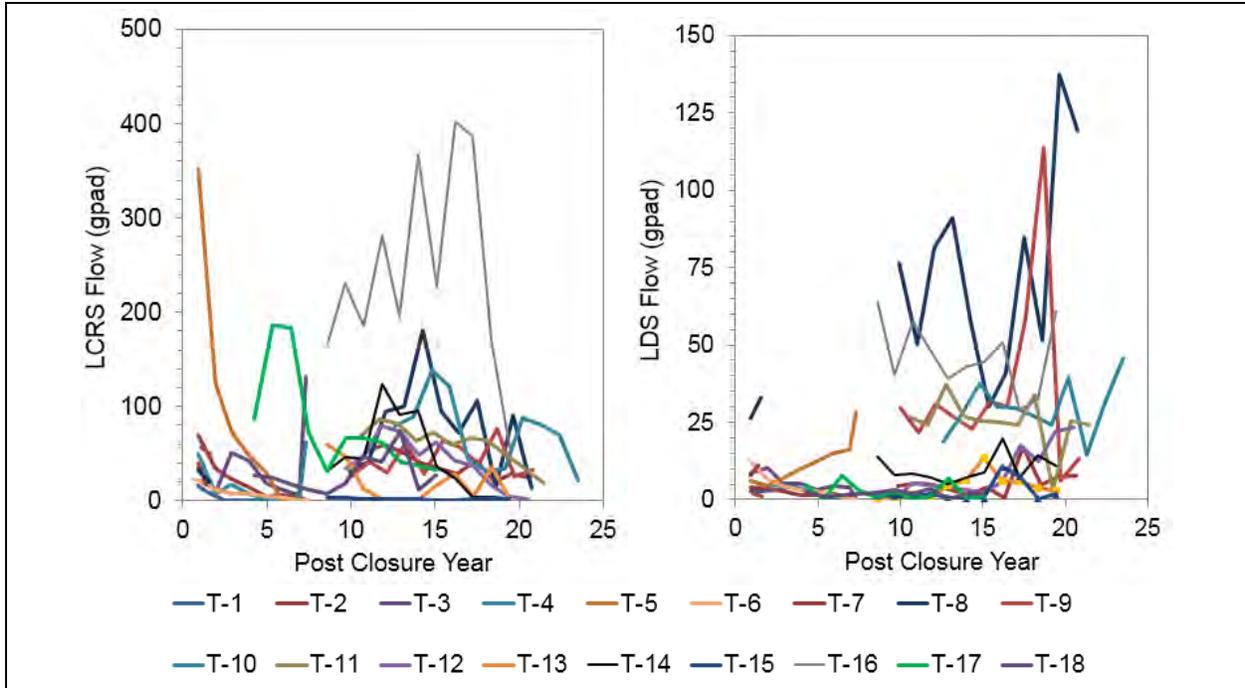


Figure 4-3. Annual average LCRS and LDS flow, Landfill T

As with LCRS flow data, steady or downward trends in LDS flow are not easy to visualize. Although LDS flow rates for T-1 to T-6 are below 10 gpad (with the exception of T-5), the LDS flow rates for T-7 to T-18 are significantly higher, similar in many cases to LCRS flow rates. This may be due to the shallow groundwater table at this site or lateral infiltration of stormwater runoff as detailed in Section 5 (Landfill T experiences an average of 70 inches of rain annually, the highest rainfall of any of the case study sites).

Landfill D (Design Configuration No. 12)

Available leachate data for Landfill D includes monthly LCRS and LDS flow volumes for two units D-1 and D-2, which have been closed for 7 and 9 years, respectively. The annual average LCRS and LDS flow rate for both units are presented in Figure 4-4. LCRS flow volumes in both units decreased rapidly from pre-closure levels and remained in a relatively steady state of decline since placement of the cover system. The climate at Landfill D is reasonably wet at 42 inches of average annual rainfall; however, of additional interest is the average annual snowfall of 17 inches at the site. Gradual melting of snow accumulated on the cover surface in spring has been reported as a significant factor influencing higher-than-expected leachate generation at landfills during the operational period; however, this does not appear to have influenced leachate rates during post-closure case at Landfill D.

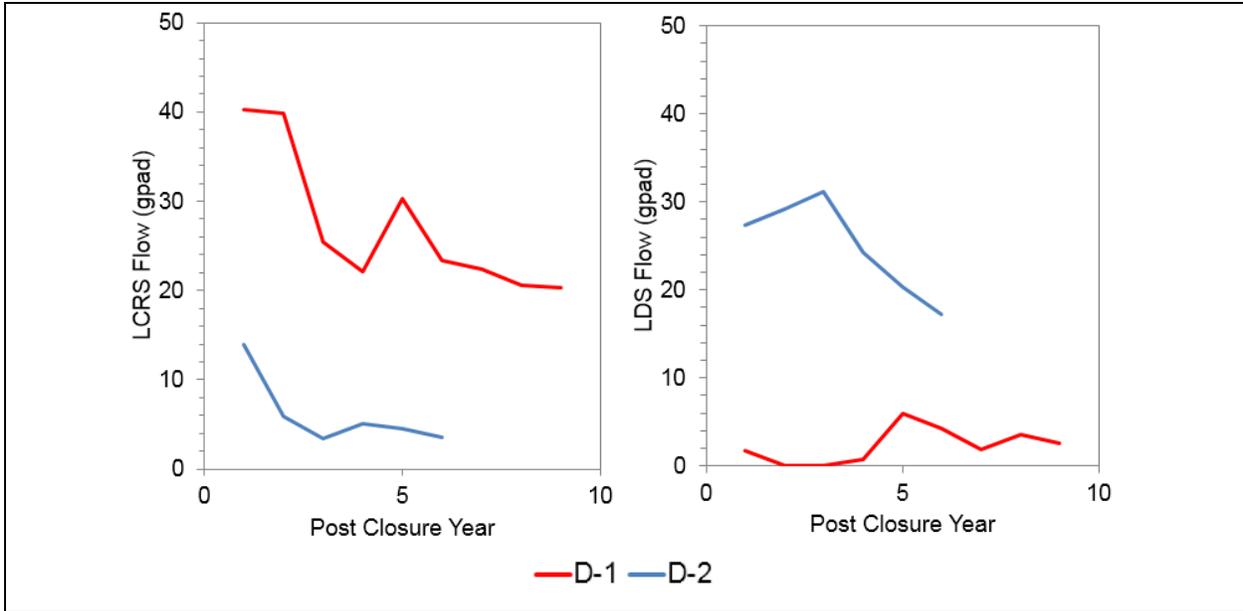


Figure 4-4. Annual average LCRS and LDS flow, Landfill D

The LDS volumes for D-1 have been below 5 gpad since the closure, whereas the LDS volumes in D-2 initially increased for the first 4 years after closure before exhibiting a steeply declining trend over the last 3 years. LDS flows in D-2 are also higher than LCRS flows. D-2 had a very short operational period of only 1 year: this compares to more than 10 years for most other facilities included in the study.

Landfill F (Design Configuration No. 13)

Available leachate data for the single case study unit F-1 at Landfill F includes monthly LCRS and LDS flow volumes for 12 years of PCC (Figure 4-5, note difference in y-axis scales).

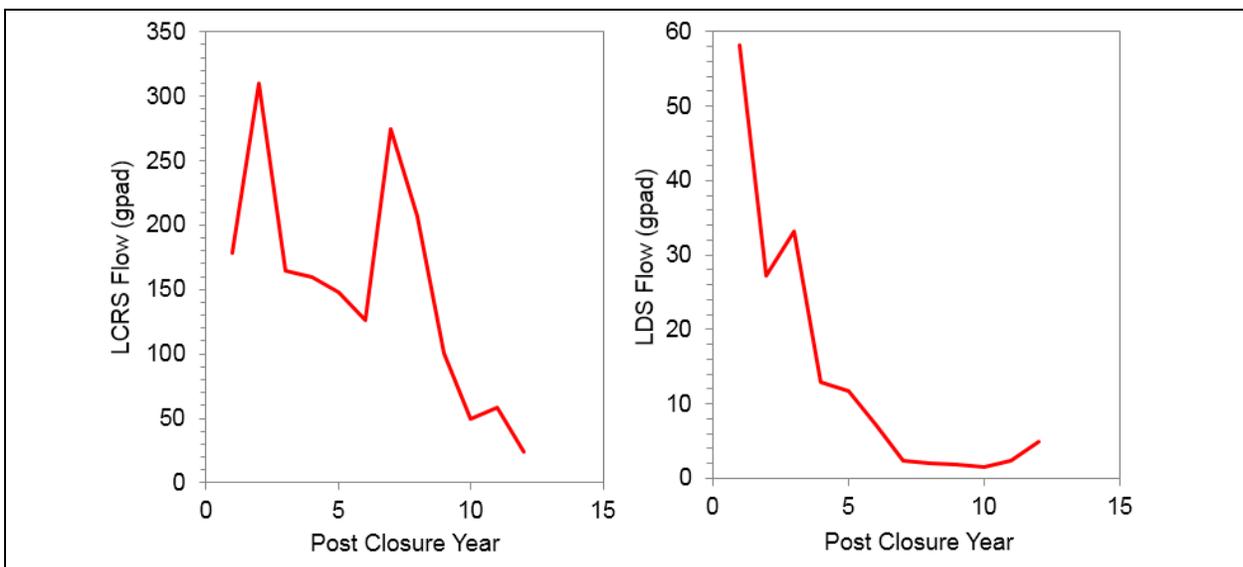


Figure 4-5. Average annual LCRS and LDS flow, Landfill F

Both LCRS and LDS flows have trended significantly downward at Landfill F, with current LCRS flows below 50 gpad and LDS flows below 5 gpad. The climate at the site is wet: 40 inches of average annual rainfall and 60 inches of average annual snowfall.

4.1.2 Dry Sites – Landfills J, R, Y, and M

This category of sites includes those in arid climates with deep groundwater tables. Landfill J receives only 11 inches of rainfall per year on average. Groundwater is also very deep at 300 feet below ground level. Landfill R is the aridest of all case study sites, with average annual precipitation of only 6.5 inches and a depth to groundwater of 250 feet. Landfill Y receives only 10 inches of annual rainfall on average, with deep groundwater (120 feet below ground level). Landfill M has average annual precipitation of only 9.5 inches and a depth to groundwater of 200 feet. Overall, it should be expected that this category of case study units would exhibit the lowest LCRS and LDS flows. Of particular note, three of the sites (Landfills R, Y, and M) are so dry that only negligible LDS flow has ever been recorded.

Landfill J (Design Configuration No. 4)

Available leachate data for Landfill J includes monthly LCRS and LDS flow volumes for all three case study units J-1 to J-3 for 17 years of PCC. Annual average LCRS and LDS flow rates are presented in Figure 4-6. LCRS and LDS flow rates for all three units are mostly below 1 gpad, which is not surprising since these units are situated directly beneath an MSW overfill landfill and the site is located in an arid region with very deep groundwater. As the cover system is directly beneath and in contact with the liner system for the overlying MSW landfill, the only liquid available to infiltrate the cover and form leachate is leakage through the liner system of the overlying MSW landfill. As such, post-closure leachate flows at Landfill J may be skewed relative to a typically closed landfill in that increased overburden pressures resulting from ongoing waste disposal in the overfill landfill may be contributing to larger-than-normal compression of the waste. This may partially help to explain why LDS flow rates generally equal or exceed LCRS flows. In particular, J-1 has experienced LDS flow rates of up to 5 gpad while LCRS flows have not exceeded 1 gpad. However, it is important to note that flows in both drainage layers are very low in relation to observations at most other sites.

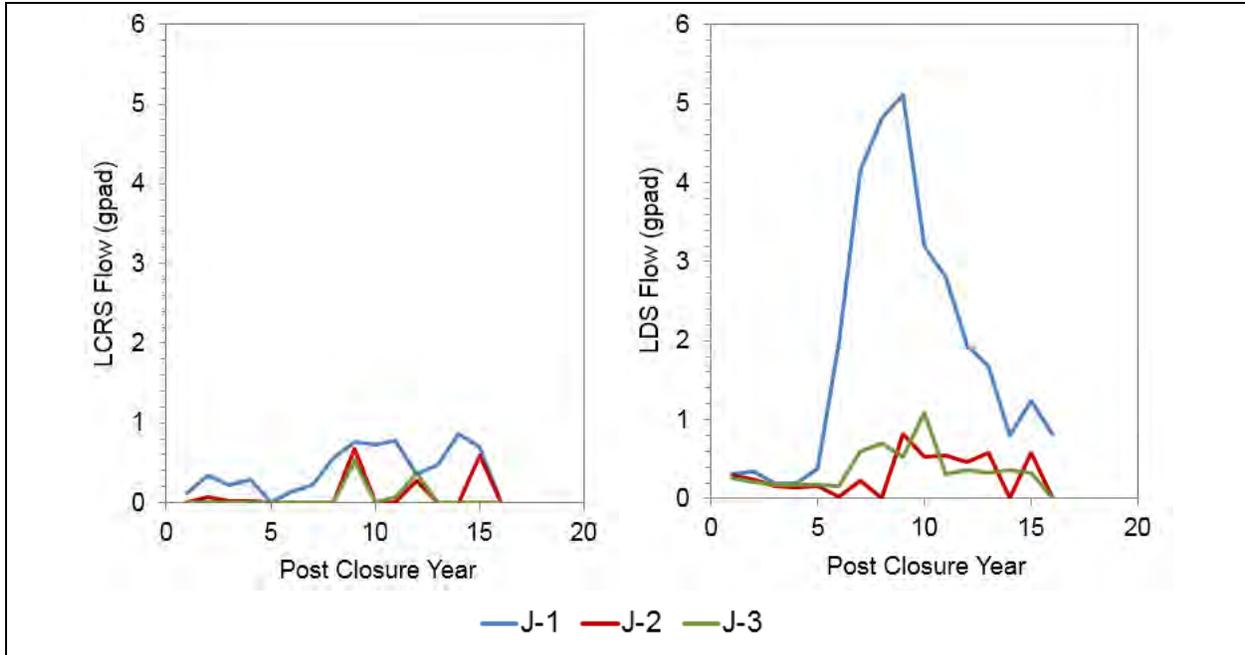


Figure 4-6. Annual average LCRS and LDS flow, Landfill J

Landfill R (Design Configuration Nos. 5 and 6)

The liner design for R-1 differs slightly from that of R-2 to R-5 with regard to the types of geosynthetic drainage (geonet vs. geocomposite) and barrier materials (40-mil PVC GM vs. 80-mil HDPE GM) specified. However, the designs are similar enough to be presented and discussed concurrently. Available leachate data for all five units includes monthly LCRS and LDS flow volumes for 13 years of PCC (Figure 4-7). The LCRS flow rate started near 10 gpad in R-1 but has declined steeply in subsequent years. LCRS flows in the other four units started slightly lower and have also declined, albeit less steeply. LDS flows have been zero throughout the PCC period with the exception of one reading of 1 gpad in year 10 in R-1.

Landfill Y (Design Configuration Nos. 9 and 10)

The liner design for Y-1 differs slightly from that of Y-2 and Y-3 with regard to the LCRS drainage layer design (Y-2 and Y-3 feature a GC in addition to a sand drainage layer, while Y-1 feature only a sand layer) and the specifications for geomembrane barrier materials. However, the designs are similar enough to be presented and discussed concurrently. Available leachate data for Landfill Y includes weekly LCRS and LDS flow volumes for three units for years 2 through 10 of PCC. Earlier records are missing. Annual average LCRS flow rates are presented in Figure 4-8. As shown, LCRS flow rates for all three units have trended downward since closure and have always been less than 10 gpad. LDS flows at this dry site (10 inches of annual rainfall on average) with deep groundwater (120 feet below ground level) have been negligible and are not shown or discussed.

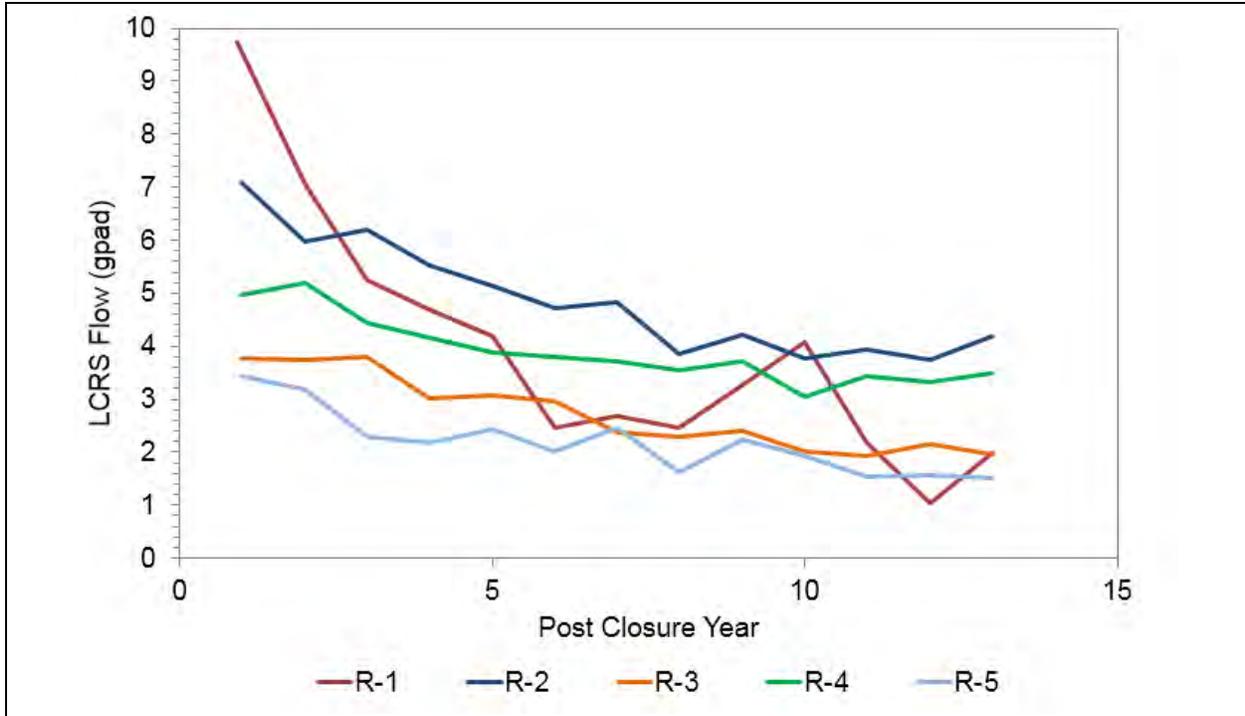


Figure 4-7. Annual average LCRS flow, Landfill R

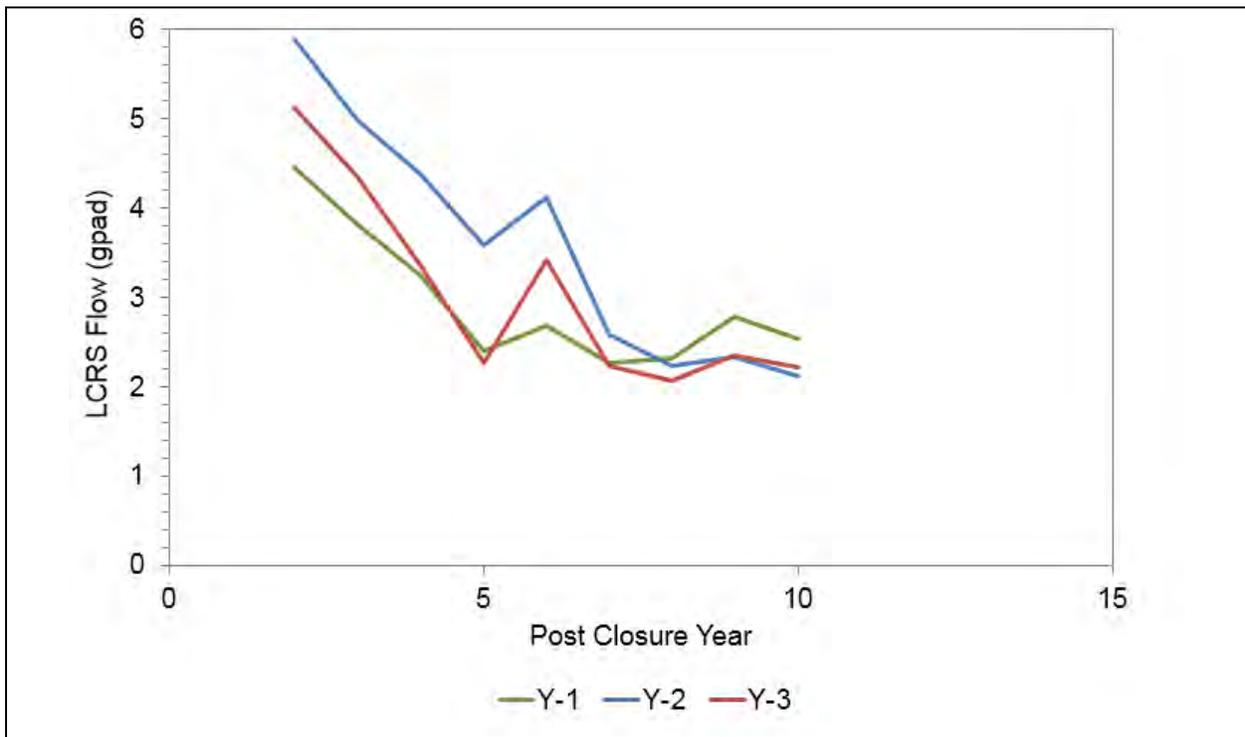


Figure 4-8. Annual average LCRS flow, Landfill Y

Landfill M (Design Configuration No. 11)

Available leachate data for Landfill M includes monthly LCRS and LDS flow volumes from one unit (M-1) within which waste has been placed to final grades and left undisturbed for

the last 4 years, but at which a final cover system has not yet been constructed. An alternative all-soil evapotranspiration final cover system has been approved for the site. The intermediate cover comprises an 18-in thick layer of the select soil. Despite this permeable cover, annual average LCRS flows are very low, trending downward from less than 20 gpad (Figure 4-9), and LDS flows have been negligible (not shown). The highest recorded LDS flow rate was 0.14 gpad in year 1, this has since declined to 0.07 gpad. Landfill M is one of the driest sites in this study, with average annual precipitation of only 9.5 inches and a depth to groundwater of 200 feet.

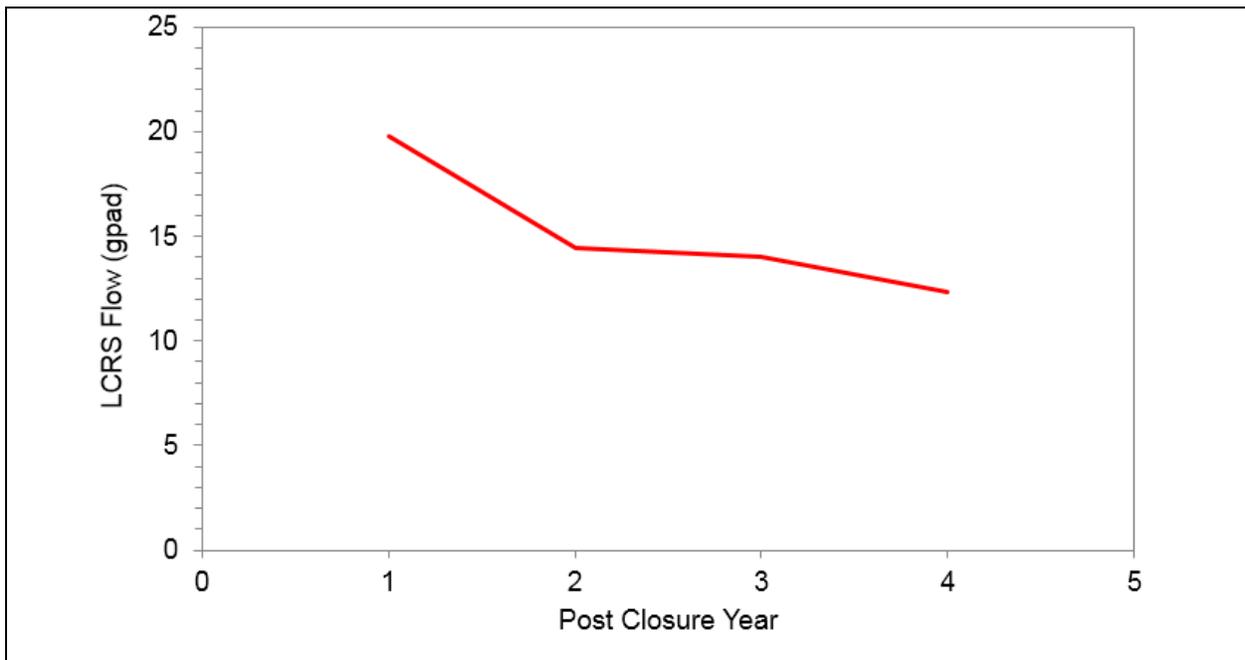


Figure 4-9. Annual average LCRS flow, Landfill M

4.1.3 Climatic Cusp Site – Landfill P

Landfill P experiences about 28 inches of rainfall annually, near the nominal value of 25 inches considered to separate wet and dry sites. As such, this site is considered a climatic cusp between the wet and dry site categories discussed above. The depth to groundwater represents an in-between condition relative to other sites; it is not excessively deep at only 40 feet below ground level, but this is significantly deeper than the three sites with shallow groundwater included in this study, at which groundwater is within 10 feet of the ground surface.

This site is also unique in that it has an intermediate liner system situated between the primary and secondary liners, with an overlying GN drainage layer providing separate recovery and recording of liquid flows (as depicted in Figure 3-5). For ease of identification

in this report, the upper LDS drainage layer is denoted “LDS1” while the lower LDS drainage layer is denoted “LDS2.” For assessing the performance of the primary liner, liquid flows in the LCRS are compared to the sum of flows in LDS1 and LDS2. The primary and intermediate liners are a single GM barrier sandwiched between two drainage layers. As such, it should not be expected that flow volume recorded in the LCRS, LDS1, and LDS2 would be substantially different if the only source of liquids is leakage through the GM.

There are four study units at Landfill P. The cover system design for the oldest unit P-1 is distinct from that of the three newer units P-2 to P-4 in that it features a 24-in CCL barrier layer whereas the other units feature a slimmer design utilizing a GCL barrier. As such, P-1 represents a notably different design configuration to that of the other three units.

Landfill P, Unit P-1 (Design Configuration No. 7)

Available leachate data for P-1 includes weekly LCRS and LDS flow volumes for years 4 to 21 of PCC (Figure 4-10). Earlier records are missing. LCRS flow exhibits a downward trend and has been less than 10 gpad for the entire PCC period. As noted above, the unusual triple-GM liner design at P-1 would be expected to result in flow volumes which show similar trends in the LCRS, LDS1, and LDS2. In fact, flow rates in LDS1 and LDS2 are similar and substantially greater than flows in the LCRS suggesting either a major defect in the primary liner GM or supplementary source of liquids in LDS1 and LDS2. Neither LDS flows exhibit real signs of trending behavior, although flow rates have declined relatively consistently since reaching peaks in year 13.

Landfill P, Units P-2 to P-4 (Design Configuration No. 8)

Available leachate data for P-2 to P-4 includes weekly LCRS and LDS flow volumes for up to 17 years of PCC. Annual average LCRS and LDS flow rates for all three units are presented in Figure 4-11. As with P-1, LCRS flow exhibits a downward trend in all three units and has been less than 10 gpad over the entire PCC period with the exception of the first 2 years in P-4. As noted above, the unusual triple-GM liner design at Landfill P would be expected to result in similar trends in flow volumes in the LCRS, LDS1, and LDS2. This is the case in P-4, although flow in the two LDS layers has been more erratic than in the LCRS. In P-2 and P-3, flow rates in LDS2 often exceed those in LDS1, while both LDS1 and LDS2 flow rates are greater than corresponding LCRS flows indicating either a major defect in the primary liner GM or supplementary source of liquids in LDS1 and LDS2.

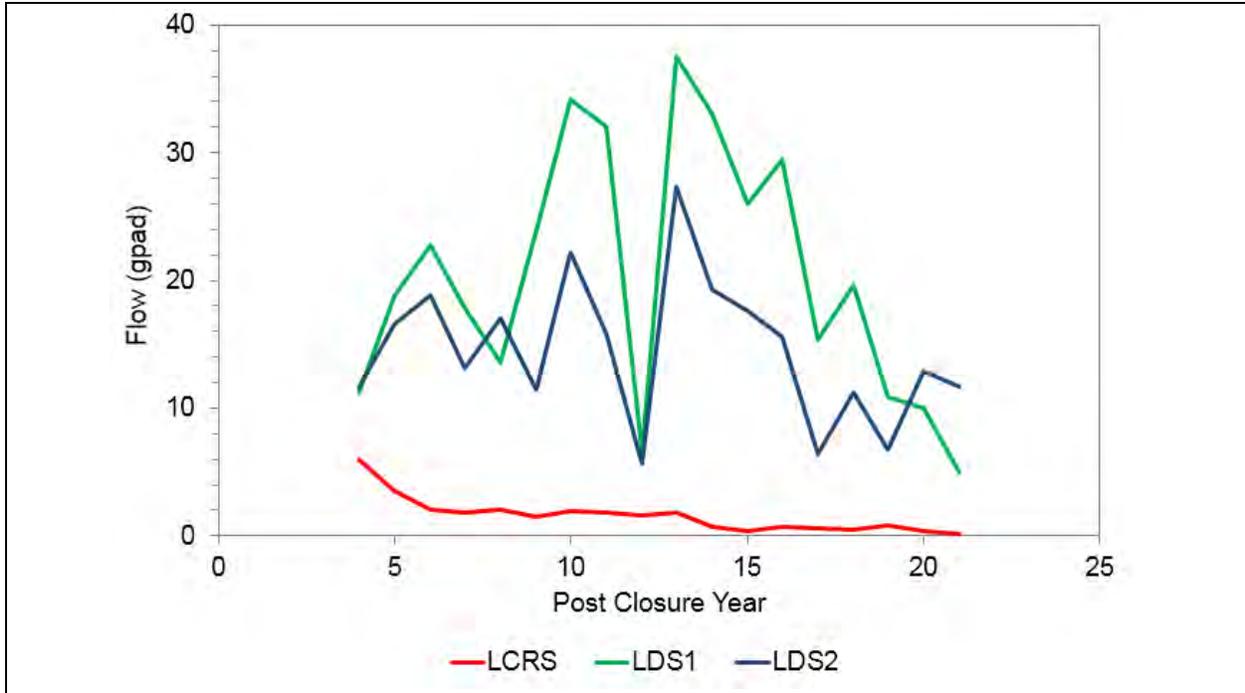


Figure 4-10. Annual average LCRS and LDS flow, P-1

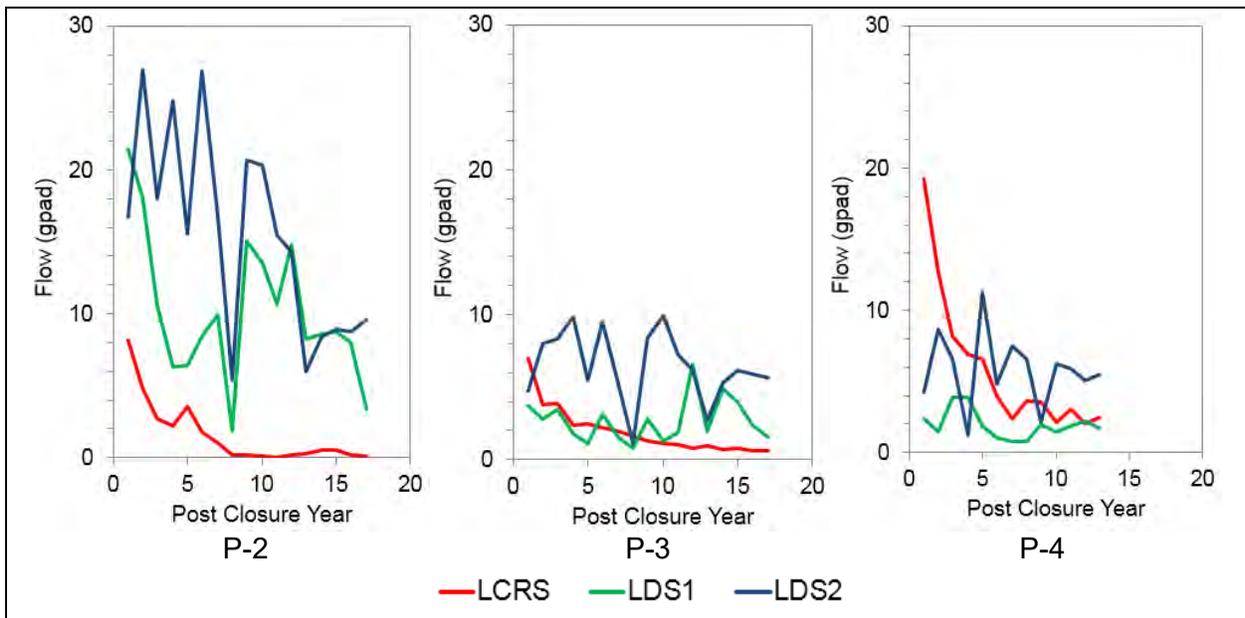


Figure 4-11. Annual average LCRS and LDS flow, P-2 to P-4

4.2 Comparing LCRS Flow Data to Modeled Predictions

Cognizant of the advantages and limitations of water balance modeling previously outlined in Section 1.5.2, the performance of the HELP Model as a predictive design tool was compared to the LCRS flow data obtained from the case study landfill units. Consistent with previous sections in Chapter 4, HELP Model results are presented and discussed in terms of

the 13 unique design configurations representing different cover, primary liner, and LCRS/LDS drainage layer configurations amongst the 45 case study units (as listed in Table 3-8).

4.2.1 Methodology

Modeling Leachate Generation

HELP Version 3.07 (Schroeder et al., 1994a) was used to estimate landfill leachate generation before and after placement of the final cover system. The methodology used average annual (not peak) HELP Model results calculated for a 100-year period to simulate the range of weather conditions that a landfill may experience. Weather data for the simulation (i.e., daily precipitation, temperature, and solar radiation values) were generated by the **model based on default assumptions for the closest city in the model's database to the landfill location**. Material properties for soil layers were selected based on default values. Protective cover soil and waste were modeled as vertical percolation layers. GMs, GCLs, and CCLs were modeled as barrier soil layers and the LCRS and LDS were modeled as lateral drainage layers. With regard to the selection of a representative number of GM defects to input to the model, a 10-year survey by Forget et al. (2005) reported 0.5 defects/hectare for sites with strict CQA and 16 defects/hectare without CQA. All the case study sites had CQA performed during liner and cover construction, suggesting a value of 0.5 defects/hectare may be appropriate. However, whether the level of CQA performed at each **case study could be similarly interpreted as "strict" was unknown. Therefore, GMs were conservatively assumed to have 2 defects/acre, in the middle of the range of 1 to 4 defects/acre suggested by EPA (Schroder et. Al, 1994) for "good quality GM installation."** Cell geometry, drainage length, and waste height were input based on landfill design plans (see Table II-1 in Appendix II). The input parameters for each HELP Model run are provided in Appendix V.

Three scenarios were modeled for each of the 13 design configurations to establish boundary conditions based on expected landfill leachate generation before and after placement of the final cover system:

- HELP Model Scenario No. 1 (pre-closure conditions with default input): The first scenario assumes that waste had been placed to final grades but no engineered measures beyond intermediate cover soil application had been implemented to prevent direct infiltration of rainwater into the landfill. This flat-line value serves to provide a (likely overestimated) upper-bound indication of short-term LCRS and LDS flows expected in the first few years after closure.

- HELP Model Scenario No. 2 (long-term quasi-steady state conditions with default input): The second scenario assumes a final cover system has been constructed over the waste and the cover is assumed to be well established, graded, and stabilized with good vegetation coverage and capable of diverting most incident rainfall as stormwater runoff. This flat-line value serves as a lower-bound limit on expected LCRS and LDS flows over the long-term.
- HELP Model Scenario No. 3 (annual LCRS flow model with site-specific input): In the third scenario, a more representative prediction of annual changes in leachate flow was attempted. First, the HELP Model was run under similar default input assumptions as for Scenario 1, except that it was assumed that waste had been placed to final grades for the entire operational life of the landfill unit(s). The model was run only for the operational period. The output file from this run was then used to assign site-specific values of volumetric water storage in each layer (i.e., cover, waste mass, and liner) in the last year of operation (i.e., immediately prior to capping). The HELP Model was then rerun for 30 years under conditions of final cover (akin to Scenario 2), but with site-specific values for water storage in the waste mass and liner (default values were used for water storage in the cover since these layers would have been newly constructed). The output file from this model run showed annual flow in the LCRS and LDS drainage layers on a year-on-year basis for 30-years of PCC.

Calculating Trends in LCRS Flow

Assuming that LCRS flow rates decrease exponentially after closure, which is consistent with observations in EPA (2002), expected leachate generation can be modeled as an exponential best-fit trend line to the data. An exponential decay model of the form shown in Equation 4-1 can be constructed, where f is a slope factor that depends mainly on the type of cover in place:

$$\frac{L_t}{L_{PEAK}} = e^{-ft} \quad (4-1)$$

Leachate generation is plotted as a function of time (t), such that the ratio of leachate generated in any given year during the post-closure period (L_t) is a function of peak leachate generation at or soon after closure (L_{PEAK}).

4.2.2 Results

Upper- and Lower-Bound Thresholds for LCRS and LDS Flow

Results from HELP Model Scenario Nos. 1 and 2 representing upper bound (intermediate cover) and lower bound (final cover) average flow rates in the LCRS and LDS, respectively, are summarized for all 13 design configurations in Table 4-1.

Table 4-1. Modeled LCRS and LDS flow

Design configuration	Climate Condition	Study Unit(s)	Average annual flow from HELP Model (gpad)				Notes
			Scenario No. 1: Intermediate cover (pre-closure)		Scenario No. 2: Final cover (post-closure)		
			LCRS	LDS	LCRS	LDS	
1	Wet	B-1 to B-6	1243	5E-01	5	4E-03	
2	Wet	B-7 to B-8	1243	5E-01	5E-01	4E-04	
3	Wet	T-1 to T-18	1319	8E-02	5E-05	2E-07	
4	Dry	J-1 to J-3	0.2	4E-05	Zero	Zero	1
5	Dry	R-1	38	2E-03	1E-04	1E-05	
6	Dry	R-2 to R-5	38	2E-05	1E-04	8E-07	
7	Cusp	P-1	122	1E-02	3E-04	5E-06	4
8	Cusp	P-2 to P-4	122	1E-02	2E-04	4E-06	4
9	Dry	Y-1	5	9E-01	8E-07	1E-05	2
10	Dry	Y-2 to Y-3	5	9E-01	8E-07	1E-05	2
11	Dry	M-1	129	2E-04	22	2E-05	3
12	Wet	D-1 to D-2	504	15	5E-05	3E-04	2
13	Wet	F-1	559	16	2E-03	3E-04	

Notes:

- 1). Landfill J has an overfill MSW landfill.
- 2). Landfills Y and D have more flow in the LDS than LCRS after the final cover is placed due to a boundary constraint in the HELP Model, see discussion below.
- 3). Scenario 2 at Landfill M is hypothetical, as the final cover has not yet been placed (current cover performance reflects Scenario 1).
- 4). LDS flow is for lower drainage layer (LDS2).

Flow values from the HELP Model are converted to gpad to allow easy comparison to field data. These results are primarily used to estimate modeled liner efficiency for comparison to effective liner efficiency calculations in Section 5.2. Blinded input and output files from each HELP Model run are provided in Appendix V. Some observations on the results presented in the table include:

- Design Configuration Nos. 1 and 2: Units B-1 to B-6 have a thinner cover system GM as compared to B-7 and B-8, and the GM and CCL layers are switched such that the CCL overlies the GM. This is reflected in the significant difference in modeled LCRS and LDS flows post-closure (a CCL overlying a GM is leakier because it does not take advantage of the synergistic benefits of a composite barrier in which an underlying CCL or GCL effectively “plugs” holes in the overlying GM).
- Design Configuration Nos. 2 and 3: Units B-7 and B-8 have an 18-in sand drainage layer above the barrier layer in the final cover system whereas all units at Landfill T have a GN drainage layer. This is reflected in the significant difference in LCRS and LDS flows post-closure between these two configurations (overall, a GN has been shown to be more efficient at reducing flows).
- Design Configuration No. 4: The cover system for J-1 to J-3 is integrated with a liner for an overfill MSW landfill covering the entire surface area of the three units. Modeling infiltration through the final cover is not meaningful under these conditions, as the only source of infiltrating water is leakage through the overlying liner, which is negligible (the HELP Model analysis would show zero flow in the LCRS and LDS).

- Design Configuration Nos. 5 and 6: R-1 has a 40-mil PVC GM liner while the other units at Landfill R (R-2 to R-5) have an 80-mil HDPE liner. The apparently superior performance of the HDPE liner at limiting leakage as compared to PVC is reflected in the model results for LDS flow as the HELP model default values assign two orders of magnitude lower permeability to HDPE GM when compared with PVC GM.
- Design Configuration Nos. 7 and 8: P-1 has a different cover system design to P-2 to P-4, although this does not significantly affect hydraulic performance. More importantly, all four units have three basal drainage layers (LCRS, LDS1, and LDS2) separated by single GM liners. The HELP Model output did not show any flow in the upper LDS1 once the final cover was placed (although flow has been observed in this layer). Results in the table thus represent modeled flow in the LCRS and lower LDS2.
- Design Configuration Nos. 9 and 10: Y-2 and Y-3 have a thicker GM in the primary liner than Y-1, although this does not affect modeled advective flow through the primary liner.
- Design Configuration Nos. 9, 10, and 12: HELP Model output for Y-1 to Y-3 and D-1 shows higher flow in the LDS than LCRS after the final cover is placed, although it is important to note that flow values in both drainage layers are extremely low in all affected units. This result is possibly due to a boundary constraint in the HELP Model in that the preferential flow path for a very small thickness of liquid above the GM in the LCRS layer is vertical (down through a hypothetical defect in the GM) rather than lateral. Therefore, all this liquid is erroneously assigned as vertical leakage through the primary liner by the model rather than lateral conveyance in the LCRS.
- Design Configuration No. 11: Landfill M is located in an arid region and has a composite primary liner resulting in minimal flow in LDS when compared with LCRS. The study unit M-1 is not technically closed, but has been filled to final grades and is inactive under the intermediate cover soil. Due to the dry climate and low leachate flows observed, an all-soil ET cover has been permitted as final cover at this facility.

Estimating Timeframes to Achieve Steady State Leachate Flow

In order to better understand behavioral trends in leachate generation based on field observations, annual LCRS flow measurements were compared to annual flow rates predicted from HELP Model Scenario 3. Acknowledging limitations in the HELP Model (see Section 1.5.2), the basis for this comparison is that if the HELP Model provides an accurate prediction of long-term leachate flow post-capping, the LCRS flow rates would decrease to a value approximating the quasi-steady state flow rate predicted by the model under this scenario. Selected results from six of the 13 design configurations representing the case study units are presented in Figure 4-12 through Figure 4-15. The selected results feature four of the case study sites: wet Landfills B and F and dry Landfills R and Y. With reference to each figure, the estimated time to reach quasi-steady state leachate flow as predicted by the HELP Model is shown for individual units as well as for the average of all data comprising a different design configuration in Table 4-2. The value of the slope factor (f) from the best-fit trend line and the coefficient of correlation to the data (R^2) are also shown.

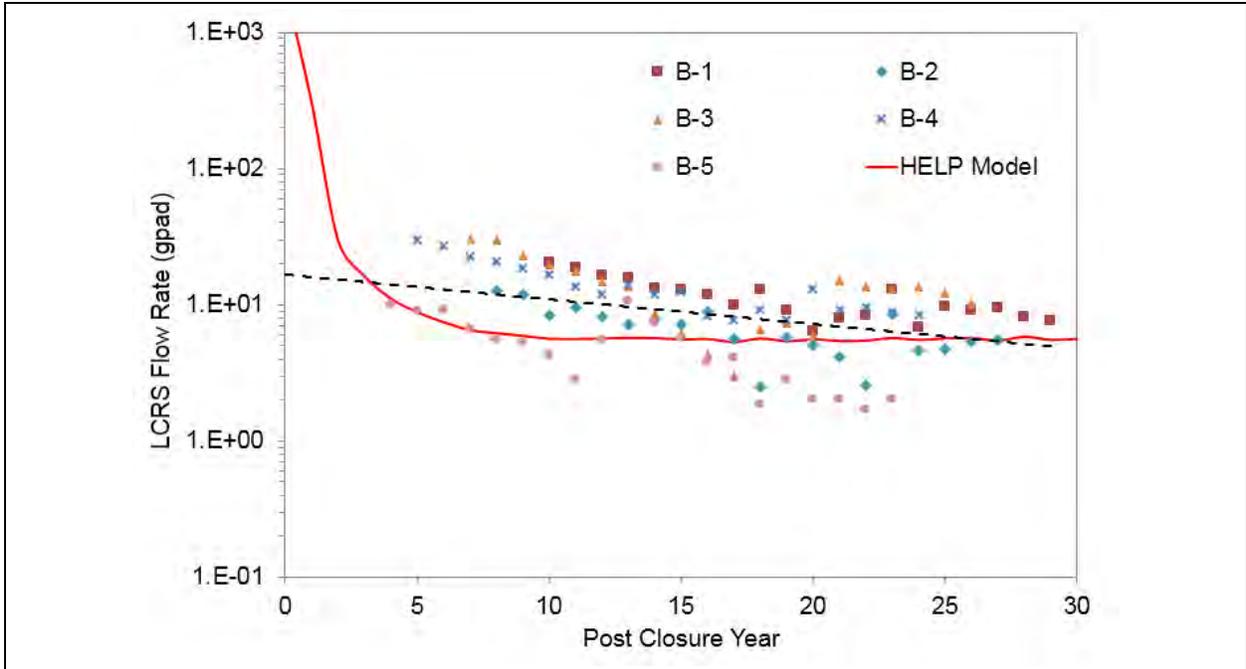


Figure 4-12. Trends in long-term LCRS flow, B-1 to B-5

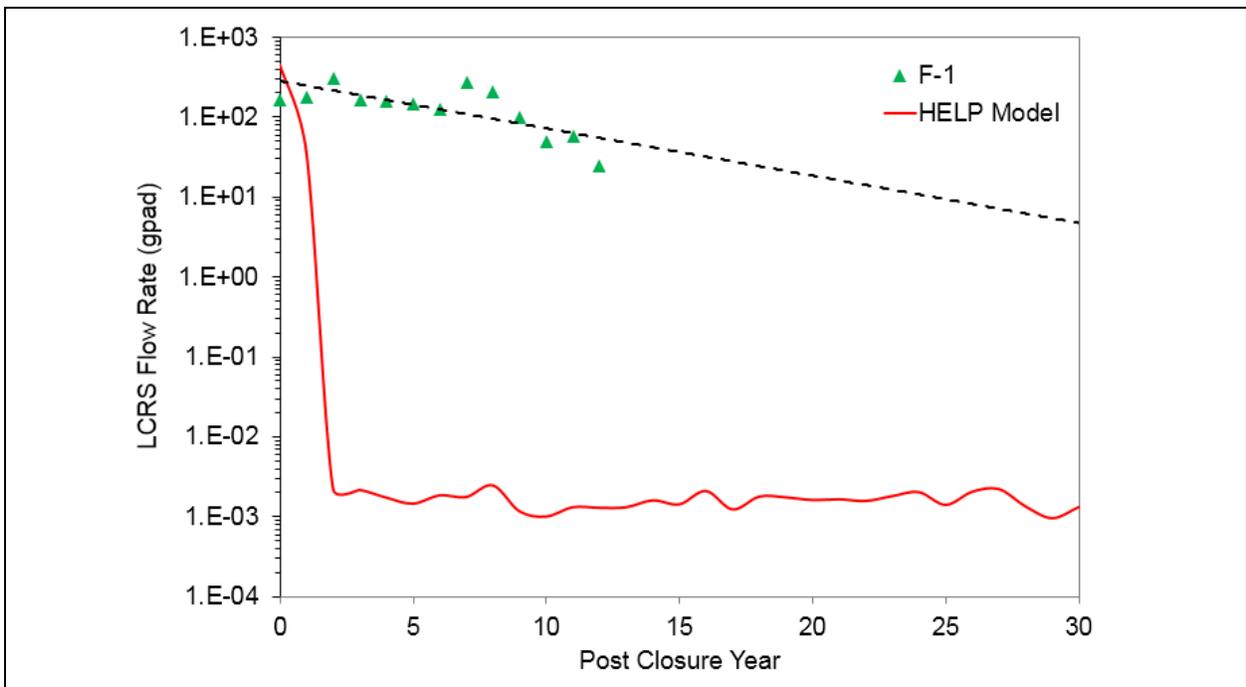


Figure 4-13. Trends in long-term LCRS flow, F-1

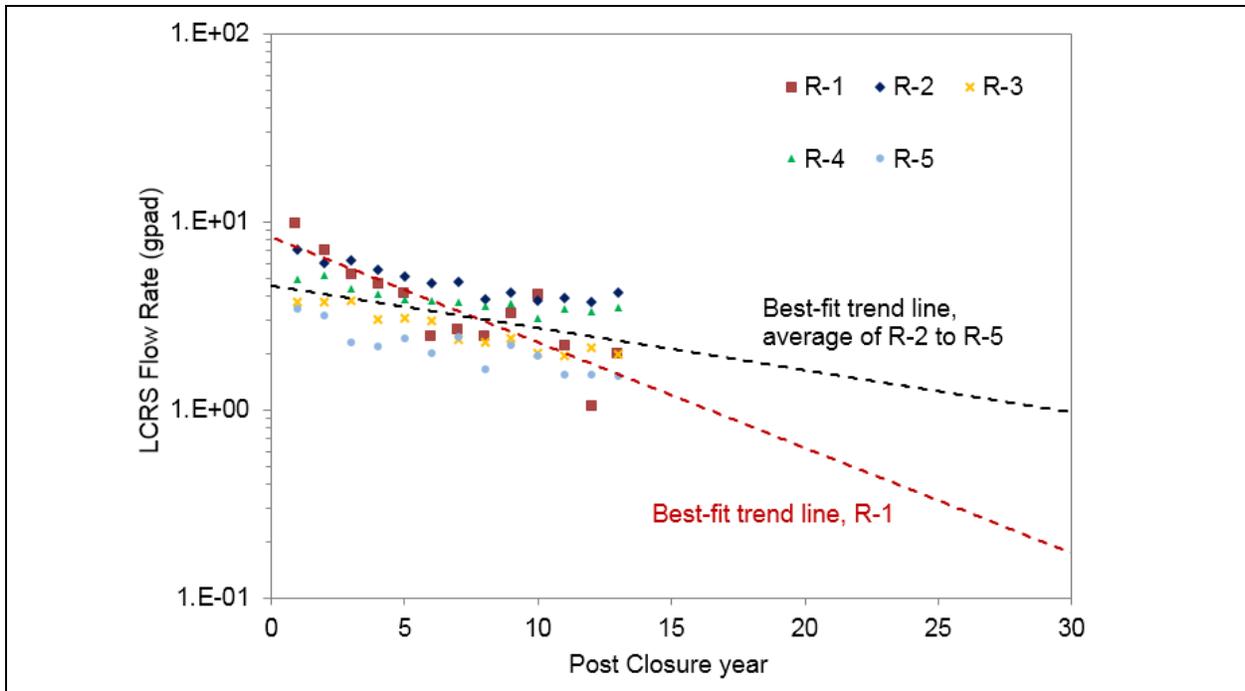


Figure 4-14. Trends in long-term LCRS flow, R-1 to R-5

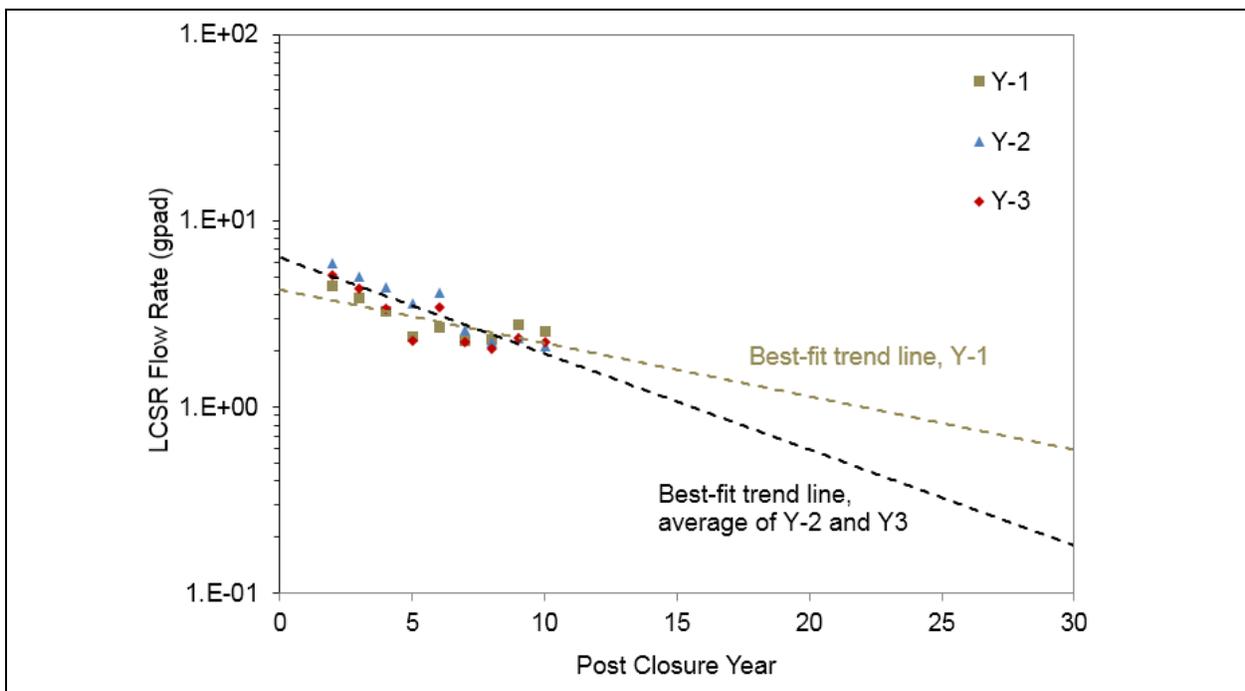


Figure 4-15. Trends in long-term LCSR flow, Y-1 to Y-3

Table 4-2. Estimated timeframe to achieve steady state leachate flow

Design configuration	Climate Condition	Cover system barrier layer design	Study Unit	Characteristic of best-fit exponential trend line to the data		Time to intercept HELP Model flow rate ⁽¹⁾ (years)
				Slope factor (f)	Correlation coefficient (R^2)	
1	Wet	24-in CCL over 80-mil HDPE GM	B-1	0.05	0.64	29
			B-2	0.05	0.43	18
			B-3	0.05	0.19	27
			B-4	0.06	0.78	28
			B-5	0.09	0.68	8
			B-1 to B-5	0.04	0.16	27
13	Wet	40-mil LLDPE over GCL	F-1	0.13	0.54	97
5	Dry	80-mil HDPE GM over 24-in CCL	R-1	0.13	0.73	Not calculated
6	Dry	80-mil HDPE GM over 24-in CCL	R-2	0.05	0.85	Not calculated
			R-3	0.06	0.91	Not calculated
			R-4	0.03	0.79	Not calculated
			R-5	0.06	0.77	Not calculated
			R-2 to R-5	0.05	0.26	Not calculated
9	Dry	40-mil HDPE GM over GCL	Y-1	0.07	0.58	Not calculated
10	Dry	40-mil HDPE GM over GCL	Y-2	0.13	0.92	Not calculated
			Y-3	0.10	0.71	Not calculated
			Y-2 and Y-3	0.12	0.78	Not calculated

Note:

1). The HELP model predicted zero LCRS flow at dry Landfills R and Y after 30 years; therefore, the time to intercept HELP model could not be calculated.

Overall, the analysis suggests that it would take from 8 to 97 years to reach the steady state flow rate suggested by the HELP model. It is interesting that only one unit require more than 30 years to reach steady state. In summary and discussion of the results presented in Table 4-2 and Figures 4-12 to 4-15:

- Design Configuration No. 1 (B-1 to B-5): The time to reach the steady-state flow rate (approximately 5 gpad) predicted by the HELP Model was less than 30 years for all five units at this wet landfill. Values for the slope factor (f) vary from 0.05 to 0.09, with the average for all units being 0.06 (the field data are poorly correlated to the trend line developed for the average). Overall, it is assumed that quasi-steady state leachate generation will continue at the level predicted by the HELP Model, perhaps declining to about 1 gpad as suggested by B-5.
- Design Configuration No. 13 (F-1): The HELP Model predicts that LCRS flow at this single-unit wet landfill would decline to about 0.001 gpad within four years of closure and remain steady thereafter. This seems an impractically low target for ending leachate management, although the best-fit trend line to the data ($f = 0.13$, $R^2 = 0.54$) suggests that this steady state flow rate would be reached in 97 years. In addition, the data record extends only 12 years at this site and fitting a trend to only the most recent 5 or 6 years of data would produce a much steeper trend line.

- Design Configurations Nos. 5 and 6 (R-1, R-2 to R-5): The steady state flow rate predicted by the HELP Model for both these configurations is on the order of 10^{-4} gpad; however, as discussed previously the model does not properly account for lateral drainage at very low flows in LCRS drainage media (as shown in Table 4-1, the HELP Model predicts that LDS flows would be two orders higher than LCRS flows). As such, the HELP Model may not provide accurate estimates of steady state leachate flow at very dry sites such as Landfill R, and comparisons to modeled predictions are not made in Figure 4-14 or Table 4-2.
- Design Configurations Nos. 9 and 10 (Y-1, Y-2, and Y-3): The steady state flow rate predicted by the HELP Model for both design configurations at dry Landfill Y is on the order of 10^{-7} gpad. This prediction may not be accurate for the reasons discussed above, and comparisons between best-fit trend lines and modeled predictions are not made in Figure 4-15 or Table 4-2.

With regard to the slope factor calculated from these results, it is interesting to note that this is significantly lower than that calculated from the dataset reported by EPA (2002), which suggested an order of magnitude decrease in LCRS flow rate should be expected every 5 years after closure. This would equate to a slope factor $f = 0.5$ in Equation 4-1. Slope factors calculated from this study suggest values less than 0.15, or even less than 0.1, may be more appropriate at most Subtitle C landfills. The dataset utilized in this study is larger (45 individual units with up to 29 years of post-closure data) than for the 2002 study (33 units with up to 9 years of post-closure data). In particular, the lack of data from many units beyond year six of PCC in the EPA (2002) study, inclusion of MSW landfills, and the very low flow rates reported for a few units after year six, suggests that the data may have been skewed by a small number of very dry or high performing sites. If current industry projections of post-closure leachate generation are based on the 2002 data, expectations may need to be reset in terms of the rate of decline in LCRS flow. Rather than an order of magnitude decrease every 5 years, it is more reasonable to expect an order of magnitude decrease every 15–20 years. However, more field studies are needed to validate this finding before recommendations for adjusting current industry projections and accruals for leachate management are made.

5. ANALYSIS OF PRIMARY LINER PERFORMANCE

5.1 Apparent Hydraulic Efficiency of the Primary Liner

5.1.1 Methodology

Consistent with the study by EPA (2002), concurrent LCRS and LDS flow data from the case study landfill units were evaluated to estimate leakage rates and the hydraulic efficiency of the primary liner. Using a method suggested by Bonaparte et al. (1996), the “**apparent**” hydraulic efficiency, EA, of the primary liner can be calculated from observed LCRS and LDS flow rates as:

$$E_A(\%) = \left(1 - \frac{LDS \text{ Flow Rate}}{LCRS \text{ Flow Rate}}\right) \times 100 \quad (5 - 1)$$

The parameter EA is referred to as an “**apparent**” hydraulic efficiency because flow into the LDS sump may be attributed to sources other than leakage through the primary liner. If the only source of flow into the LDS sump is primary liner leakage, then Equation 5-1 provides the “**true**” liner hydraulic efficiency (ET). True liner efficiency provides a measure of the effectiveness of a particular liner in limiting or preventing advective transport across the liner. For example, if a liner has an ET of 99%, the rate of leakage through the primary liner would be 1% of the LCRS flow rate. The leakage rate for a given composite liner system is directly proportional to a number of defects in GM and leachate head over the liner system (Touze-Foltz and Giroud 2003; Giroud and Touze-Foltz 2005). Therefore, the true efficiency of a liner is not a constant, but rather a function of the hydraulic head in the LCRS and size of the area over which LCRS flow is occurring (Bonaparte et al., 2016).

The higher the value of EA, the smaller the flow rate from the LDS compared to the LCRS flow rate. The value of EA may range from zero to 100% with a value of zero corresponding to a LDS flow rate equal to the LCRS flow rate, and a value of 100% indicating no flow in the LDS (indicating a perfect liner). Negative values indicate that flow volumes in the LDS exceed those in the LCRS, which was observed at many case study units (Section 4.1).

5.1.2 Values Calculated by EPA (2002)

EPA (2002) reported the apparent efficiency of a number of different primary liner configurations for individual cells for which continuous LCRS and LDS flow rate data were available from the start of operation and for a significant monitoring period thereafter. In summary of data provided for the post-closure period:

- GM Liners: Calculated values for E_A from six individual cells ranged from 62.0 to 85.4%, 68.8%, 91.1 to 98.6%, 92.3%, 99.7%, and 99.6%.
- GM/GCL Liners: Calculated values for E_A from six individual cells ranged from 89.6%, 98.8%, 100%, 100%, 99.6%, and 100%.
- GM/CCL and GM/GCL/CCL Liners: Calculated values for E_A from four individual cells ranged from -1,300 to 83.4%, 36.3 to 80.5%, and 81.8 to 96.8%, and 94.0 to 97.2%.

In each category, single values (other than a range) indicate that only one calculation was performed for an individual cell. It is noted that the above results may include cells at non-hazardous waste landfills (i.e., MSW landfills with higher leachate generation potential than most hazardous waste landfills). Negative values indicate that flow in the LDS exceeded that in the LCRS. **Overall, the authors concluded that “flows from the LDS of cells with composite liners are usually very low. The true hydraulic efficiency of composite liners may often exceed 99.9%.”**

5.1.3 Values Calculated in this Study

Available flow data from each of the 45 units at the nine case study landfills was evaluated to calculate leakage rates and apparent hydraulic efficiencies of the primary liners based on landfill leachate generation rates (i.e., LDS versus LCRS flow rates). The minimum, maximum, and average value of liner efficiencies for each unit are summarized in Tables 5-1 to 5-3 along with the number of data points for each unit that falls into different E_A ranges.

values were calculated. In addition, average values were calculated using positive values only. It is also important to note that the liner efficiencies reflected in the tables were calculated as static values from average LCRS and LDS flow rates in the post-closure period. As these flows tend to decrease with time, E_A values may dynamically increase or decrease with time depending on the relative rates of decrease of LCRS flow versus LDS flow. Overall, only 5% of the units exhibited an efficiency E_A value greater than 99% with 73% of the units having an E_A value that is less than 90%. Furthermore, the apparent liner efficiencies calculated across the sites are significantly higher at dry than at wet sites. For the wet landfill sites, the total number of E_A values exceeding 99% is 96, representing 79% of the data, in Table 5-2. The proportion of sites at which E_A values are negative is approximately equal at 16 and 17% at wet and dry sites, respectively. It is interesting to note that the collection efficiencies calculated for dry landfill sites were much lower than those for wet ones. Only 1 percent of the units examined demonstrated an efficiency that is higher than 90% as presented in Table 5-1C.

Table 5-1. Apparent liner efficiency, E_A (wet sites)

Design configuration	Primary liner system	Unit	Apparent liner efficiency, E_A			Breakdown of data				
			Minimum	Maximum	Average ¹	Number	E_A value			
							<0%	0% to 90%	90% to 99%	>99%
1	80-mil HDPE GM over 60-in CCL	B-1	62%	83%	72%	20	0	20	0	0
		B-2	35%	87%	67%	20	0	20	0	0
		B-3	38%	94%	81%	20	0	17	3	0
		B-4	79%	98%	89%	20	0	9	11	0
		B-5	0%	68%	48%	20	15	5	0	0
		B-6	73%	98%	91%	20	0	8	12	0
2	80-mil HDPE GM over 60-in CCL	B-7	77%	99%	91%	18	0	8	6	4
		B-8	0%	97%	65%	12	3	6	3	0
3	60-mil HDPE GM over 36-in CCL	T-1	0%	84%	77%	8	6	2	0	0
		T-2	71%	94%	83%	8	1	6	1	0
		T-3	0%	90%	73%	8	1	6	0	1
		T-4	0%	88%	42%	8	1	6	0	1
		T-5	0%	98%	84%	8	3	2	3	0
		T-6	31%	100%	54%	8	0	6	0	2
		T-7	60%	98%	83%	13	0	8	5	0
		T-8	0%	68%	35%	13	6	7	0	0
		T-9	0%	82%	56%	14	4	8	2	0
		T-10	0%	82%	56%	11	1	10	0	0
		T-11	0%	93%	60%	11	1	9	1	0
		T-12	0%	99%	81%	11	2	4	5	0
		T-13	0%	100%	68%	11	4	5	1	1
		T-14	0%	94%	74%	11	3	5	3	0
		T-15	0%	75%	57%	11	4	7	0	0
		T-16	0%	92%	81%	11	1	9	1	0
		T-17	83%	100%	96%	11	0	1	7	3
		T-18	64%	100%	88%	11	0	4	6	1
12	60-mil HDPE GM	D-1	80%	100%	92%	10	0	4	3	3
		D-2	0%	45%	45%	7	6	1	0	0
13	80-mil HDPE GM	F-1	59%	99%	88%	13	0	4	7	2
TOTAL						367	62 17%	207 56%	80 22%	18 5%

Note:

1. Average values were calculated using positive values only.

Table 5-2. Apparent liner efficiency, E_A (dry sites)

Design configuration	Primary liner system	Unit	Apparent liner efficiency, E_A			Breakdown of data				
			Minimum	Maximum	Average ¹	Number	E_A value			
							<0%	0% to 90%	90% to 99%	>99%
4	60-mil HDPE GM over 18-in CCL	J-1	0%	35%	18%	15	12	3	0	0
		J-2	0%	1%	1%	6	5	1	0	0
		J-3	0%	3%	3%	3	2	1	0	0
5	40-mil PVC GM over 36-in CCL	R-1	84%	100%	99%	13	0	1	0	12
6	80-mil HDPE GM over 36-in CCL	R-2	100%	100%	100%	13	0	0	0	13
		R-3	100%	100%	100%	13	0	0	0	13
		R-4	100%	100%	100%	13	0	0	0	13
		R-5	100%	100%	100%	13	0	0	0	13
9	60-mil HDPE GM	Y-1	100%	100%	100%	9	0	0	0	9
10	80-mil HDPE GM	Y-2	100%	100%	100%	9	0	0	0	9
		Y-3	99%	100%	100%	9	0	0	0	9
11	60-mil HDPE GM over 18-in CCL	M-1	99%	100%	99%	5	0	0	0	5
TOTAL						121	19 16%	6 5%	0 0%	96 79%

Note:

1. Average values were calculated using positive values only.

Table 5-3. Apparent liner efficiency, E_A (cusp site)

Design configuration	Primary liner system	Unit	Apparent liner efficiency, E_A			Breakdown of data				
			Minimum	Maximum	Average ¹	Number	E_A value			
							<0%	0% to 90%	90% to 99%	>99%
7	60-mil HDPE GM	P-1	0%	0%	0%	18	18 ²	0	0	0
			0%	0%	0%	18	18 ²	0	0	0
8	60-mil HDPE GM	P-2	0%	48%	48%	18	17	1	0	0
			0%	1%	1%	18	17	1	0	0
		P-3	0%	72%	39%	18	10	8	0	0
			0%	46%	46%	18	17	1	0	0
		P-4	0%	92%	62%	14	1	12	1	0
			0%	79%	48%	14	10	4	0	0
TOTAL¹						136	108 79%	27 20%	1 1%	0 0%

Note:

1. Average values were calculated using positive values only.

2. Calculated as $[1-(LDS1+LDS2)]/LCRS$.

As noted previously, a calculated value for E_A below zero indicates that sources other than liner leakage are contributing to liquid volumes in the LDS. For simplicity, minimum apparent liner efficiency is reported as 0% in the tables where one or more negative values were calculated. In addition, average values were calculated using positive values only. It is

also important to note that the liner efficiencies reflected in the tables were calculated as static values from average LCRS and LDS flow rates in the post-closure period. As these flows tend to decrease with time, EA values may dynamically increase or decrease with time depending on the relative rates of decrease of LCRS flow versus LDS flow. Overall, only 5% of the units exhibited an efficiency EA value greater than 99% with 73% of the units having an EA value that is less than 90%. Furthermore, the apparent liner efficiencies calculated across the sites are significantly higher at dry than at wet sites. For the wet landfill sites, the total number of EA values exceeding 99% is 96, representing 79% of the data, in Table 5-2. The proportion of sites at which EA values are negative is approximately equal at 16 and 17% at wet and dry sites, respectively. It is interesting to note that the collection efficiencies calculated for dry landfill sites were much lower than those for wet ones. Only 1 percent of the units examined demonstrated an efficiency that is higher than 90% as presented in Table 5-3.

While the data evaluated here are limited, it appears that the use of a GM/CCL composite barrier system may not outperform the use of a single GM barrier as the primary liner in either wet or dry sites. Landfills B and T do not perform better than Landfills D and F, while Landfills J, R, and M do not outperform Landfill Y.

The following site-specific observations are offered, which emphasize a number of important limitations on the use of EA as a measure of liner performance:

- Rainfall and the depth of groundwater are at/near ground level in wet climates (such as that found at Landfill sites B and T) can be factors contributing to the low apparent liner efficiencies at those sites. While the hydraulic connection between the secondary liner and groundwater is not confirmed there is a potential for groundwater to intrusion into the LDS. Furthermore, precipitation, especially after large events, may infiltrate into anchor trenches or defects/appurtenances in the final cover system and migrate to the LCRS or LDS.
- Low leachate generation volume at arid climate landfills (such as Landfill J) can be low, LCRS (<0.001 gpad) and LDS (<0.1 gpad). Thus small variability in leachate volume measurement could translate to an extreme calculated efficiency. This may explain why the apparent liner efficiencies are mostly below zero site J.

Thus, it is important to take into account site-specific consideration when evaluating liner efficiency. Below are some examples of how site-specific conditions were used to explain efficiencies calculated for some of these sites:

- Landfills R, Y, and M are located in arid climates with little yearly rainfall, resulting in very low LCRS flows, negligible LDS flows, and high apparent liner efficiencies.
- Landfill F is an industrial site located in NE region with high average annual precipitation as both rainfall and snowfall. Minimum and maximum EA values for this site represent a temporal improvement: liner efficiency increased from 59% at closure to 94% five years

later. This suggests that EA may not be an effective measure of liner performance in early years while excess residual water in the landfill from the construction and operational phases is working its way out via the LCRS.

From the above, site-specific climatic, construction, and operational factors clearly have an important influence over calculated values for EA. This level of subjectivity provides a high level of uncertainty when using EA. A method to improve estimates of liner efficiency by correcting for relative LDS and LCRS chemistry data is discussed in Section 5.3.

5.2 Modeled Hydraulic Efficiency of the Primary Liner

5.2.1 Methodology

Expected post-closure LCRS and LDS flow rates for each of the 13 different design configurations were calculated using the HELP Model based on default inputs considered representative of pre-closure conditions (intermediate cover soil only) and post-closure conditions (final cover). The methods used to calculate these flow rates were described previously in Section 4.2.1 as HELP Model Scenario Nos. 1 and 2, respectively. Results were previously summarized in Table 4-1 (qualifications on the modeling approach and validity of output values were provided with the table; these are also relevant in the context of the use of these values here). Based on these values, a comparison of actual LCRS and LDS flow rates with modeled predictions can be made, which will facilitate critical assessment of the model's **adequacy** in predicting long-term leachate generation and, by association, leakage through the primary liner.

Similar to the method used for calculating apparent liner efficiency, EA (Section 5.1.1), the hydraulic efficiency of the primary liner system can be calculated as (Equation 5-2):

$$E_M(\%) = \left(1 - \frac{\text{LDS Flow Rate from HELP}}{\text{LCRS Flow Rate from HELP}} \right) \times 100 \quad (5 - 2)$$

Where EM is the modeled liner efficiency calculated based on relative flow in the LDS and LCRS as predicted by the HELP Model.

5.2.2 Results

Using the methodology above, EM was calculated for the 13 unique design configurations reflecting different cover, primary liner, and LCRS/LDS drainage layers used in the construction of the 45 case study units (Table 5-4). Some qualifications regarding results presented in the table include:

- Design Configuration No. 4: The cover system for J-1 to J-3 is integrated with a liner for an overfill MSW landfill covering the entire surface area of the three units. Modeling

infiltration through the final cover is not meaningful under these conditions, as the only source of infiltrating water is leakage through the overlying liner, which is negligible (the HELP Model analysis would show zero flow in the LCRS and LDS). Model results under intermediate cover suggest very high levels of hydraulic performance at this dry site.

- Design Configuration Nos. 9, 10, and 12: HELP Model output for Y-1 to Y-3 and D-1 shows higher flow in the LDS than LCRS after the final cover is placed, although it is important to note that flow values in both drainage layers are extremely low in all three units. This result is possibly due to a boundary constraint in the model in that the preferential flow path for a very small thickness of liquid above the GM in the LCRS layer is vertical (down through a hypothetical defect in the GM) rather than lateral. There is no head buildup over the primary liner as shown in HELP Model output files; therefore, the hydraulic performance of the primary liner is likely to be higher but cannot be calculated for post-closure conditions using the HELP Model method. The apparently low E_M values for these three units under conditions of intermediate cover should also be treated with caution given the limitations of the HELP Model described here.
- Design Configuration No. 11: Landfill M is located in an arid region and has a composite primary liner resulting in minimal flow in LDS when compared with LCRS. The study unit M-1 is not technically closed, but has been filled to final grades and is inactive under the intermediate cover soil. As such, hydraulic performance of the primary liner under intermediate cover represents current conditions.

Table 5-4. Modeled liner efficiency, E_M

Design configuration	Climate condition	Study unit(s)	Average annual flow from HELP Model (gpad)				Modeled liner efficiency, E_M	
			Intermediate cover		Final cover		Intermediate Cover	Final cover
			LCRS	LDS	LCRS	LDS (note 2)		
1	Wet	B-1 to B-6	1243	5E-01	5	4E-03	99.96%	99.93%
2	Wet	B-7 to B-8	1243	5E-01	5E-01	4E-04	99.96%	99.92%
3	Wet	T-1 to T-18	1319	8E-02	5E-05	2E-07	99.99%	99.58%
4 (note 1)	Dry	J-1 to J-3	0.2	4E-05	0	0	99.98%	Not Calculated
5	Dry	R-1	38	2E-03	1E-04	1E-05	99.99%	93.0%
6	Dry	R-2 to R-5	38	2E-05	1E-04	8E-07	100%	99.4%
7	Cusp	P-1	122	1E-02	3E-04	5E-06	99.99%	98.38%
8	Cusp	P-2 to P-4	122	1E-02	2E-04	4E-06	99.99%	97.54%
9	Dry	Y-1	5	9E-01	8E-07	1E-05	82.65%	Not Calculated
10	Dry	Y-2 to Y-3	5	9E-01	8E-07	1E-05	82.65%	Not Calculated
11 (note 3)	Dry	M-1	129	2E-04	22	2E-05	100%	Not Calculated
12	Wet	D-1 to D-2	504	15	5E-05	3E-04	96.94%	Not Calculated
13	Wet	F-1	559	16	2E-03	3E-04	97.16%	84.12%

Notes:

- 1). Landfill J has an overfill MSW landfill.
- 2). Landfills Y and D have more flow in the LDS than LCRS after the final cover is placed due to a boundary constraint in the HELP Model, see discussion below.
- 3). Final cover has not yet been placed at Landfill M.

Ignoring the HELP Model's limitations at accurately predicting LCRS and LDS flows at Landfills Y and D as discussed above, calculated values for EM exceed 99.9% at all but one other site (Landfill F, with calculated EM value of 97.2%).

5.3 Correcting Apparent Liner Efficiency Calculations

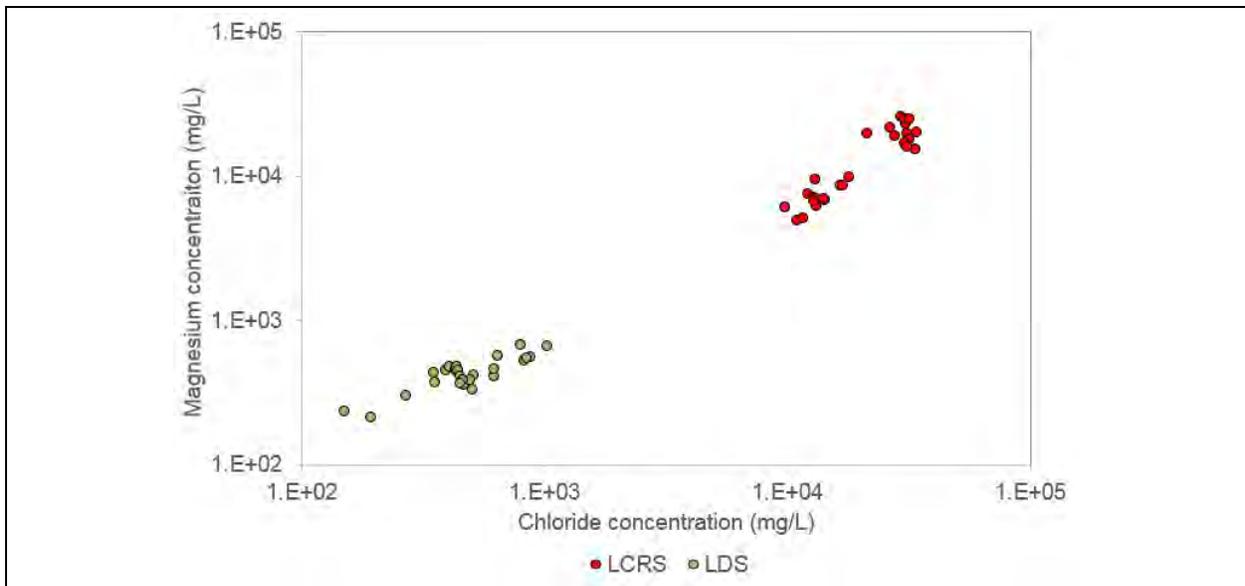
5.3.1 *Technical Basis*

Leachate chemistry data can be used to quantify the portion of liquids comprising total LDS flow that should be attributable to primary liner leakage as opposed to other sources (e.g., groundwater infiltration) by demonstrating a lack of hydraulic connection between the LCRS and LDS. This is done by distinguishing liquids with similar chemical signatures. The rationale is simple: if leakage through the primary liner is the main source of liquids in the LDS, then the concentrations of key indicator parameters in LDS liquids should be comparable to the concentrations of the same constituents in LCRS leachate for all samples collected concurrently in the two drainage layers. If the concentrations in LDS liquids are significantly lower than the corresponding concentrations in LCRS leachate, however, a lack of hydraulic connection can be inferred (i.e., dilution of LDS liquids from a non-leachate source is occurring). As discussed in more detail below, the preferred indicator parameter for comparing LCRS and LDS chemistry is chloride, a conservative (unattenuated) anion that does not take part in biochemical reactions and is typically found in elevated concentrations in leachate (Rowe, 1991). Comparison of LDS and LCRS chloride concentrations assumes that advective flow through pinhole defects in the GM is the only mechanism by which leakage through the primary liner occurs (i.e., diffusion through the GM, which is a potential transfer mechanism for volatile compounds, can be ignored). Potential adsorption of chloride to solids in the CCL or GCL component of a composite primary liner is also ignored, which is reasonable given the weak ion affinity of chloride and typically has low attenuation in clays (Pansu and Gautheyrou, 2006). Where chloride data are available, the apparent liner efficiencies calculated in Section 5.1 can be corrected. The main goal of this section is to demonstrate the presence (or lack thereof) of a hydraulic connection between the LCRS and LDS within a particular landfill unit.

Concentrations of Key Indicator Constituents

To initiate an evaluation of whether primary liner leakage had contributed to the observed LDS flows, the concentrations of key chemical constituents in LCRS and LDS flows were investigated. This included the four major cations (calcium, magnesium, potassium, and sodium) and three major anions (alkalinity, chloride, and sulfate), which typically are enough to calculate the ionic charge balance in environmental effluents, although in some

regions nitrate and ammonium can be important (Andersen et al., 2014). Comparing the ionic composition of liquids from the LCRS and LDS allows the chemical signatures of the samples to be easily differentiated. There are several ways in which to graphically portray the ionic composition of liquid samples, including Stiff, Piper, or Schoeller diagrams or simple bivariate plots (Bonaparte et al., 2011). Even simpler, anion/cation ratios (e.g., chloride/sodium or chloride/calcium) can be calculated. If the shapes of these diagrams or values of ratios differ significantly between LDS liquids and LCRS leachate, this provides a strong indication that the source of liquids may be different. The chemistry database for this study was very limited as only a few of the parameters were available concurrently in both the LCRS and LDS at each site. Alkalinity has not been analyzed at any site. Therefore, Stiff or piper plots could not be constructed, leaving Schoeller diagrams or bivariate plots as an alternative method of portraying the data. As an example of this approach, chloride and magnesium concentrations in the LCRS and LDS for four study units at Landfill T were plotted (Figure 5-1).



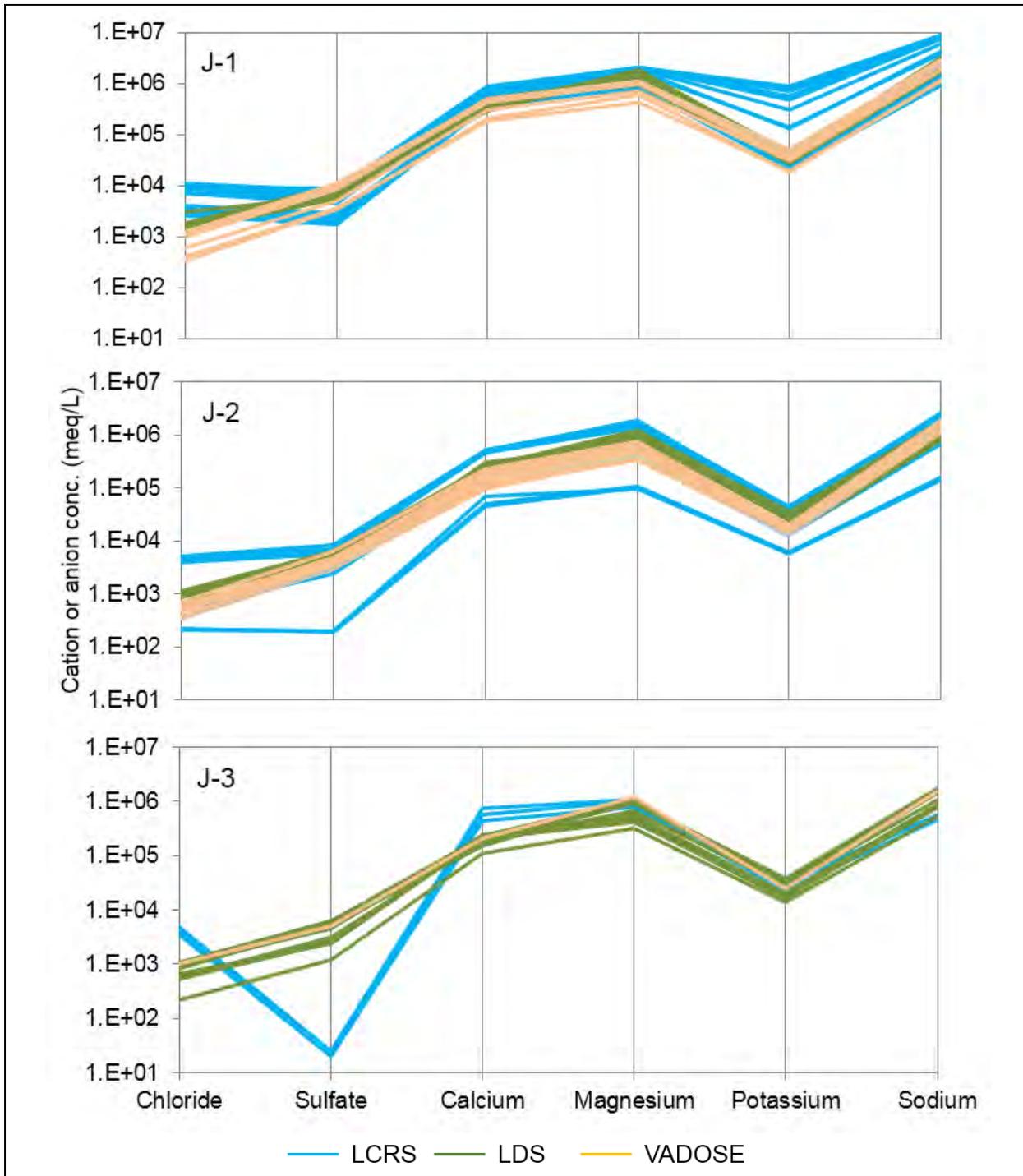


Figure 5-2. Ionic composition of liquids in the LCRS, LDS, and vadose zone, Landfill J

In all three units at Landfill J, the shape of the diagram for the LDS was much closer to the vadose zone than the LCRS, indicating that the chemical composition of liquids in the LDS are more closely related to data for the vadose zone than to leachate. The LCRS data exhibit significant variability between years, particularly in J-2, while the LDS and vadose zone data

are very consistent over time. The two anions (chloride and sulfate) showed the most significant contrast between the LCRS and LDS datasets.

Selection of Chloride as Indicator Parameter

Examination of the ionic composition of liquids in the LCRS and LDS at Landfills T and J presented above appears to support the use of chloride as the indicator parameter with the most significant (generally, orders of magnitude) distinction between these two drainage layers. Chloride is a conservative ion that does not take part in biochemical reactions and is not physically altered by the leaching process in landfills (Rowe, 1991). Therefore, chloride concentrations should be fairly similar between the LCRS and the LDS if the source of liquids in the LDS is due to primary liner leakage and not dilution from groundwater or other sources.

To further validate the selection of chloride as a useful indicator parameter with which to correct liner efficiency calculations, all available chloride data from the case study units were plotted (Figure 5-3). Chloride concentrations in the LCRS range from 53 to 34,320 mg/L with a median value of 9,290 mg/L, whereas concentrations in the LDS range from 2 to 3,000 mg/L with a median value of 22 mg/L. As shown in the figure, the significant contrast in chloride concentrations between the LCRS and LDS is relatively constant (especially once the first few years of post-closure have been completed), confirming selection of chloride as the preferred indicator parameter with which to review the hydraulic performance of the primary liner.

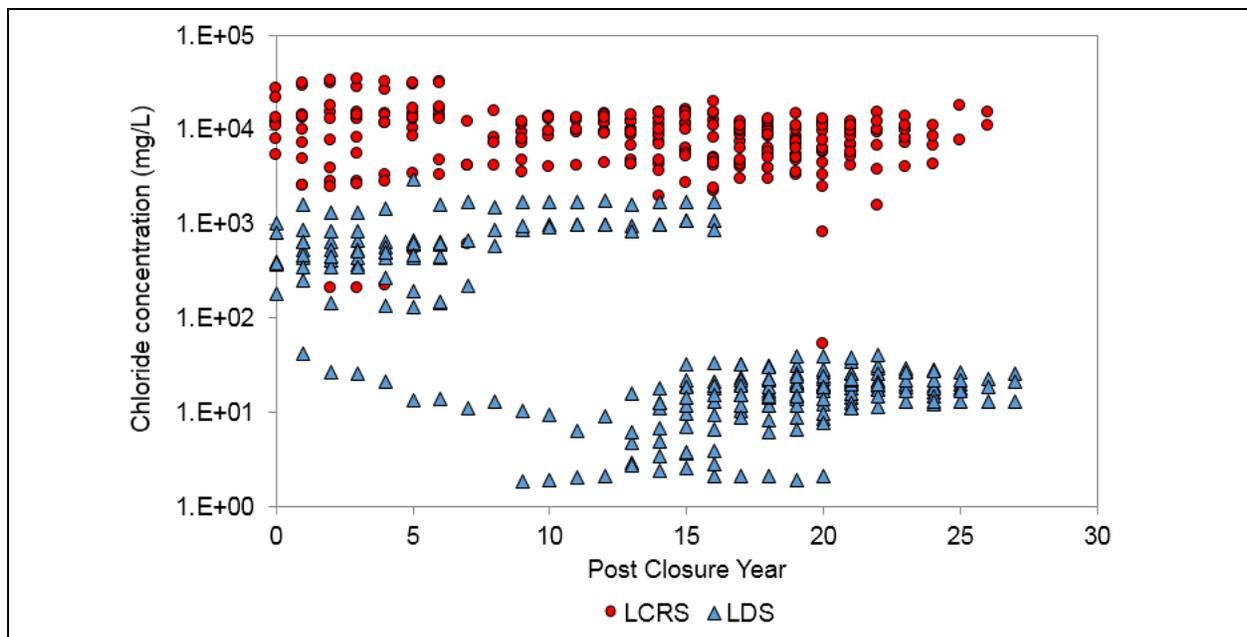


Figure 5-3. Comparison of chloride concentrations in LCRS and LDS liquids

5.3.2 Methodology

The LCRS and LDS chemical constituent data can be compared to estimate the portion of liquids in LDS which could be attributed to leakage through the primary liner only and separate this from liquids present in LDS due to construction water, consolidation of the primary liner, compression of drainage layer materials, and infiltration of groundwater (see Figure 1-2 and discussion in Section 1.5.3). For this comparison, chloride concentrations in the LCRS and LDS are used per the previous discussion.

A correction factor (CF) is first calculated as the ratio of chloride concentrations in the LDS and LCRS at a given time. CF is a dimensionless parameter as shown in Equation 5-3:

$$CF = \frac{\text{Chloride Concentration in LDS}}{\text{Chloride Concentration in LCRS}} \quad (5 - 3)$$

Equation 5-3 assumes that only leachate (i.e., LCRS liquids) contributes to the chloride concentration in the LDS and that chloride is not adsorbed to soils in the primary liner or drainage layers, or otherwise attenuated as it passes through the primary liner. Any contribution of chloride from other sources (e.g., brackish groundwater) would serve to increase the numerator in the expression and thus increase the magnitude of the CF value computed.

Using CF, a corrected liner efficiency, EC, can be derived using the simple expression in Equation 5-4:

$$E_c(\%) = \left(1 - CF \frac{\text{LDS Flow Rate}}{\text{LCRS Flow Rate}}\right) \times 100 \quad (5 - 4)$$

Note that chloride concentration and flow data must be temporally coincidental in the LCRS and LDS in order to calculate a corrected liner efficiency using Equation 5-4.

5.3.3 Results

The current leachate chemistry database has limited temporally coincidental chloride data in the LCRS and LDS. Overall, concurrent data are available for only 17 units at three sites (Landfills B, T, and J). A correction factor (CF) and corrected liner efficiency (EC) was calculated for each of these units as summarized in Table 5-5. The EC values shown in the table are average values for each unit, based on the number of data points indicated. The following observations are offered with respect to the performance of the primary liner based on the information shown in the table:

- CF values ranged over three orders of magnitude from 0.0004 (B-8) to 0.4 (J-1). Values less than 1.0 indicate that only a portion of the total flow in LDS should be attributed to

primary liner leakage. This was the case for all 17 study units, which suggests that values of E_A at other sites could be correctable if applicable LCRS and LDS chemistry data were available. This represents a shortcoming on the part of site operators at collecting data that could help understand long-term liner performance.

- Both Landfills B and T receive above-average rainfall (>25 inches) each year and the top of groundwater is at/near the ground surface, which increases the probability of infiltration into the LDS from groundwater. This is borne out in the data: based on the CF calculations, only 1 to 3% of the liquids volume in the LDS is attributable to leachate leakage at Landfill T, and only 0.04 to 0.05% of the volume in the LDS is attributable to leachate leakage at Landfill B. Based on this, the corrected liner efficiency could be greater than 99% for all units at Landfill B and four of the six units at Landfill T.
- At Landfill J, concentrations of chloride in the LDS are similar to those in the vadose zone (Figure 5-2). The site is located in an arid climate with little to no LCRS flow. In most cases, the LDS flow volumes, while minor, exceeded LCRS flow volumes. Based on the CF value, it appears that 20 to 40% of the total LDS flow may be attributable to primary liner leakage at this site.
- There appears to be a positive correlation between the number of data available to calculate E_c and the magnitude of the value calculated. In general, more data mean higher corrections, which should incentivize site operators to collect these data.

Table 5-5. Corrected liner efficiency, E_c

Design configuration	Climate condition	Primary liner system	Study unit	Correction factor (CF)	Corrected liner efficiency (E_c)	Number of concurrent data available
1	Wet	80-mil HDPE GM over 60-in CCL	B-1	0.002	99.93%	12
			B-2	0.005	99.89%	16
			B-3	0.002	99.97%	16
			B-4	0.004	99.76%	14
			B-5	0.003	99.49%	16
			B-6	0.001	99.98%	16
2	Wet	80-mil HDPE GM over 60-in CCL	B-7	0.004	99.87%	7
			B-8	0.0004	99.99%	12
3	Wet	60-mil HDPE GM over 36-in CCL	T-1	0.01	91.99%	5
			T-2	0.02	99.50%	7
			T-3	0.01	99.42%	8
			T-5	0.03	99.08%	8
			T-6	0.03	98.47%	13
			T-7	0.03	99.57%	17
4	Dry	60-mil HDPE GM over 18-in CCL	J-1	0.4	71.79%	3
			J-2	0.2	79.03%	2
			J-3	0.3	71.79%	1

6. ANALYSIS OF LEACHATE QUALITY DATA

6.1 Overview

Chapter 6 presents an analysis of leachate chemistry data as background for understanding long-term leachate management at closed Subtitle C landfills. As discussed in Section 1.5.4, leachate chemistry from hazardous waste (HW) landfills has received relatively little scrutiny in recent years, although some research at a number of landfills for containment of mixed LLRW and HW from the nine facilities included in this study, this may be due in large part to a paucity of data, particularly with regard to the chemistry of liquids recovered from LDS drainage layers. This is primarily due to two reasons:

- Leachate chemistry data are most commonly collected semi-annually or annually at each site, thereby limiting the overall size of the dataset available for analysis at each site; and
- The leachate constituent list monitored is dependent on site-specific waste history and local practices for leachate treatment and disposal, thereby limiting the number of similar constituents for which data were available at all sites.

Intra-unit comparisons (i.e., comparison of leachate chemistry between the LCRS and LDS in the same unit) are dependent on the same constituents being monitored on the same date, while inter-unit comparisons (i.e., comparison of leachate chemistry between different units and sites) are dependent on the similarity of the leachate analyte lists.

6.1.1 Data Availability

The availability of leachate constituents of interest to this study in the LCRS and LDS at each case study landfill is shown in Table II-7 in Appendix II. The leachate chemistry database is included in full in Appendix IV. Data for 30 analytes were sought, where available; selection of these analytes mirrored that of EPA (2002). In summary:

- Within the available leachate chemistry dataset, the LCRS has been sampled far more frequently than the corresponding LDS;
- The highest levels of data availability are at wet Landfills B and T and dry Landfill J;
- Very limited leachate chemistry data are available at non-arid Landfills P and F;
- No data are available for dry Landfill Y due to low liquid volumes and difficulty in sample collection in both the LCRS and LDS; and
- Monitoring of leachate chemistry in the LDS is not performed at dry Landfills R and M because liquid volumes are too small for samples to be collected.

In the remainder of this chapter, available leachate chemistry data are evaluated with a focus on estimating the time that may be required for the concentrations of constituents of interest to decrease to asymptotic levels during PCC.

6.1.2 Temporal Analysis of Leachate Quality

The chemical composition of MSW landfill leachate has been well studied, with comprehensive data collected over multiple years from several sites as summarized by Kjeldsen et al. (2002), SWANA (2004), and Öman and Junestedt (2008). Longitudinal studies of MSW leachate data (e.g., Gibbons et al., 2014) have shown that concentrations of dissolved organic matter indicators tend to decrease rapidly after landfill closure. Statom et al. (2004) evaluated over 12 years of MSW leachate data from a site in Florida and found an overall declining trend in major ion chemistry, with data collected after closure capping showing an overall reduction in the amplitude of short-term variations. Conservative inorganic ions such as chloride, however, are released in leachate over time by flushing. Therefore, limiting infiltration post-closure curtails their removal in leachate (Rowe, 1991). Several processes affect long-term concentration trends for VOCs, including volatilization to gas (Kjeldsen and Christensen, 2001), diffusive loss through cover geosynthetics (Foose et al., 2002), sorption to or desorption from waste, or leaching and degradation (Lowry et al., 2008).

The fate of trace metals under various landfill operating and internal biochemical conditions has been extensively researched in MSW landfills (Gibbons et al., 2014). As a landfill ages, pH tends to increase causing a decrease in metal solubility. Thus, although accumulation of humic acid concentrations over the long term may be expected to mobilize metals, elevated metals concentrations are not typically observed in leachate from well decomposed waste (Barlaz et al., 2002), in part because trace metals are strongly attenuated by in situ sorption and precipitation (Christensen et al., 2001). This is consistent with reviews of leachate data from multiple landfills (e.g., Kjeldsen et al., 2002; SWANA, 2004) which generally reported trace metal concentrations at or below federal maximum contaminant levels (MCLs).

Relative to the well-documented leaching behavior within MSW landfills as summarized above, hazardous waste materials may not degrade, or only degrade very slowly, under landfill conditions. While leachate chemistry at HW landfills may share many of the trends of MSW leachate, this has received relatively little scrutiny in recent years. Tian (2015) analyzed leachate composition from four landfills constructed for containment of mixed LLRW and HW in the United States and compared concentrations of dissolved organic matter (measured as TOC), inorganic macro-components (including major cations and anions), and trace metals to values reported in the literature for MSW leachate, concluding that:

- Dissolved organic matter concentrations were insignificant when compared with MSW leachate;

- Concentrations of inorganic macro-components were broadly similar to MSW leachate;
- Trace metal concentrations were relatively lower than in MSW leachate and tended to exhibit steady or slightly increasing trends.

If current expectations for the time required for the concentrations of constituents of concern in leachate to decrease to asymptotic levels or meet regulatory standards such as MCLs are rooted in observations from MSW landfills, this may not be appropriate. Therefore, an important component of this study is to review concentration trends in leachate from Subtitle C landfills.

6.1.3 Leachate Chemistry Constituents

Consistent with EPA (2002), 30 chemical parameters were selected to represent leachate constituents of interest for this study. These include the following categories of analytes:

- Water quality indicator parameters (pH, specific conductance, TDS);
- Macro indicators of dissolved organic matter (COD, BOD₅, and TOC);
- Major inorganic cations (calcium, magnesium, potassium, and sodium) and anions (alkalinity, chloride, and sulfate);
- Trace metals (arsenic, cadmium, chromium, lead, and nickel); and
- Trace VOCs frequently observed to be present in landfill leachate, represented by a group of 12 aromatic hydrocarbons and chlorinated solvents (and their degradation products).

The minimum, maximum, median, and arithmetic mean values are summarized for LCRS and LDS liquids in Tables 6-1 and 6-2, respectively. The number of reporting landfills and units is also listed, along with the total number of data and non-detect (ND) values. Finally, MCLs for public drinking water from 40 CFR Part 141 are also listed in the table, if available, for specific analytes. The use of MCLs as comparison values is justified by the fact that potential leachate releases would most likely occur to groundwater rather than surface water or other environmental media.

Table 6-1. Summary of LCRS leachate concentrations in case study landfills

Chemical constituents	Units	Concentration				Data availability				MCL ⁽²⁾
		Minimum	Maximum	Median	Mean	Landfills reporting	Units reporting	Total data	ND data ⁽¹⁾	
pH	s.u.	5.6	12.2	7.7	8.3	5	17	97	0	6.5-8.5 ⁽³⁾
Specific conductance	µmhos/cm	1,560	89,800	28,833	31,674	5	12	64	0	-
Total dissolved solids	mg/L	8,207	20,500	14,353	14,353	1	2	2	0	500 ⁽³⁾
COD ⁽⁵⁾	mg/L									-
BOD ⁽⁵⁾	mg/L									-
TOC ⁽⁵⁾	mg/L	337	10,579	3,803	4,641	2	9	48	0	-
Alkalinity	mg/L									-
Chloride	mg/L	53	34,320	9,290	9,865	5	20	249	2	250 ⁽³⁾
Sulfate	mg/L	20	10,488	4,850	4,740	3	8	61	0	250
Calcium	mg/L	0.4	23,467	390	1,668	5	20	227	2	-
Magnesium	mg/L	0.6	2,100	68	354	4	14	179	8	-
Sodium	mg/L	6	25,760	5,064	6,552	3	9	80	0	-
Potassium	mg/L	17	8,700	795	1,860	2	5	52	0	-
Arsenic	µg/L	0.01	234,333	223	11,951	7	25	265	9	10
Cadmium	µg/L	0.001	26,000	6	318	5	19	212	51	5
Chromium	µg/L	0.01	4,750	130	283	5	21	196	18	100
Lead	µg/L	0.001	1,790	33	87	6	23	226	75	15 ⁽⁴⁾
Nickel	µg/L	0.13	30,000	509	1,295	6	19	123	13	-
Benzene	µg/L	0.0002	48,600	10	537	6	21	271	69	5
1,1-Dichloroethane	µg/L	0.0015	474,000	8	6,319	5	20	270	61	-
1,2-Dichloroethane	µg/L	0.0003	104,000	9	1,573	5	19	253	115	5
cis-1,2-Dichloroethene	µg/L	0.0002	68,300	2	1,164	2	10	174	80	70
trans-1,2-Dichloroethene	µg/L	0.0004	2,440	7	66	5	19	263	135	100
Ethylbenzene	µg/L	0.002	1.7E+06	15	19,361	5	20	271	55	700
Methylene chloride	µg/L	0.002	4.3E+06	20	81,928	5	20	254	88	-
1,1,1-Trichloroethane	µg/L	0.0002	739,000	6	11,605	5	20	265	125	200
Trichloroethylene	µg/L	0.0003	671,000	6	10,404	5	21	263	99	5
Toluene	µg/L	0.002	8.7E+06	43	94,022	5	19	263	43	1,000
Vinyl chloride	µg/L	0.0001	150,900	10	2,548	5	20	266	112	2
Xylenes (total)	µg/L	0.002	5.7E+06	36	56,998	4	14	222	27	10,000

Notes:

- 1). ND = non-detect (values reported between the method detection limit and reporting limit).
- 2). MCL = Maximum contaminant level per 40 CFR Part 141.
- 3). SMCL = secondary maximum contaminant level.
- 4). Action level required if 10% of the water exceeded 15 µg/L.
- 5). COD = chemical oxygen demand, BOD = biochemical oxygen demand, TOC = total organic carbon.

Table 6-2. Summary of LDS liquids concentrations in case study landfills

Chemical constituents	Units	Concentration				Data availability				MCL ⁽²⁾
		Minimum	Maximum	Median	Mean	Landfills reporting	Units reporting	Total data	ND data ⁽¹⁾	
pH	s.u.	6.5	8.3	7.2	7.2	4	17	129	0	6.5-8.5 ⁽³⁾
Specific conductance	µmhos/cm	1,391	27,600	6,630	8,277	4	12	105	0	-
Total dissolved solids	mg/L	79	620	150	179	1	5	15	0	500 ⁽³⁾
COD ⁽⁵⁾	mg/L									-
BOD ⁽⁵⁾	mg/L									-
TOC ⁽⁵⁾	mg/L	3	63	8	13	3	12	64	0	-
Alkalinity	mg/L									-
Chloride	mg/L	2	3,000	22	286	3	17	251	0	250 ⁽³⁾
Sulfate	mg/L	134	10,000	5,100	4,642	2	6	58	0	250
Calcium	mg/L	110	3,048	400	713	3	10	98	0	-
Magnesium	mg/L	310	1,800	1,000	1,031	2	3	48	0	-
Sodium	mg/L	13	667	37	178	2	7	76	0	-
Potassium	mg/L	550	3,200	1,600	1,650	1	3	48	0	-
Arsenic	µg/L	2.5	773	11	34	5	17	128	30	10
Cadmium	µg/L	0.5	15	3	4	2	9	84	4	5
Chromium	µg/L	9	190	17	36	2	13	65	28	100
Lead	µg/L	3	897	9	27	4	16	115	24	15 ⁽⁴⁾
Nickel	µg/L	6	1,200	110	151	3	11	89	37	-
Benzene	µg/L	0.0002	208	1	5	4	18	327	141	5
1,1-Dichloroethane	µg/L	0.0003	355	4	18	3	18	327	104	-
1,2-Dichloroethane	µg/L	0.0003	750	1	34	3	17	315	135	5
cis-1,2-Dichloroethene	µg/L	0.0002	421	0.2	28	1	9	169	66	70
trans-1,2-Dichloroethene	µg/L	0.2	100	2	8	3	18	286	153	100
Ethylbenzene	µg/L	0.002	306	2	8	3	18	327	201	700
Methylene chloride	µg/L	0.002	176	2	5	3	17	315	139	-
1,1,1-Trichloroethane	µg/L	0.0002	162	1	6	2	18	327	210	200
Trichloroethylene	µg/L	0.0003	81	1	11	3	18	327	143	5
Toluene	µg/L	0.002	255	2	6	3	16	318	145	1,000
Vinyl chloride	µg/L	0.0001	4,948	1	68	3	18	328	151	2
Xylenes (total)	µg/L	0.002	100	2	9	2	11	278	175	10,000

Notes:

- 1). ND = non-detect (values reported between method detection limit and reporting limit).
- 2). MCL = Maximum contaminant level per 40 CFR Part 141.
- 3). SMCL = secondary maximum contaminant level.
- 4). Action level required if 10% of the water exceeded 15 µg/L.
- 5). COD = chemical oxygen demand, BOD = biochemical oxygen demand, TOC = total organic carbon.

As shown in the tables, there is significant variability in the data for many constituents, particularly in the LCRS where differences between maximum and minimum observed values often span six or more orders of magnitude for cations/anions, trace metals, and

VOCs. For this reason, the median is considered more representative of overall leachate quality in both the LCRS and LDS than the mean, which is more sensitive to one or two significant outliers. The general water quality characteristics of liquids from the LCRS and LDS drainage layers are also significantly different, again by multiple orders of magnitude in many cases. This strengthens previous findings in Chapter 5 that additional sources of liquids rather than simply primary liner leakage are contributing to the liquid volumes measured in the LDS.

In the remainder of this section, select leachate data representing the five major categories of analytes of interest are reviewed. The purpose is twofold:

- Estimate when concentration trends may be asymptotic, and
- Compare concentrations to limit values (drinking water MCL or secondary MCL [SMCL]), where available.

In the latter regard, the focus of the comparison is LCRS concentrations as this represents characteristics of source leachate in the landfill. However, it is important to recognize that any potential leakage from the landfill to the subsurface will occur via the LDS since this underlies the LCRS across the base and side slope areas of the landfill. As such, LDS concentrations are of more significance than LCRS concentrations in an environmental setting, assuming the relationship between LCRS and LDS concentrations is understood and remains stable over the long term. In addition, rather than directly comparing leachate concentrations to a limit value, a universal dilution/attenuation factor (DAF) of 20 was applied to represent expected concentrations at the point of compliance (e.g., monitoring well) rather than in source leachate. This is consistent with the default DAF specified in the **EPA's Soil Screening Guidance (EPA, 1996)**. As a potential release of leachate moves through soil and groundwater, constituent concentrations are attenuated by sorption, degradation, and dilution in clean groundwater. A DAF of 20 is deemed appropriate for contaminant sources up to 0.5 acres in size. While landfills are generally much larger than that, the potential release points (i.e., potential pinhole defects and tears in liner GM barriers) are collectively much smaller than 0.5 acres. As such, this approach is appropriate for the purposes of this portion of the study, which is to investigate whether leachate concentrations are exhibiting downward trends and/or reaching asymptotic levels that meet applicable limit values. Nevertheless, it is important to note that assigning this default DAF in this study does not imply any endorsement from EPA with regard to the universal application of this approach to assessing long-term leachate management and groundwater monitoring at Subtitle C landfills.

In keeping with the above-stated goals, the focus of the discussion presented in the next subsections is on general trends in the composite concentration data rather than absolute values for individual units at specific times. Therefore, the data selected for presentation are not identified by individual units, although the colors and shapes of data points direct the reader to commonality amongst datasets with regard to the represented landfill and drainage layer, respectively.^{4F5} Insufficient flow data and waste manifests were available to accurately review leachate chemistry in terms of contaminant removal loads (e.g., cumulative mass of contaminant removal per ton of waste in place), although this would potentially have been advantageous in terms of normalizing the data between the different study units, climatic conditions, and cover/liner design configurations. No statistical analysis was conducted to eliminate outliers, test the significance of trends, assign correlation coefficients to trending data, or calculation of confidence limits. No correlations between leachate chemistry and flow rates were investigated, so it is not known whether the goal of excluding liquids from RCRA landfills in the post-closure period contributes to chemical changes or whether cover design and performance has a direct effect on leachate quality. Interested readers can review specific leachate flow and chemical characteristics from the blinded raw data provided in Appendices II and III.

6.1.4 Water Quality Indicators

pH

Leachate pH in the LCRS varied from 5.6 to 12.2 s.u., with a median value of 7.7 s.u. Temporal variability in the data is plotted in Figure 6-1. For the most part, readings are within the 6.5 to 8.5 s.u. the range specified under 40 CFR Part 141. If data from the first 5 years of PCC from Landfill T are ignored, the pH range would narrow further to 6.5 to 10 s.u, which is broadly consistent with the range of 7.6 to 9.4 s.u. and average 8.2 s.u. reported for HW landfills by EPA (2002). Temporal trends in pH appear stable based on the data plotted for each facility.

Most readings, particularly over the longer term, are above neutrality, which suggests that leachate at Subtitle C sites should be expected to be in the alkaline range. One possible explanation for the high pH values could be the relatively widespread practice of solidifying and stabilizing hazardous waste with cement, fly ash, or kiln dust prior to landfill disposal.

⁵ In all plots presented in Section 6.2, circular markers (●) are used to present LCRS data while triangular markers (▲) present LDS data. Markers are color-coded with a unique color representing each case study landfill, with colors grouped between geographic regions (SE = green, NE = blue, SW = red/pink, NW = orange/yellow).

Based on discussions with operators at case study sites, it appears that such stabilizers may occupy as much as 40% of the total airspace volume. The potential implications of an alkaline medium on limiting trace metals mobilization from the waste are important, as discussed in Section 6.2.4.

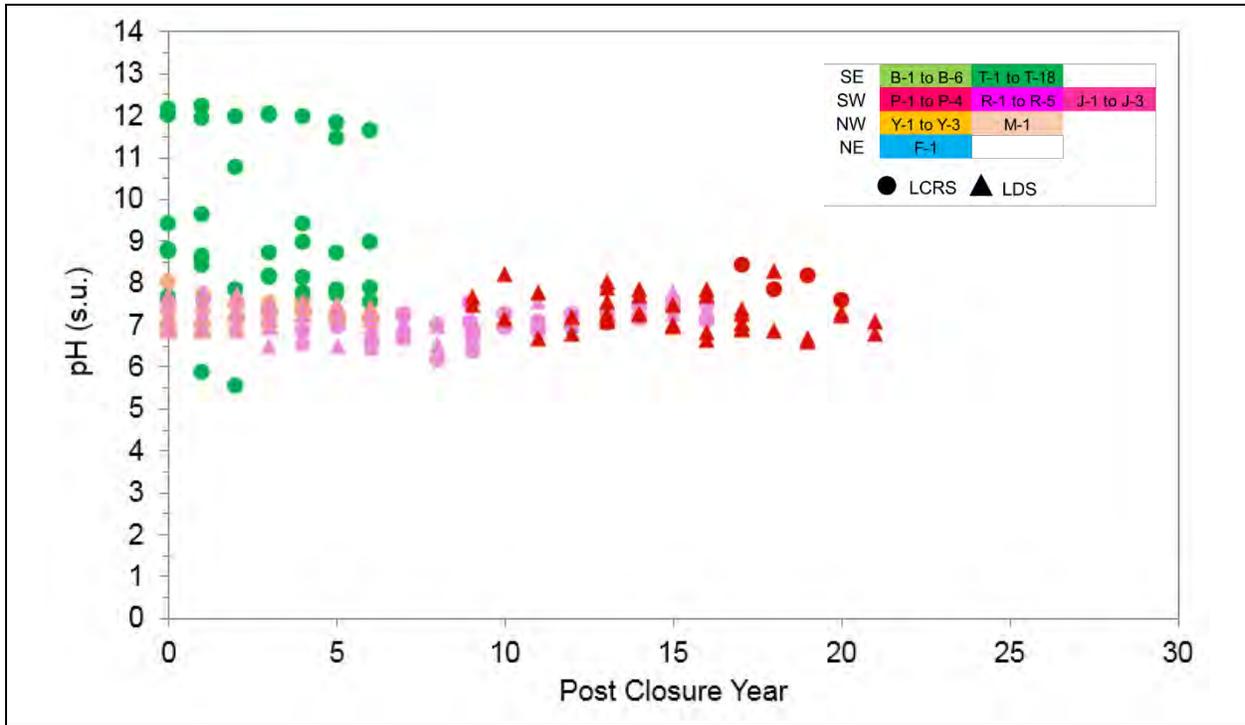


Figure 6-1. Temporal variability in pH

Specific Conductance

The specific conductance (or conductivity) of a solution is a measure of its ability to carry an electric current, which provides an estimate of the dissolved solids in solution. The mean conductivity of LCRS leachate samples from the case study units was about 31,700 $\mu\text{mhos/cm}$, with a median of 28,800 $\mu\text{mhos/cm}$, which is higher than the mean of about 22,100 $\mu\text{mhos/cm}$ from three HW sites reported by EPA (2002) and substantially higher than the mean of 250 to 3,500 $\mu\text{mhos/cm}$ reported for MSW landfills by Öman and Junestadt (2008). Values in the LCRS and LDS are of the same order of magnitude (Figure 6-2).

Overall, conductivity measurements are highly variable with no evidence of asymptotic behavior. A number of datasets (notably Landfill P) are exhibiting apparent upward trends with conductivity values approaching 100,000 $\mu\text{mhos/cm}$.

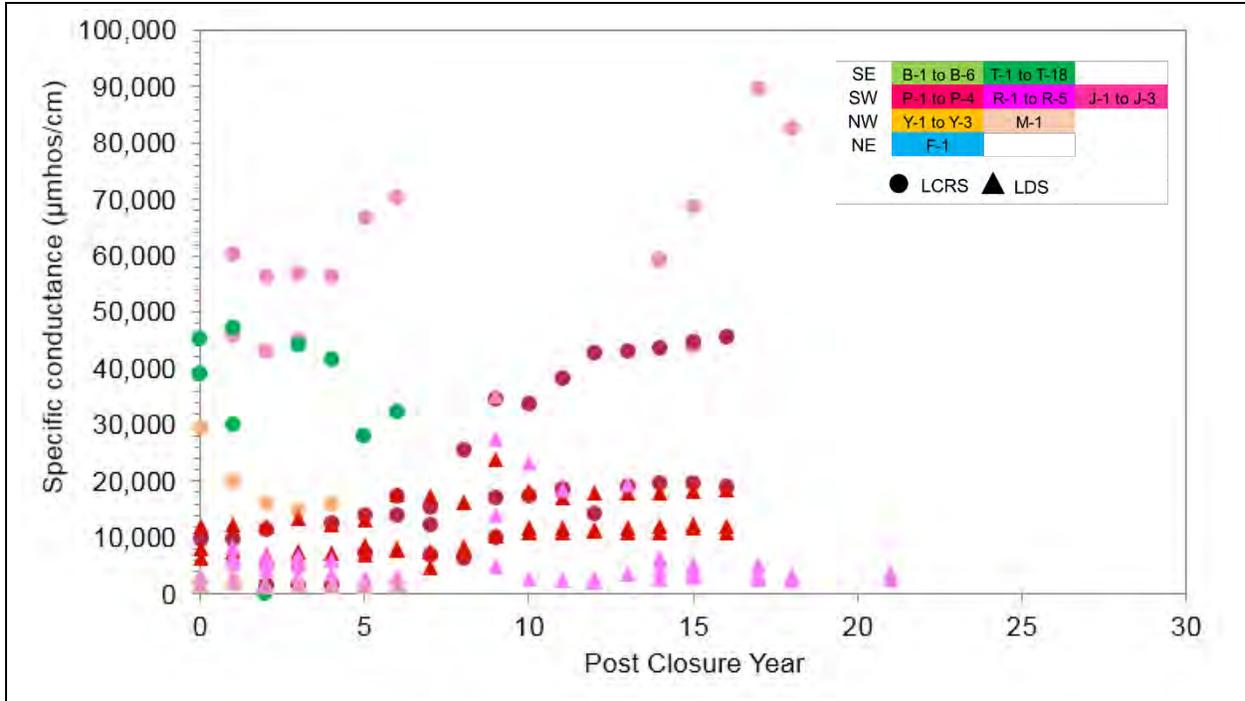


Figure 6-2. Temporal variability in specific conductance

Total Dissolved Solids (TDS)

Limited TDS data were available for the study and are not plotted, although it is expected that TDS trends would correlate closely with specific conductance in which case no evidence of declining or asymptotic behavior would be expected. TDS analysis was conducted on samples from the LCRS at Landfill R and the LDS at Landfill B. LCRS concentrations for two data points at Landfill R were 8,200 and 20,500 mg/L (mean 14,350 mg/L). Based on applying a DAF of 20 to the SMCL limit value of 500 mg/L, the mean is above but consistent with the modified benchmark of 10,000 mg/L. Overall concentrations in the LDS are one or two orders of magnitude lower than the LCRS.

6.1.5 Dissolved Organic Matter

Dissolved organic matter (DOM) in leachate is composed of a variety of constituents that are collectively expressed in terms of biochemical oxygen demand (BOD) and chemical oxygen demand (COD). A higher oxygen demand generally implies a higher organic loading, even though other constituents such as ammonia and nitrogen also exert an oxygen demand; therefore, COD and BOD are useful indicators of pollution potential posed to receiving systems such as surface water bodies. BOD is generally considered as a measure of biologically degradable organic materials and is measured as oxygen consumption over five days, hence the suffix BOD₅ often used in the annotation for this parameter. COD, on the other hand, also includes recalcitrant compounds that are not easily biologically

degradable; as such, COD concentrations are generally substantially higher in leachate than BOD concentrations. BOD and COD data are not collected at any case study site.

A measure of some of the constituents of DOM is provided by total organic carbon (TOC). TOC has been analyzed on a handful of occasions in both the LCRS and LDS at Landfills P and T. Values in Table 6-1 range from about 340 to 10,580 mg/L with a median concentration of 3,800 mg/L, which compares reasonably close to the mean of about 1,620 mg/L reported for two HW sites by EPA (2002). Mean and median TOC concentrations in the LDS are two to three orders of magnitude lower than in the LCRS. The maximum TOC concentration in MSW leachate reported by Kjeldsen et al. (2002) was 29,000 mg/L.

In summary, DOM data do not appear to be routinely analyzed in leachate at Subtitle C facilities. COD and BOD data were not reported for any HW landfill in the study by EPA (2002). The limited data available for this study generally support the finding by Tian (2015) that DOM concentrations are insignificant in HW leachate when compared with MSW leachate (although this does not imply that DOM concentrations in HW leachate are insignificant with respect to potential impacts to HHE). No MCL or SMCL is specified for any constituents of DOM listed above. Overall, beyond complying with potential influent limitations on COD/BOD imposed by off-site wastewater treatment facilities, it is assumed that concentrations of DOM in leachate are of little interest to Subtitle C landfill operators.

6.1.6 Major Cations and Anions

With the exception of alkalinity, which is not analyzed at any case study landfill, data for the other major cations (calcium, magnesium, potassium, and sodium) and anions (chloride, and sulfate) were available in both the LCRS and LDS at a number of case study sites. Given the nature of HW, they are also expected to be present in significant concentrations in leachate. Tian (2015) reported that concentrations of cation and anions in HW leachate are broadly similar to MSW leachate. These parameters offer a meaningful opportunity to estimate the time to reach asymptotic levels and/or MCL/SMCL limit values, which are specified for sulfate (MCL = 250 mg/L) and chloride (SMCL = 250 mg/L). Using a DAF of 20, as previously discussed, the modified benchmark for comparison of sulfate and chloride concentrations to limit values is 5,000 mg/L.

Chloride

As presented in Table 6-1, leachate chloride concentrations ranged widely from about 50 to 34,320 mg/L, with similar mean and median concentrations of 9,870 mg/L and 9,290 mg/L, respectively. This is higher than the range of 150 to 4,500 mg/L reported for MSW landfills

by Kjeldsen et al. (2002). The mean LCRS chloride concentration reported for two HW landfills by EPA (2002) was 11,700 mg/L, which suggests that chloride concentrations have remained fairly constant in HW leachate over the intervening period. These values exceed the modified benchmark by a factor of about two. However, the temporal plot of LCRS chloride data (Figure 6-3) indicates that many datasets exhibit no downward trend or are even trending upwards, although several data points are below the modified benchmark (shown as the dashed line in the figure). Overall, it cannot be concluded that LCRS chloride concentrations should be expected to routinely meet the modified benchmark within 30 years of closure. However, LDS concentrations routinely exhibit stable trends and are all below the modified benchmark (most LDS concentrations meet the SMCL directly).

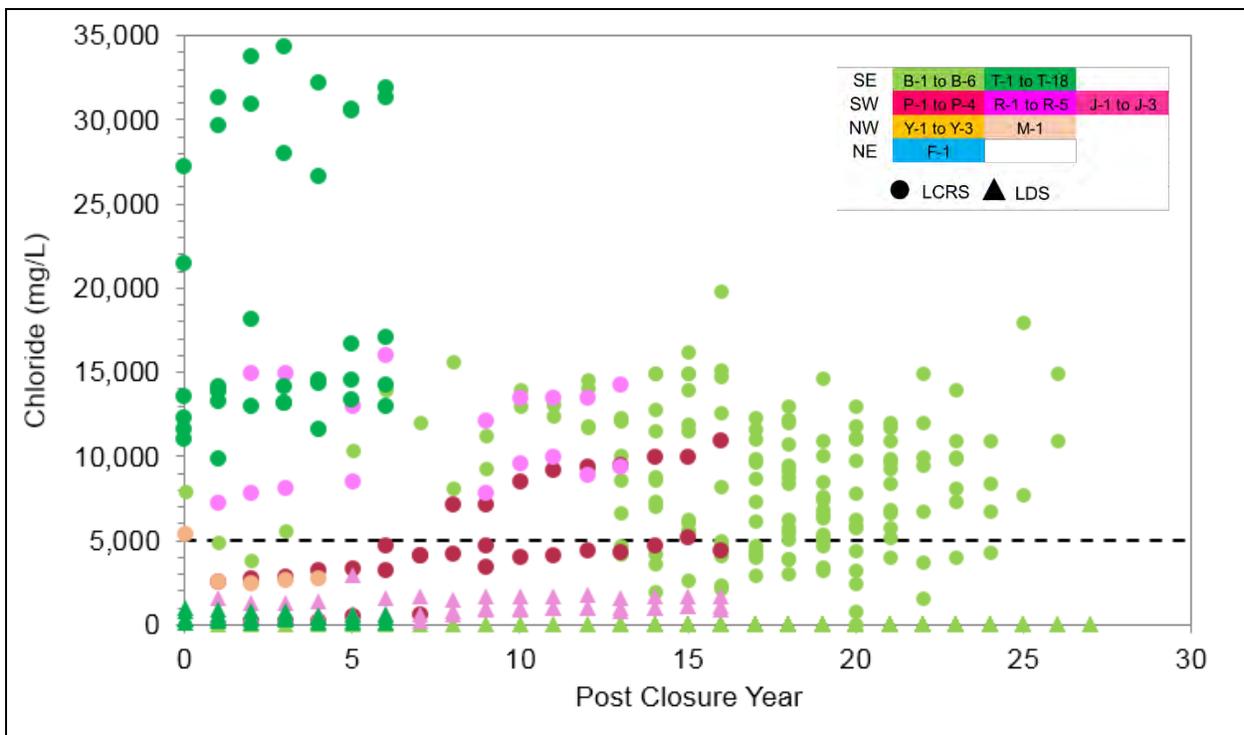


Figure 6-3. Temporal variability in chloride

Sulfate

Sulfate data availability at the case studies is limited to Landfills T, J, and R (Figure 6-4). LCRS concentrations range widely from 20 to 10,500 mg/L, although the median and mean values are similar at around 4,800 mg/L, which suggests the variability is limited to a few outliers. The mean/median concentration is slightly below the modified benchmark of 5,000 mg/L. The mean sulfate concentration reported for two HW landfills by EPA (2002) was below 3,000 mg/L. Concentrations of sulfate in the LDS are very similar to those in the LCRS. The median and mean concentrations in the LDS are 5,100 mg/L and 4,650 mg/L,

close to the modified benchmark of 5,000 mg/L. There is no evidence of trending to asymptotic levels in either the LCRS or LDS dataset presented in the figure.

It is noted that the reasons why sulfate should persist at high concentrations in HW leachate are not well understood. It is possible that the waste contains a sulfate source such as gypsum-containing wallboard, although it is not known why significant quantities of this material would be disposed of in an (expensive) Subtitle C landfill rather than an MSW or a construction and demolition debris landfill facility. Sulfate would typically be reduced to sulfide under the anaerobic conditions assumed to be prevalent in closed landfills (Plaza et al., 2007). For example, Kjeldsen et al. (2002) reported sulfate concentrations in the range of 10 to 420 mg/L in methanogenic leachate. It is speculated that HW landfills have much lower DOM availability and thus reduced microbial activity relative to MSW landfills, and redox conditions may not reach a sulfate-reducing environment (i.e., Eh below -250 mV).

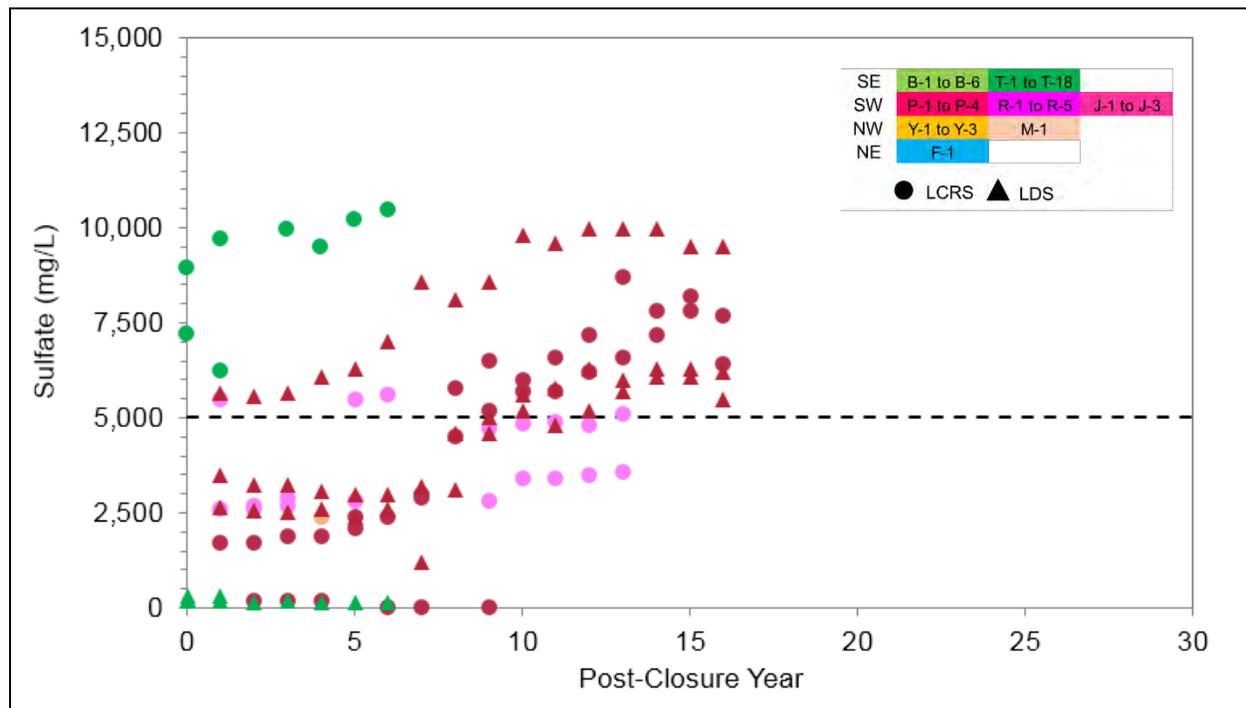


Figure 6-4. Temporal variability in sulfate

Ionic Charge Balance

Schoeller diagrams (absent alkalinity) were constructed to concurrently illustrate the concentrations of six cations and anions in LCRS leachate at Landfill R (Figure 6-5). Data are collected separately for R-1 and as a combined sample for R-2 to R-5. The figure provides a geochemical "fingerprint" of data collected over 13 years of PCC, with each line on the two graphs representing a single year. With the exception of sodium, and to a lesser extent chloride, the very close bunching of lines in the figure shows there has been no

significant change in overall geochemical makeup of leachate over the 13 years of PCC. Although not discernable from these graphs, the variability in sodium, chloride, and sulfate values represents concentration fluctuations that are apparently random on a year-on-year basis rather than evidence of trend behavior.

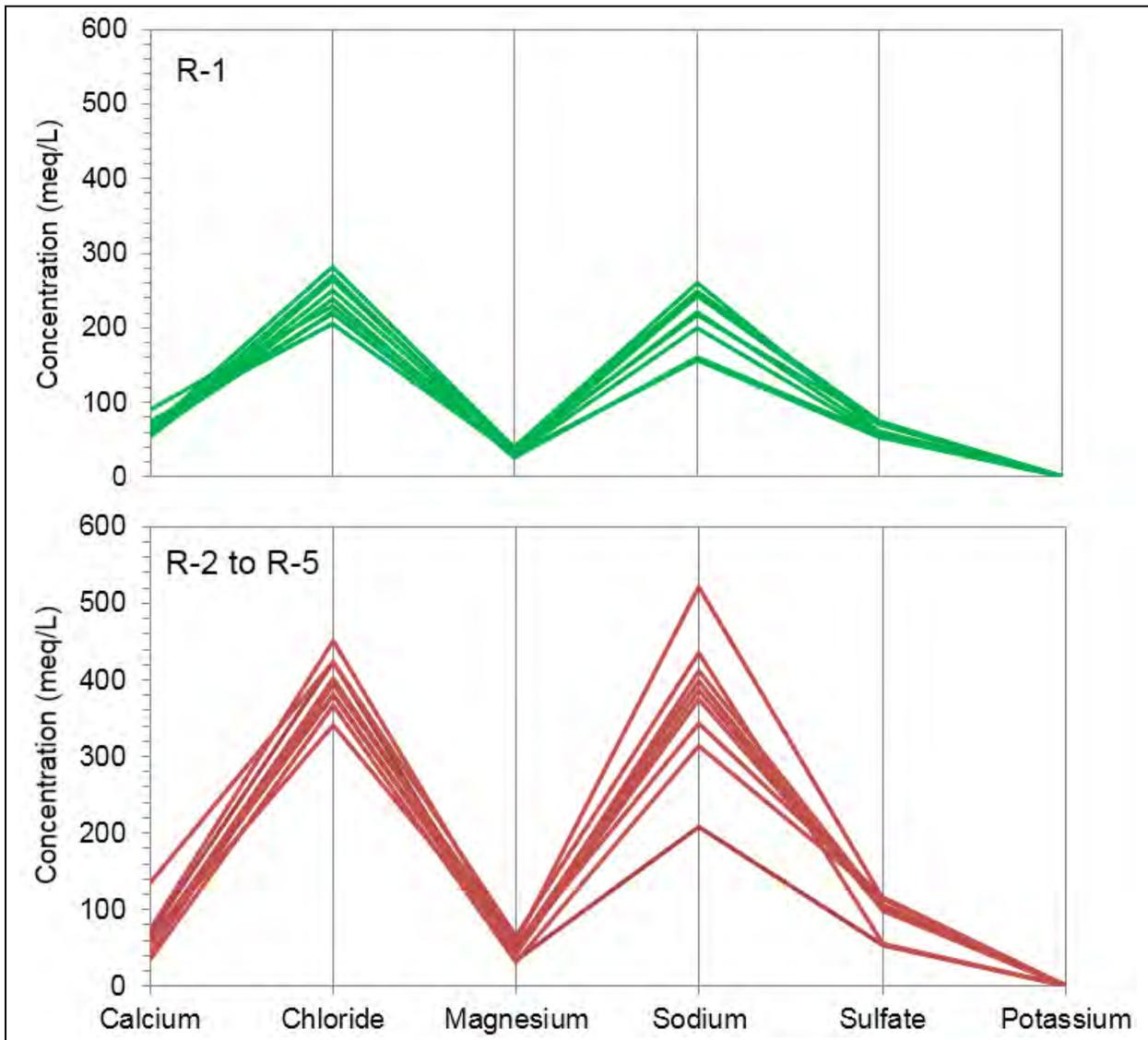


Figure 6-5. Schoeller diagrams for cations and anions, Landfill R

6.1.7 Trace Metals

Trace metals selected for this study comprise arsenic, cadmium, chromium, lead, and nickel. These five analytes were selected based on their inclusion in EPA (2002). As shown in Table 6-1, MCLs are specified for arsenic, cadmium, and chromium while a limit value for lead is provided as an action level. Using a DAF of 20 as previously discussed, modified

benchmarks can be established **for arsenic (200 µg/L), cadmium (100 µg/L), chromium (2,000 µg/L), and lead (300 µg/L).**

Review of data from EPA (2002), which reported on leachate quality from three HW landfills, indicates that the mean concentration of all five trace metals in that study was similar (within the same order of magnitude) but slightly lower than the corresponding values reported in this study. This may reflect the reduced practice of co-disposing of MSW and other non-hazardous wastes with HW in the intervening period, as MSW has been shown to offer significant buffering capacity for adsorption of trace metals (Gibbons et al., 2014). Overall, however, the long-term similarity in concentrations over a long period suggests that trace metals in leachate may be relatively stable or increase only slightly over time. This is consistent with findings reported by Tian (2015) that trace metal concentrations were relatively lower in HW landfills than in MSW leachate and tended to exhibit steady or slightly increasing trends. Concentrations of trace metals in the LDS are universally lower than in the LCRS and typically meet limit values without the need for DAF modification.

Arsenic

Arsenic data are highly variable with no evidence of asymptotic behavior, particularly in the LCRS dataset (Figure 6-6, note log scale on y-axis). The variability observed is not **dissimilar to the range of 10 to 1,000 µg/L reported for MSW landfills (Öman and Junestadt, 2008).** Although a significant proportion of the data are below the modified benchmark limit **of 200 µg/L** (indicated by the dashed line in the figure), the relatively high concentrations of arsenic that persist long into the PCC period may be due to the relatively alkaline pH of leachate, potentially attributable to the practice of solidifying and stabilizing HW with cement, fly ash, or kiln dust prior to disposal. Arsenic is generally more mobile under alkaline conditions (Smedley and Kinniburgh, 2002). Another observation that is evident from inspection of the figure is the clustering of data according to sites. This suggests that arsenic concentrations in HW leachates are highly site-specific and dependent on the cover soils used and/or sources of waste placed in a particular unit.

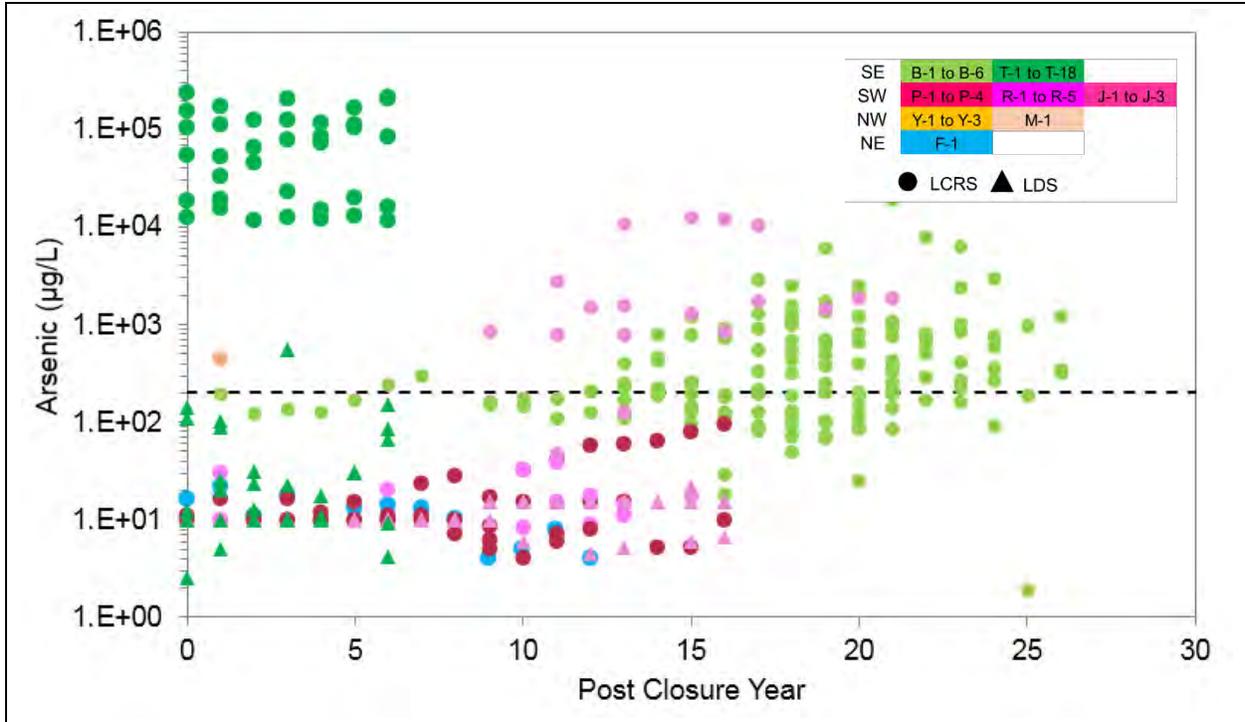


Figure 6-6. Temporal variability in arsenic

The marked difference between mean and median values of arsenic in the LCRS dataset suggests that a few data are skewing the overall results; as such, the median value (220 µg/L in the LCRS, 11 µg/L in the LDS) is considered a better representation of leachate quality. The median concentration of arsenic in LCRS leachate slightly exceeds the modified benchmark.

Total Chromium

Chromium (as total chromium) data from the case studies are highly variable with concentrations ranging over six orders of magnitude. The data do not suggest a trend to asymptotic behavior (Figure 6-7), although almost all data are below the modified benchmark limit of 2,000 µg/L indicated by the dashed line in the figure. The majority of data are not dissimilar to the range of 20 to 1,500 µg/L reported for MSW landfills (Öman and Junestadt, 2008).

The similarity between mean (282 µg/L) and median (130 µg/L) concentrations of chromium in leachate suggests that the variability in data is extensive and not limited to a few data points. The median value in the LCRS (130 µg/L) slightly exceeds direct comparison to the MCL of 100 µg/L but easily meets the modified benchmark. The median value in the LDS (17 µg/L) is slightly above the MCL. The low concentrations of chromium in

leachate may be due to the relatively alkaline pH, which may be attributable to the practice of solidifying and stabilizing HW with cement, fly ash, or kiln dust prior to disposal.

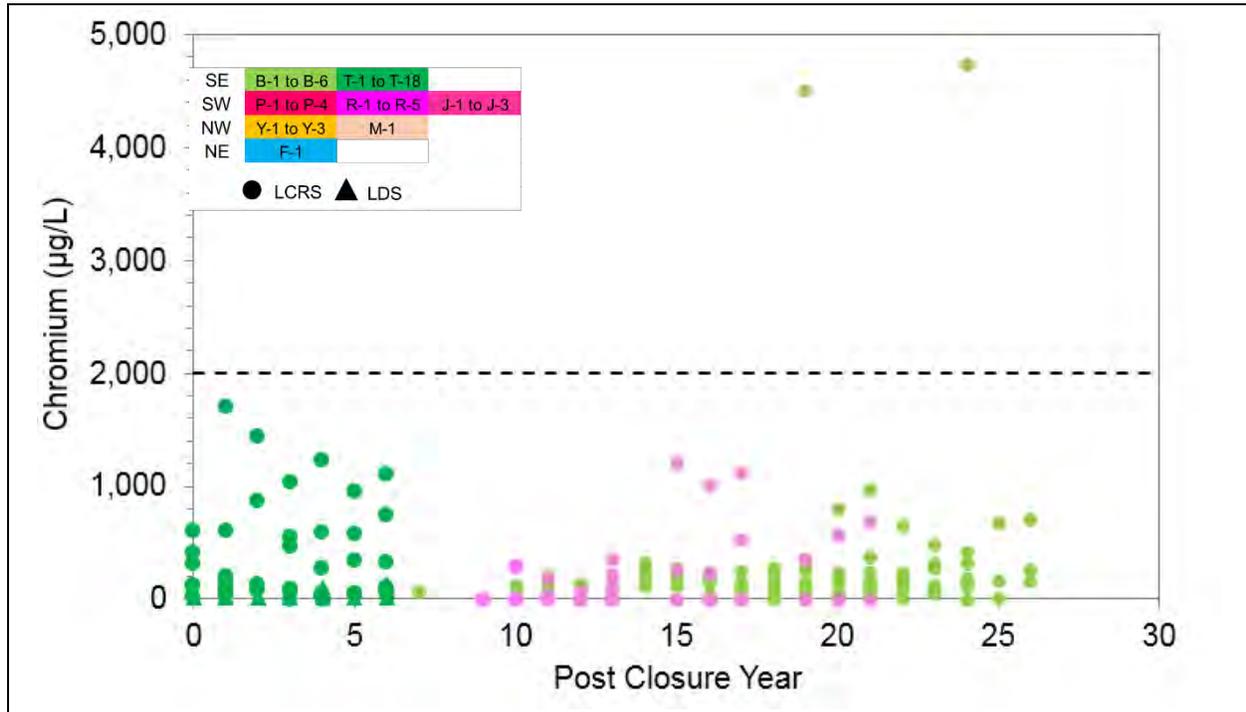


Figure 6-7. Temporal variability in total chromium

Lead

Similar to chromium data, lead concentrations in leachate are highly variable with no evidence of asymptotic behavior (Figure 6-8). However, with the notable exception of recent data from Landfill B, most data are below the modified benchmark indicated by the dashed line in the figure. Concentrations are generally below the upper-bound value of 5,000 µg/L reported for MSW leachate by Öman and Junestadt (2008).

Like chromium, the similarity between mean and median values of lead in leachate suggests that the variability in data is extensive. The median value in the LCRS (33 µg/L) slightly exceeds direct comparison to the action level of 15 µg/L, while the median value in the LDS (15 µg/L) equals the action level. Overall, this appears promising in terms of trace metal concentrations in leachate from Subtitle C landfills meeting acceptable limit values within the presumptive 30-year PCC period. The low concentrations of lead in leachate may again be due to the relatively alkaline pH.

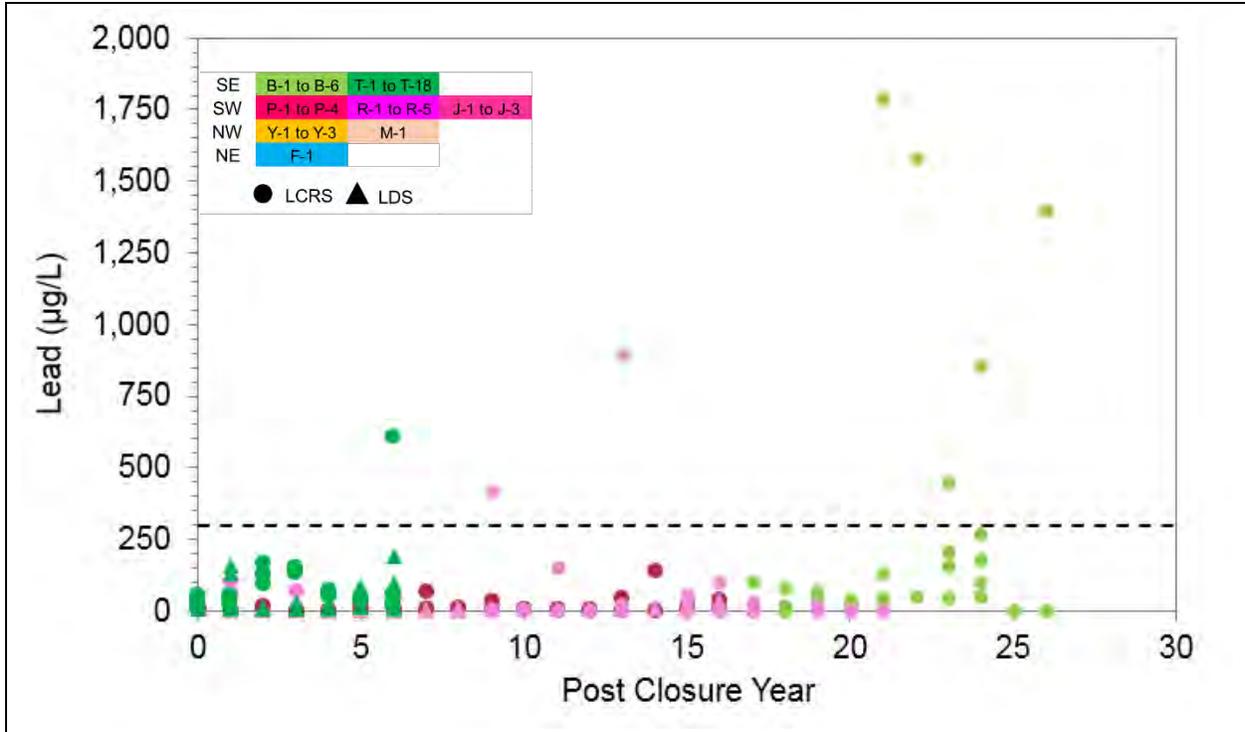


Figure 6-8. Temporal variability in lead

6.1.8 Volatile Organic Compounds

For this study, trace VOCs—represented by a group of 12 aromatic hydrocarbons and chlorinated solvents (and degradation products) frequently observed to be present in landfill leachate—were selected as constituents of interest. Of all the groups of chemical constituents considered in this study, VOCs exhibited the most variability in concentration between different landfill units, and sometimes between different sumps within the same unit.

As indicated in Table 6-1, the variability among the LCRS data is significant for all VOCs, with reported concentrations ranging over six or more orders of magnitude. LDS data exhibit far less variability than LCRS, with similar median and mean concentrations for all but one VOC (trichloroethylene). The wide discrepancy between median and mean values in the LCRS suggests that the data are skewed by a few outliers; as such, the median is **considered to be more representative of “typical” concentrations. This is supported by** comparison of LCRS data to values for VOCs reported for MSW leachate. Median values of all VOCs from this study fall within the concentration range reported by Kjeldsen et al. (2002) and Öman and Junestadt (2008) but mean values significantly exceed the reported range for MSW leachate.

MCLs are specified for all but two of the VOCs selected. The median value of VOCs in the LCRS in this study only exceeds the MCL in four out of ten cases, although the four failing parameters easily meet the DAF-adjusted benchmark. The median LDS concentration directly meets the MCL in eight of the ten cases, the failures being trichloroethylene and vinyl chloride. Again, these two parameters easily meet the DAF-adjusted benchmark.

Review of data from EPA (2002), which it should be noted included data from only two HW landfills, indicates that mean concentration of all VOCs were slightly higher in that report than the median values calculated in this study. This may be suggestive of an overall downward trend in the VOC content of HW in the intervening years.

Temporal variability and trends within the VOC category of leachate constituents are illustrated using benzene (Figure 6-9, note log scale on y-axis). Overall, the data are somewhat variable, although with the exception of Landfill B there is evidence of downward trending behavior. A significant number of data are below the DAF-adjusted benchmark (20 × MCL), which is indicated by the dashed line in the figure.

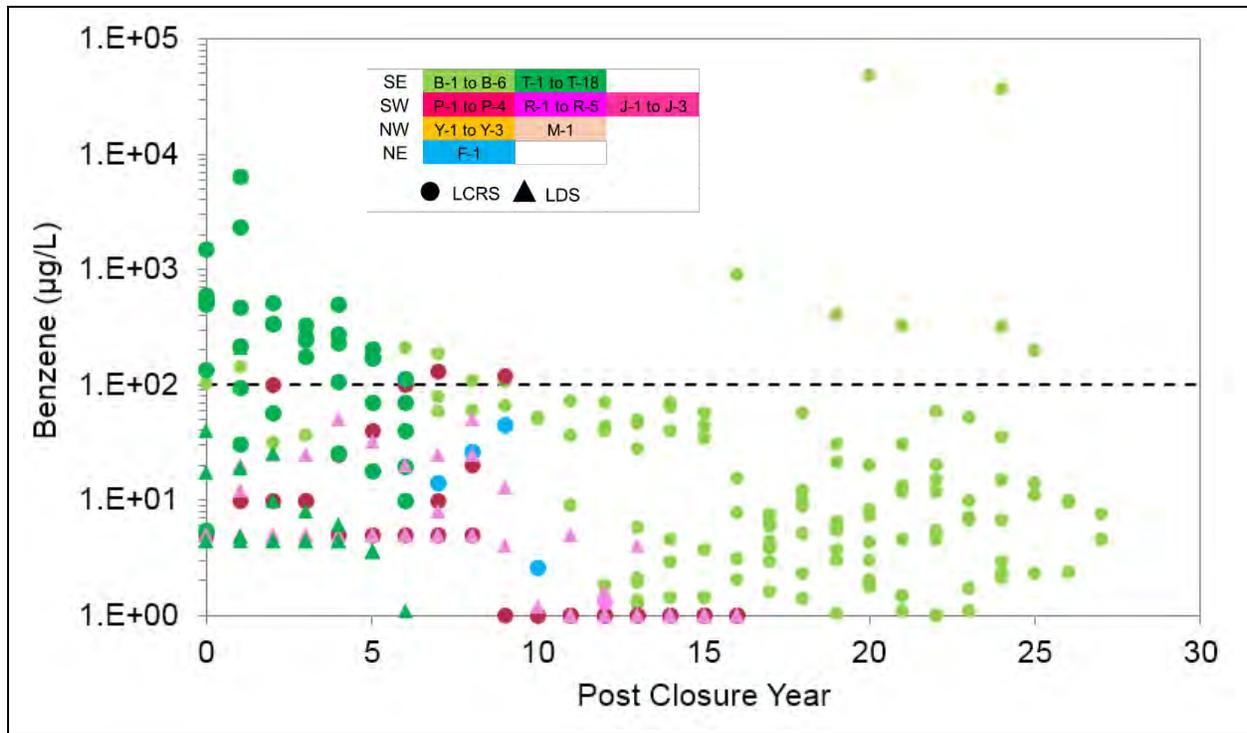


Figure 6-9. Temporal variability in benzene

7. SUMMARY AND CONCLUSIONS

7.1 Study Implications on Understanding Long-Term Landfill Performance

The purpose of this section is to summarize findings from analysis of liquids management data for double-lined hazardous waste landfills located throughout the United States. Specifically, LCRS and LDS flow rate and flow chemistry data were evaluated for 45 landfill units at nine closed Subtitle C sites. Design and operational characteristics of the study units were summarized in Chapter 3 of this report along with post-closure data availability. These data were used to evaluate:

- Long-term trends in leachate generation rates;
- Leakage rates and hydraulic efficiency of primary liner systems; and
- Leachate chemistry data, including time for constituents of interest to degrade and potentially reach asymptotic levels or water quality limit values.

The primary aims of this assessment are to understand whether existing regulations for containment system design and performance evaluation at Subtitle C landfill facilities are appropriate and what main issues or shortcomings in data collection are evident that could be addressed.

7.1.1 Leachate Flow Rate and Trends

The focus of this discussion is to address the following two key research questions: How much leachate is generated in closed Subtitle C landfills and what are the potential effects of site location (climatic region), cover system design and construction, facility operation or waste type, and other factors on leachate generation rates? How do predictions of leachate generation using the HELP Model compare to observed generation rates at these sites?

General Observations Based on Field Data

Flow data in the LCRS and LDS at the case study sites were summarized in Section 4.1. In general, LCRS and LDS flow volumes declined soon after closure with a steady or decreasing trend behavior thereafter. Discrepancies or short-term deviations from general trends were generally attributable to known O&M issues affecting cover system performance. For example:

- Landfill B: A spike in the LCRS flow rate from unit B-6 was attributed to a localized cap failure. This has since been repaired and the flow rate has returned to similar levels as recorded for other units at the landfill.

- Landfill T: Unit T-16 exhibited an increase in LCRS flow rate between years 7 and 16 of PCC that was finally traced to a localized cover system failure. The cover was repaired in year 16, which resulted in rapid reduction in LCRS flow to rates similar to other units.
- Landfill D: Liquid volumes in the LDS for D-2 were observed to increase for the first 4 years after closure before exhibiting a declining trend thereafter. This cell had a short operational period of only 1 year, during which time the contractor reported difficulties in fully eliminating rainwater from the unit during construction and waste filling. As such, LDS volumes could be attributable to the potential this excess water contributing to construction, compression, and consolidation water rather than leakage through the primary liner. The LDS flow volumes are anticipated to continue to lessen as this excess water works its way out of the unit.

Erosion damage to cover systems is a key factor affecting landfill performance in the PCC period, with higher costs and effort associated with repairs needed during initial years of PCC before cover vegetation is fully established which helps stabilize the system. Breaches in the cover system may result in relatively long-term setbacks in terms of returning LCRS flow rates in affected landfill units to expected levels once the cover is repaired. In this regard, it may be beneficial to maintain hydraulic separation between the liners of different units, and potentially minimize the size of individual units which may assist in isolating the impacts of a potential cover damage.

This further highlight the importance of routine cover inspection in identifying problems related to erosion damage, water ponding on the cover system, or other issues, as this facilitates timely maintenance and repair to minimize the likelihood of water seeping back into the cell. Furthermore, as weather patterns seem to be changing, it is advisable that routine inspections be supplemented with non-routine cover ones following an extreme weather (e.g., flood, excessive precipitation, drought, or tsunami) or seismic event. The largest challenge at one of the examined sites was erosion control and protection of the cover specifically due to high rainfall following a long spell of dry weather, particularly in the first few years of closure before cover vegetation has matured.

Potential Factors Affecting Leachate Flow in the LCRS and LDS

All liner and cover system construction at the 45 landfill units included in this study was performed under a program of third-party COA; therefore, the potential effects of construction in the absence of COA cannot be examined.

The potential effects on relative LCRS and LDS flow were qualitatively examined with regard to several variables as summarized in Table 7-1, including:

- Time in post-closure;

- Climatic and hydrogeologic conditions at the site (average annual rainfall, average annual snowfall, depth to groundwater, and types of subsurface soils); and
- Containment system design and material specifications (cover type, primary liner type, and secondary liner type).

Consistent with previous report sections, the table is structured around the 13 unique configurations for the containment system design that are represented amongst the 45 study units. Although it is important to recognize the limitations in the small sample size and non-random nature of the case studies, the following observations are made:

- The table clearly illustrates that rainfall has an effect on leachate generation, with higher LCRS flows recorded at the four wet sites (Landfills B, T, D, and F) and very low or negligible flows recorded at dry sites (Landfills J, R, and Y). Landfill M is excluded from this comparison since only intermediate cover had been installed at this site. It is noted also that Landfill J is a special case at which a landfill overfill liner is integrated with the cover; as such, leakage through the cover system would be expected to be very low, independent of rainfall. Overall, the incidence of precipitation as rainfall versus snowfall does not appear to affect leachate generation.
- Ignoring Landfills J and M as unrepresentative, the performance of three different final cover system designs was evaluated as part of this study. Six units at Landfill B feature a reversed CCL/GM cover design, while nine units at four sites (Landfills P, Y, D, and F) feature a composite GM/GCL rather than GM/CCL cover barrier design at the other 27 units. For final cover systems, the units in this study with the GM/GCL design had slightly better overall performance in terms of flow reduction than GM/CCL. The reversed CCL/GM design was leakiest for the units evaluated here.
- In all cases, placement of cover leads to a reduction in the LCRS flow rate, including Landfill M which has only 12-inches of intermediate cover soil in place. Although LCRS flows are an order of magnitude higher at Landfill M than the other three dry sites.
- In general, LDS flows are affected by LCRS flows (as would be expected). But, depth to groundwater may significantly affect relative LDS flows, with higher flows recorded at wet sites with shallow groundwater (Landfills B, T, D, and, to some extent, P). Hydraulic connection between groundwater and the secondary liner has not been established at any site. Thus, it cannot be concluded that groundwater rather than infiltration of rainwater is the main alternative source of LDS liquids.
- Four sites (Landfills P, Y, D, and F) feature a GM-only barrier in the primary liner, while all other designs feature a composite GM/CCL barrier. With the exception of Landfill P, the primary liner design also does not appear to affect LDS flows.

Table 7-1. Summary of potential factors affecting relative flow in the LCRS and LDS

Design configuration	Study unit(s)	Years in post-closure	Site details				Containment system design ⁽¹⁾⁽²⁾			LCRS flow ⁽³⁾				LDS flow ⁽³⁾			
			Geographic Region	Annual precipitation (in.)	Depth to groundwater (feet)	Subsurface soil type	Cover barrier design	Primary liner barrier design	Secondary liner barrier design	>100 gpad	11-100 gpad	1-10 gpad	<1 gpad	>10 gpad	1-10 gpad	0.1-1 gpad	<0.1 gpad
1	B-1 to B-6	22-29	SE	47	<5	Claystone	CCL/GM-H30	GM-H80/CCL	GM-H80/CCL		3	3			4	2	
2	B-7 and B-8	11-17					GM-H60/CCL			1		1		2			
3	T-1 to T-18	7-23	SE	70	<5	Sandy silt, clay	GM-H60/CCL	GM-H60/CCL	GM-H60/CCL		8	8	2	8	8	2	
4	J-1 to J-3	17	SW	11	300	Clays, sandstone	Landfill overfill liner	GM-H60/CCL	GM-H60/CCL				3			3	
5	R-1	13	SW	6.5	250	Sands, clays	GM-H80/CCL	GM-P40/CCL	GM-P40/CCL			1					1
6	R-2 to R-5							GM-H80/CCL	GM-H80/CCL			4					
7	P-1	21	SW	28	40	Clay	GM-H60/CCL	GM-H60	GM-H60/CCL				1	1			
8	P-2 to P-4	13					GM-H60/GCL					1	2	1	2		
9	Y-1	10	NW	10	120	Gravelly sands, silty clays	GM-H40/GCL	GM-H60	GM-H40/CCL			1					1
10	Y-2 and Y-3							GM-H80	GM-H60/CCL			2					
11	M-1	4	NW	9.5	200	Clays	Soil	GM-H60/CCL	GM-H60/CCL		1					1	
12	D-1 and D-2	6-9	NE	42 (17) ⁴	10	Clays, gravelly sands	GM-L40/GCL	GM-H60	GM-H60/GCL		1	1		1	1		
13	F-1	12	NE	40 (60) ⁴	90	Sands, silty loam	GM-L40/GCL	GM-H60	GM-H80/CCL		1				1		

Notes:

- 1). GM = geomembrane, -H = high density polyethylene, -P = polyvinyl chloride, -L = linear low density polyethylene. The number denotes GM thickness (mil).
- 2). CCL = compacted clay liner, GCL = geosynthetic clay liner.
- 3). The number of units with the average annual flow in the range shown over last 3 years of post-closure. Gpad = gallons per acre per day.
- 4). Average annual snowfall in inches shown in parenthesis.

Predicted vs. Observed Trends in Leachate Flow

Although it is reasonable to expect that leachate generation rates generally trend downwards after cover placement, it may not be reasonable to expect that flow rates will decline at such a significant rate over the long term.

In order to better understand behavioral trends in leachate generation based on field observations, annual LCRS flow measurements were compared to annual flow rates predicted using the HELP Model. If the model provides an accurate prediction of long-term leachate flow post-capping, it should be expected that LCRS flow rates would decrease exponentially to a value approximating the quasi-steady state flow rate predicted by the model, after allowing for a reasonable time lag for water already in the waste body prior to capping to percolate down to the LCRS and be removed.

Overall, the HELP Model appeared to be better suited to predicting long-term LCRS flow at wet rather than dry sites, consistent with previous findings (e.g., Vorster, 2001). The model predicts zero or near-zero LCRS flows at dry sites, whereas some LCRS flow was observed at all four dry case study landfills (although three had no/negligible LDS flow). Although this **study did not focus in detail on this aspect of the model's application, it is suggested that** the model significantly underestimates lateral flow where the thickness of liquids on the primary liner is very small. This manifests as a predicted LDS flow that exceeds the corresponding LCRS flow, although in both cases the volumes are very low (this issue was observed where modeled flows were in the range of 10^{-4} to 10^{-7} gpad).

Selected results from 6 of the 13 design configurations (different cover, primary liner, and LCRS/LDS drainage layer) represented by the 45 case study units were reviewed in detail in Section 4.2.2. The selected results featured four of the case study sites: wet Landfills B and F and dry Landfills R and Y. Based on these results, if the requirement under §264.310(b)(2) that operation of the LCRS should be continued "until leachate is no longer detected" **is interpreted strictly to mean that LCRS operation must continue until leachate flow is at or near zero, then this cannot reasonably be achieved, even at extremely dry sites.** Therefore, it is more appropriate to apply the performance-based standard implicit in 40 CFR §264.117 that PCC is required until a demonstration that the absence of care would not pose a threat to water quality at the POC. Such demonstrations could be made on the basis of leachate flow and concentrations having reached quasi-steady-state, predictable, and non-impacting conditions, albeit at a non-zero flow rate.

Data from this study suggest the rate at which LCRS flow rate declines post-closure may be three to five times slower at most Subtitle C landfills than previously suggested by EPA (2002). The dataset utilized in this study was more comprehensive (45 individual units with up to 29 years of post-closure data) than the 2002 study (33 units with up to 9 years of post-closure data). In particular, the lack of data from many units beyond year six of PCC in the earlier study, and the very low flow rates reported for a few units after year six suggests that the dataset may have been skewed by a small number of very dry or high performing sites. If current industry projections of post-closure leachate generation are based on EPA (2002), expectations may need to be reset in terms of the rate of decline in LCRS flow. Rather than an order of magnitude decrease every 5 years, it may be more reasonable to expect an order of magnitude decrease every 15–20 years. However, more field studies are needed to validate this finding before recommendations for adjusting current industry projections and accruals for leachate management are made. Since the rate of decrease also appears to be linked to maximum leachate generation at closure, this emphasizes the importance of good stormwater control during the latter stages of operation and competent cover design and construction performed under strict CQA procedures so as to minimize leachate generation immediately after closure.

7.1.2 Liner Design and Performance

The focus of this discussion is to address the following key research question: What conclusions can be drawn regarding the hydraulic efficiencies of double-liner systems (i.e., leakage rates through primary liners) at Subtitle C landfills based on available LCRS and LDS data? It should be made clear that all case study sites reported competent CQA programs during liner and cover construction events. As such, the effect of CQA practices on long-term containment system performance cannot be assessed. The findings in this section are thus predicated on the assumption that good CQA will be employed during liner and cover system installation.

Apparent Hydraulic Efficiency of the Primary Liner

The “**apparent**” hydraulic efficiency, EA, of the primary liner can be calculated as the flow in the LDS relative to the flow in the LCRS (Section 5.1.1). The higher the value of EA, the smaller the flow rate from the LDS compared to the LCRS flow rate. Based on the data provided only 5% of the evaluated data demonstrated an effective efficiency greater than 99% with the bulk of the data (73%) exhibiting an efficiency that is less than 90%. It is noteworthy to mention that the leachate flow from some of the LDS of some units was greater than that from the LCRS. With the data provided, it is unclear if the source of the

increased in the liquid flow from the LDS(s) is an external source (e.g. groundwater) or a major defect in the liner system. The liner efficiencies are significantly higher at dry than at wet sites: 79% of the data from dry sites produced E_A values exceeding 99% (Table 5-1B), whereas only 5% of the data from wet sites did so (Table 5-1).

Modeled Hydraulic Efficiency of the Primary Liner

There is a high degree of uncertainty associated with using the HELP Model to calculate efficiency as the method does not utilize the site-specific available data. As described in Section 5.2.1, expected leachate generation rates in the post-closure period were simulated by the HELP Model using default input assumptions, including the number of geomembrane defects. This allowed the modeled hydraulic efficiency, EM, of a liner system to be calculated using model output values as substitutes for field data of LCRS and LDS flow. Values for EM were calculated for the 13 unique design configurations reflecting the study units. Results were summarized in Table 5-2. Calculated EM values varied from 93 to 99.9%, close to corrected E_c .

Corrected Hydraulic Efficiency of the Primary Liner

By distinguishing liquids with similar chemical signatures, leachate chemistry data can potentially be used to quantify the portion of liquids comprising total LDS flow that should be attributable to primary liner leakage as opposed to other sources. This approach has two major limitations:

1. Only leachate (i.e., LCRS liquids) contributes to the chloride concentration in the LDS. Any contribution of chloride from other sources (e.g., brackish groundwater) would serve to increase the numerator in the expression and thus increase the magnitude of the CF value computed.
2. Chlorides are not adsorbed to soils in the primary liner or drainage layers, or otherwise attenuated as it passes through the primary liner.
3. It is noted that this analysis may overestimation of the liner efficiency since it assumes no attenuation in the liner system.
4. The concentration of the indicator chemical and flow data must be temporally coincidental in the LCRS and LDS in order to calculate a corrected liner efficiency, E_c .
5. Specific to this study, data available for calculating E_c were limited to 173 sets of readings at 17 units from only three sites (Landfills B, T, and J).

Landfill J is located in an arid climate with little to no LCRS flow. In most cases, the LDS flow volumes, while minor, exceeded LCRS flow volumes. Based on the E_c value calculated only 20–40% of the total LDS flow could be attributed to primary liner leakage at the site. The E_c calculations for the other two sites (B and T) that receive more than 25 inches of rain

per year, showed that less than 3% of the liquids generated from the LDS are attributable to leachate leakage through the liner. This is a significant improvement over E_A values of 63 to 93% calculated for corresponding units in Landfill B and E_A values of 54 to 95% calculated for corresponding units in Landfill T. However, we note again that these E_C values are most likely an overestimation of the actual liner efficiency as stated above.

Comparison between Apparent, Modeled, and Corrected Liner Efficiencies

The results for liner efficiencies computed using the three methods discussed in this report are compared in Table 7-2 for the three sites (Landfills B, T, and J) for which values from all three methods could be calculated. Modeled liner efficiency (E_M) values shown in the table are directly copied from results in Table 5-2. Corrected liner efficiency (E_C) values are average values for each different design configuration based on the values for individual units shown in Table 5-3. Finally, apparent liner efficiency (E_A) was calculated based on monthly average LCRS and LDS flow data for units where concurrent chloride concentration data were also available for calculation of an E_C value. This allows direct comparison of corresponding values for E_A , E_M , and E_C .

For all design configurations listed in the table, E_C is greater than E_A . Although this data set is very limited and does not include the high values for E_A calculated for a number of units in dry Landfills R and Y, this nevertheless suggests that true liner efficiency is potentially higher than the simple value calculated as the LDS flow volume relative to the LCRS flow volume. Values of E_M and E_C are very close. This suggests that E_M may be a useful representation of liner efficiency where field data with which to estimate E_A or E_C are not available. For landfills constructed with good COA, this may suggest the use of the HELP Model with default assumptions as a method of estimating the liner efficiency after the closure of a landfill.

Table 7-2. Comparison of liner efficiency calculations

Design	Climate condition	Study units	Apparent liner efficiency (E_A)	Modeled liner efficiency (E_M)	Corrected liner efficiency (E_C)	Number of data used to calculate E_C
1	Wet	B-1 to B-6	82.8%	99.93%	99.84%	90
2	Wet	B-7 and B-8	84.9%	99.92%	99.95%	19
3	Wet	T-1 to T-18	73.2%	99.58%	98.57%	64
4	Dry	J-1 to J-3	9.1%	99.98% ⁽¹⁾	74.2%	6

Note:

1). E_M cannot be calculated under conditions of final cover at units J-1 to J-3 due to the fact that Landfill J resides beneath an overfill landfill. Therefore, E_M was calculated for intermediate cover conditions.

7.1.3 Trends in Leachate Chemistry

A review of the technical literature revealed that leachate chemistry from HW landfills has received relatively little scrutiny in recent years. Tian (2015) analyzed leachate composition from four landfills constructed for containment of mixed LLRW and HW and compared concentrations of dissolved organic matter (measured as TOC), inorganic macro-components (including major cations and anions), and trace metals to values reported in the literature for MSW leachate, concluding that:

- Dissolved organic matter concentrations were insignificant when compared with MSW leachate;
- Concentrations of inorganic macro-components were broadly similar to MSW leachate; and
- Trace metal concentrations were relatively lower than in MSW leachate and tended to exhibit steady or slightly increasing trends.

If current expectations for the time required for the concentrations of constituents of concern in leachate to decrease to asymptotic levels or meet regulatory standards such as maximum contaminant levels (MCLs) are rooted in observations from MSW landfills, this may not be appropriate. Therefore, an important component of this study is to review concentration trends in leachate from Subtitle C landfills.

From the above, the focus of this discussion is to address two key research questions: What is the leachate chemistry at the case study sites, and does it exhibit asymptotic behavioral trends over the long-term? How does leachate chemistry at Subtitle C landfills compare with water quality limit values such as drinking water MCLs? Addressing these questions is intended to broaden an understanding of long-term leachate management at closed Subtitle C landfills in the context of the performance standard implicit in 40 CFR §264.117.

Parameter Selection and Evaluation Approach

Thirty chemical parameters were selected to represent leachate constituents of interest, based on those investigated by EPA (2002). These included water quality indicator parameters (pH, specific conductance, TDS), macro indicators of dissolved organic matter (COD, BOD, and TOC), major inorganic cations (calcium, magnesium, potassium, and sodium) and anions (calcium, chloride, and sulfate), trace metals (arsenic, cadmium, chromium, lead, and nickel), and trace VOCs frequently observed to be present in landfill leachate (represented by a group of 12 aromatic hydrocarbons and chlorinated solvents).

The general approach to the evaluation was as follows:

- Where available, leachate concentrations were compared to federal water quality standards (MCL or SMCL). However, rather than directly comparing leachate

concentrations to a limit value, a universal dilution/attenuation factor (DAF) of 20 was applied to represent concentration increases that would be expected prior to detection at a POC monitoring well. This is consistent with the default DAF specified in the EPA's Soil Screening Guidance (EPA, 1996), for which a DAF of 20 is deemed protective of contaminant sources up to 0.5 acres in size. While landfills are generally much larger than that, the potential release points (i.e., potential pinhole defects and tears in liner GM barriers) are collectively much smaller than 0.5 acres. As such, this approach is appropriate for the purposes of this study. Nevertheless, it is important to note that assigning this default DAF in this way does not imply any endorsement from EPA with regard to the universal application of this approach to assessing long-term leachate management and groundwater monitoring at Subtitle C landfills.

- The availability of leachate chemistry data at the case study landfills was limited in most cases, which restricted the level of analysis that could be completed in this study. Data availability and gaps are discussed in further detail in Section 7.2.

Summary of Main Findings

There is significant variability in the data for many constituents, particularly in the LCRS where differences between maximum and minimum observed values often span six or more orders of magnitude for cations/anions, trace metals, and VOCs. For this reason, the median may be considered more representative of overall leachate quality, which is more sensitive to one or two significant outliers. The general water quality characteristics of liquids from the LCRS and LDS drainage layers are also significantly different, again by multiple orders of magnitude in many cases.

Observations from the evaluation of leachate chemistry data with regard to the five categories of interest are summarized as follows:

- **Water Quality Indicators:** Temporal trends in leachate pH appear relatively stable, with most readings within the range of 6.5 to 8.5 s.u. The majority of readings were alkaline mainly as a result of solidifying and stabilizing hazardous waste with cement, fly ash, or kiln dust prior to disposal. Overall, conductivity measurements were highly variable with no evidence of asymptotic behavior. Values in the LCRS and LDS are of the same order of magnitude.
- **Dissolved Organic Matter:** The DOM content of leachate cannot be extensively commented on in this report since these data were not routinely collected at any case study site, although TOC has been analyzed on a handful of occasions in both the leachate and the LDS at Landfills P and T. TOC concentrations were significantly lower than those reported for MSW leachate. The mean and median TOC concentrations in the LDS were two to three orders of magnitude lower than in the LCRS. The limited data available for this study generally support the finding by Tian (2015) that DOM concentrations are insignificant in HW leachate when compared with MSW leachate.
- **Major Cations and Anions:** With the exception of alkalinity, which is not analyzed at any case study landfill, data for the other major cations (calcium, magnesium, potassium, and sodium) and anions (chloride, and sulfate) were available in both the LCRS and LDS at a number of case study sites. Based on the literature, they are also

expected to be present in significant concentrations in HW leachate at concentrations broadly similar to MSW leachate. Overall, the cation/anion data are highly variable with no consistent evidence of asymptotic behavior or downward trends. With the exception of sodium, and to a lesser extent chloride, there has been no significant change in the overall geochemical makeup of leachate over the 13 years of PCC. Using chloride as an example of behavioral trends, concentrations ranged widely over three orders of magnitude, with a median concentration higher than the range reported for MSW landfills but broadly consistent with that reported for two HW landfills by EPA (2002).

- **Trace Metals:** Generally, the median concentrations of trace metals in the studies HW landfill leachate from was above the DAF-modified benchmark. Specifically, arsenic was measured at a median concentration of approximately 12 mg/L. Concentrations of trace metals in the LDS were universally lower than in the LDS. However, a review of data from EPA (2002), which reported on leachate quality from three HW landfills, indicates that mean concentration of trace metals were slightly lower than corresponding values reported in this study. The order-of-magnitude similarity in concentrations over the 15-year intervening period suggests that trace metal concentrations in leachate are relatively stable over the long term.
- **Volatile Organic Compounds:** Of all the groups of chemical constituents considered in this study, VOCs exhibited the most variability in concentration between different landfill units, and sometimes between different sumps within the same unit. The variability among the LCRS data was significant for all VOCs, with reported concentrations ranging over six or more orders of magnitude. LDS data exhibited far less variability, with similar median and mean concentrations for all but one VOC (trichloroethylene). The wide discrepancy between median and mean values in the LCRS suggests that the data could potentially be skewed by outliers. This is supported by comparison of LCRS data to values for VOCs reported for MSW leachate: median values of all VOCs from this study fall within the concentration range reported for MSW landfills but mean values significantly exceed the reported range for MSW leachate. MCLs are specified for all but two of the selected VOCs.

The long-term outlook for leachate management based on observations of behavioral trends amongst selected leachate data from this study is mixed. Water quality indicators and major cations/anions suggest that the materials contained in Subtitle C landfill may not degrade under landfill conditions, or only degrade very slowly, such that observations based on leachate data from non-hazardous Subtitle D landfills cannot be extrapolated to characterize the expected performance of Subtitle C landfills. On the other hand, data for trace metals and VOCs, while highly variable, thus site-specific considerations are important when evaluating these parameters.

7.2 Data Availability and Limitations

The unavailability of some critical site information and monitoring data limited the extent to which evaluations could be completed or even performed in this study. Data gaps and their effects were identified throughout Chapters 4 to 6. The focus of the discussion in this section is to reiterate key data limitations and discuss their effect on limiting the study. The

goal is to provide some guidance to site operators and regulators as to what data that are not routinely collected would be valuable in demonstrating that one or more components of PCC at Subtitle C landfills could be scaled back or terminated over the long term. This is intended to provide motivation, rather than an obligation, for additional data collection. Indeed, it is important to emphasize here that this discussion is not concerned with data collection and reporting for compliance purposes. Rather, this section seeks to address the following key research question: How could current monitoring, reporting, and recordkeeping requirements be improved to better ensure that the data necessary for performance demonstrations are collected?

7.2.1 General Site Information

Overall, this study included 45 individual double-lined units at nine separate landfill facilities. Significant variability existed between the units, which is beneficial to a study of this nature. For example, individual units ranged in size by an order of magnitude from 1.4 acres to 11.3 acres, time of post-closure from 6 to 29 years (Landfill M is not closed, but has been inactive for 4 years pending final capping after waste was filled to final grades), and various LCRS and LDS flow measuring devices. In terms of variability in construction details, 11 different liner system designs and a further 11 different cover system designs were featured amongst the study units featured, combining to provide 13 unique containment system design configurations. Major variables in cover system design were **represented: CCL/GM (an apparently accidental “upside down” design), GM/CCL, GM/GCL, and all soil.** However, primary liner designs essentially comprised only two variables: GM/CCL and GM only. No case study units were constructed having a GM/GCL composite primary liner, although one site (Landfill D) utilizes a GCL in the secondary liner. As such, the efficacy of a GM/GCL primary liner design cannot be evaluated in this study, an important limitation, given the widespread use of GCLs in the liner systems at both Subtitle C and D landfills.

In terms of facility operations, seven of the nine landfills are commercial TSDFs accepting HW from a wide range of generators, while two are industrial facilities providing disposal for a single HW generator or as part of site remediation). As such, an original research question from this study (does waste type affect leachate concentrations?) cannot be addressed, as the commercial facilities accepted waste from multiple sources thereby making it difficult to compare waste chemistry for these sites, while there were insufficient data from non-commercial TSDFs against which to gauge variability between commercial and non-commercial operations. Further, waste manifests were not made available by any site

operator, which meant that although some findings appeared to support the hypothesis that facility/waste type would affect leachate characteristics, this could not be confirmed. For example, arsenic concentrations appeared to cluster according to a unit, suggesting that arsenic concentrations in HW leachates are highly site-specific and dependent on the waste source or sources disposed of in a particular unit. In addition, the relatively high concentrations of arsenic and low concentrations of heavy metals that persist long into the PCC period may be due to the relatively alkaline pH of leachate, potentially attributable to the practice of solidifying and stabilizing HW with cement, fly ash, or kiln dust prior to disposal. Details and data regarding these practices are needed in order to fully understand the long-term leaching behavior of disposed HW.

7.2.2 Leachate Flow Data

For all case study landfills, leachate flow data were normalized to an annual average and a peak flow in terms of gallons per acre per day (gpad) to provide a common unit for comparison between sites. Attempts were made to collect data from the date of closure (i.e., time zero for PCC) through to the current time in order to obtain a complete timeline of post-closure flow from each unit. However, the availability and level of granularity amongst the data varied considerably between sites. Available leachate data for Landfill T, for example, includes annual total LCRS and LDS flow volumes for up to 7 years in some units and daily flow volumes for years 10 through 23 of PCC in other units. Respective earlier and later records were not available. Leachate data for Landfill Y included weekly LCRS and LDS flow volumes for all three study units for the duration of PCC, except the first year (an important data point in terms of assessing trends in leachate generation post-closure). Fuller LCRS and LDS datasets would have expanded the level of detail to which cover and liner system performance could be evaluated and would likely have enabled a clearer picture of long-term stable leachate generation to be gathered.

An important limitation regarding the use of LCRS and LDS leachate chemistry to correct EA values is the need for concurrent chemistry data to be available in both the LCRS and LDS. Overall, such data were available for only 17 units at three sites (Landfills B, T, and J). This represents a shortcoming on the part of site operators at collecting data that could help understand long-term liner performance.

7.2.3 Leachate Chemistry Data

It is noted through the discussions in this report that the leachate chemistry database is limited in terms of its completeness and the duration of monitoring, although it is important to **acknowledge that “completeness” in this context refers to the availability of the full suite**

of 30 parameters targeted in this study and not to data requirements specified for compliance. Many targeted leachate constituents are poorly represented in the LCRS dataset, while LDS chemistry is not monitored at all at many sites. In the latter case, it is noted that three sites (Landfills R, Y, and M) have zero flow in the LDS, which negates the ability to collect samples for analysis. In the context of this data assessment, therefore, this should not be construed as a data gap as these site operators have effectively achieved a goal of PCC, which is to end monitoring and management of liquids in the LCRS and LDS. An issue of importance identified in the process of collecting leachate chemistry data for this study is that site operators are only required to keep records for three years; as such, many older data are no longer available. If this lack of data at the case study landfills is representative of that at other Subtitle C facilities, which seems likely, this represents an important limitation on assessing the long-term performance of containment systems and potential modifications to existing PCC programs.

Intra-unit comparisons (i.e., comparison of leachate chemistry between the LCRS and LDS in the same unit) are dependent on the same constituents being monitored on the same date, while inter-unit comparisons (i.e., comparison of leachate chemistry between different units and sites) are dependent on the similarity of the leachate analyte lists. Both intra and inter-unit comparisons, which can provide important operational insights into relative cover system infiltration and other indicators of landfill performance, are obviously hampered by a lack of data. The general paucity of leachate chemistry data at Subtitle C landfills, particularly with regard to the chemistry of liquids recovered from LDS drainage layers, may be due to two reasons:

- Leachate chemistry data are most commonly collected semi-annually or annually at each site, thereby limiting the overall size of the dataset available for analysis; and
- The leachate constituent list monitored is dependent on site-specific waste history and local practices for leachate treatment and disposal (i.e., non-compliance data), thereby limiting the number of similar constituents for which data were available at all sites (i.e., constituent lists vary between different wastewater treatment facilities).

Anecdotal information received from site operators was that leachate volumes and chemistry have not deviated from acceptance criteria for treatment and disposal, such that leachate management has not been an issue or represented a higher-than-expected cost. Operators did not complain of any significant problems related to leachate treatment and disposal, which suggests that this is not an issue at Subtitle C facilities. As noted previously, the strong focus on containment and reducing leachate flow volumes under RCRA is likely a contributing factor to the lack of LDS data. Certainly, data availability was highest at two

wet sites (Landfills B and T), which may be reflective of the level of concern operators have on managing leachate treatment and disposal costs (in other words, low leachate flows attract little interest, because disposal costs are modest for small volumes). The fact that many of the data used to support the performance demonstrations made in this study rely on non-compliance data is borne out that very limited leachate chemistry data were available at wet Landfill F, for which data were obtained directly from the state rather than from the operator. As such, the chemistry dataset would not be expected to include non-compliance leachate chemistry data collected only to meet influent limits imposed by the receiving wastewater treatment plant (WWTP). This has implications in terms of being able to assess site performance independent of the operator, which would be important if, say, an operator was unable to continue providing care.

In terms of specific availability of individual analytes from the list of 30 targeted for this study, data were available for all analytes with the exception of COD, BOD, and alkalinity, although many analytes were available only in LCRS and not LDS datasets. The lack of COD and BOD data may not be important, as concentrations of dissolved organic matter in leachate are not a primary concern to Subtitle C landfill operators beyond complying with potential influent limitations imposed by a receiving WWTP. However, it should be recognized that long-term changes in leachate management may require consideration of effluent discharge to receiving water bodies, in which case BOD may become a critical analyte. More important in the context of this study is the absence of alkalinity data, as data for the other major cations (calcium, magnesium, potassium, and sodium) and anions (chloride, and sulfate) that make up the majority of the ionic charge balance were available in both the LCRS and LDS at a number of case study sites. This prevented the construction of Stiff or Piper plots, which are effective methods of portraying the data.

7.3 Recommendations for Future Research and Development

This study identified a number of areas in which further research could be beneficial in understanding the long-term performance of Subtitle C landfills. In the longer term, this could potentially facilitate the development of guidance on improving long-term performance, reducing the duration and costs of PCC, and allowing more flexibility in the manner in which the regulation is applied (which may be appropriate recognizing that that end goals for PCC are defined in terms of performance). Better understanding and improved predictability of long-term performance could allow more innovation and creativity enclosure system designs. Ultimately, performance data from studies such as this could be used to

assign risk-based evaluation criteria and procedures for demonstrating long-term protection of HHE and completion of PCC.

Two short-term suggestions for further research include the following:

- Multi-site evaluation of leachate chemistry data: This study focused on leachate flow and containment system performance. While leachate chemistry data were examined, for various reasons identified in the report these examinations were not sufficiently detailed nor were the dataset extensive enough to draw firm conclusions regarding the interrelated factors affecting long-term trend behavior and leachate quality. Insufficient flow data and waste manifests were available to accurately review leachate chemistry in terms of contaminant removal loads (e.g., cumulative mass of contaminant removal per ton of waste in place), although this would potentially have been advantageous in terms of normalizing the data between the different study units, climatic conditions, and cover/liner design configurations. The effects of facility operations and waste types on leachate chemistry were not examined. No statistical analysis was conducted to eliminate outliers, test the significance of trends, or assign correlation coefficients to trend data. No correlations between leachate chemistry and flow rates were investigated, so it is not known whether the goal of excluding liquids from RCRA landfills in the post-closure period contributes to chemical changes or whether cover design and performance has a direct effect on leachate quality.
- The vulnerability of Subtitle C landfills to short and long-term hazards: Currently, landfills are designed and operated assuming that future climate and precipitation intensity will be similar to historical records. In addition, seismic design requirements are not explicitly cited for Subtitle C landfills. Some hazardous waste management units in place today may thus be vulnerable to future conditions. This could have serious consequences for the integrity of hazardous waste disposal facilities and protection of HHE. Therefore, the resilience of hazardous waste disposal facilities should be evaluated with regard to both long-term hazards (e.g., inundation due to sea level rise, elevated temperatures, seismic events, and/or groundwater elevation rise) and short-term hazards (e.g., possible increase in precipitation and associated flooding, increases in storm flooding/surges, and changes in waves, currents, king tides, or El Niño effects). To date, little research has been published on the long-term vulnerability of closed landfills to short and long-term hazards. As such, this represents an important research need in terms of assessing the long-term performance of landfill containment systems.

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9. APPENDIX I

LANDFILL DATA FORM

FACILITY NAME:
LOCATION:

1. CRITICAL ATTRIBUTES CHECKLIST

SUBTITLE C SITE ONLY

AT LEAST ONE CLOSED (I.E., CAPPED) SUBTITLE C UNIT AT SITE:
NO. OF CLOSED SUBTITLE C UNITS WITH DOUBLE LINER SYSTEMS (LCS AND LOS):
LCS AND LOS FLOW DATA AT CLOSED SUBTITLE C UNITS AND NON-SUBTITLE C UNITS:

YES/NO	YES/NO
YES/NO	YES/NO
YES/NO	YES/NO

→ IF NO, STOP HERE
→ IF NONE, STOP HERE
→ IF NO, CONTACT PROJECT TEAM TO DISCUSS BEFORE PROCEEDING
→ IF YES, CONTINUE TO ITEM (2) NEXT

2. GENERAL SITE INFORMATION

LCS FLOW DATA
LOS FLOW DATA
LCS (LEACHATE) CHEMISTRY DATA

DATA IS COMBINED ACROSS ALL CLOSED UNITS
DATA IS COMBINED ACROSS ALL CLOSED UNITS
DATA IS COMBINED ACROSS ALL CLOSED UNITS

ON-SITE WEATHER DATA:

PRECIPITATION: YES/NO
TEMPERATURE: YES/NO

IF YES, PROJECT TEAM WILL FOLLOW UP TO OBTAIN DATA

3. DATA MANAGEMENT MATRIX

PLEASE COMPLETE DATA MANAGEMENT MATRIX BELOW FOR EACH CLOSED UNIT

THE NUMBER OF COLUMNS SHOULD MATCH THE NUMBER IN BOX 1A ABOVE. PLEASE ADD ADDITIONAL COLUMNS IF NECESSARY FOR EACH UNIT. ANSWER QUESTIONS 1-26 OR INDICATE "YES" IF DATA ARE AVAILABLE, "N/A" IF DATA ARE NOT AVAILABLE, "N/A" IF NOT APPLICABLE FOR EACH YES ENTRY. PROJECT TEAM WILL FOLLOW UP TO OBTAIN DATA.

UNIT ID #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	
1. UNIT SIZE (LINER PLAN AREA) (ACRES)																					
2. CONSTRUCTION DETAILS (LINER, LCS, AND LOS CROSS-SECTIONS)																					
3. LINER PLAN LAYOUT DRAWING AVAILABLE																					
4. AVERAGE LINER BASE SLOPES (%)																					
5. 3RD PARTY COA PROGRAM FOR LINER CONSTRUCTION																					
6. END OF LINER CONSTRUCTION DATE																					
7. START OF OPERATING PERIOD																					
8. END OF OPERATING PERIOD																					
9. PREDOMINANT WASTE TYPE																					
10. WASTE RECEIPT/PLACEMENT RECORDS AVAILABLE																					
11. TOTAL VOLUME/MASS OF WASTE IN PLACE																					
12. MAXIMUM WASTE HEIGHT (FEET)																					
13. CLOSURE CONSTRUCTION DETAILS (COVER CROSS-SECTIONS)																					
14. GAS MANAGEMENT SYSTEM (VENTS, WELLS)																					
15. COVER PLAN LAYOUT DRAWING AVAILABLE																					
16. FINAL COVER TOP SLOPES (%)																					
17. FINAL COVER SIDE SLOPES (H:V)																					
18. 3RD PARTY COA PROGRAM FOR COVER CONSTRUCTION																					
19. END OF COVER CONSTRUCTION DATE																					
20. LCS FLOW DATA																					
21. NO. OF INDEPENDENT LCS FLOW MEASUREMENT LOCATIONS																					
22. METHODS OF LCS FLOW MEASUREMENT ¹																					
23. LOS FLOW DATA																					
24. NO. OF INDEPENDENT LOS FLOW MEASUREMENT LOCATIONS																					
25. METHODS OF LOS FLOW MEASUREMENT ²																					
26. LEACHATE CHEMISTRY DATA																					
27. NUMBER OF INDEPENDENT LEACHATE SAMPLING POINTS																					
28. LEACHATE DATA QA/QC PROTOCOL																					
29. ADDITIONAL OBSERVATIONS/EXPLANATION OF LCS/LOS DATA ³																					
30. COVER SYSTEM DRAIN / REPAIR RECORDS																					
31. COVER SYSTEM SETTLEMENT DATA																					
32. TOPOGRAPHIC SURVEYS																					

Notes:

- CODES FOR METHOD FOR FLOW MEASUREMENT:
 - A. AUTOMATIC PUMPING SYSTEM, LIQUID VOLUME RECORDED FROM ACCUMULATING FLOW METER
 - B. PERIODIC PUMPING IF LIQUID IS PRESENT, VOLUME RECORDED FROM ACCUMULATING FLOW METER
 - C. PERIODICALLY MEASURE TIME TO FILL A KNOWN VOLUME
 - D. PERIODIC PUMPING IF LIQUID IS PRESENT IN SUMP TO A HOLDING TANK, VOLUME TRANSFERRED FROM HOLDING TANK MEASURED
 - E. PERIODIC PUMPING IF LIQUID IS PRESENT IN SUMP, VOLUME ESTIMATED FROM CHANGE IN LIQUID LEVEL
 - F. AUTOMATIC PUMPING SYSTEM, LIQUID VOLUME ESTIMATED BY MULTIPLYING PUMP CAPACITY X TIME
 - G. OTHER (PLEASE SPECIFY)
- FOR DRAINAGE STUDIES HAVE BEEN CONDUCTED INTO STORMWATER INFILTRATION ISSUES, EQUIPMENT PROBLEMS, METER CALIBRATION/TOLERANCES, DATA GAPS, ETC. PLEASE RECORD ADDITIONAL NOTES AND COMMENTS ON NEXT TAB (IF APPLICABLE)

10. APPENDIX II

Table II-1. Landfill construction details

Landfill designation	Total number of study units	Total area of all units included in study (acres)	Dates of waste placement in study units ⁽¹⁾	Dates of closure of study units ⁽¹⁾	Total waste in place (million cubic yards)	Average duration of study unit operation (years)	Maximum waste thickness (feet)	Average base liner slope (%)	Average final cover slope, top area (%)	Average final cover slope, side slopes (H:V)	Third-party COA performed on liner	Third-party COA performed on cover	Flow measurement method for LCRS and LDS ⁽²⁾
B	8	50	M-1980s to L-1990s	L-1980s to M-2000s	3.3	3	110	2	2	3:1	Yes	Yes	i
T	18	48	L-1980s to M-1990s	M-1980s to M-1990s	4.6	3	80	2.5	5	5:1	Yes	Yes	ii
J	3	31	M-L 1980s	M-L 1990s	3.0	9	90	2	2	NA	Yes	Yes	ii
R	5	31	M-1980s to E-1990s	E-2000s	2.2	10	100	2	5	3:1	Yes	Yes	iii
P	4	40	L-1980s to E-1990s	M-1990s to E-2000s	1.7	7	50	2	0.5	20:1	Yes	Yes	iv
Y	3	5	M-1980s	M-2000s	0.24	19	55	1	4	3:1	Yes	Yes	ii
M	1	9	M-1990s	M-2010s	0.8	21	110	2	5	3:1	Yes	NA	iv
D	2	9	M-2000s	M-L 2000s	0.7	3	100	4.5	10	3:1	Yes	Yes	iv
F	1	6	L-1980s	E-2000s	0.31	14	50	2	10	3:1	Yes	Yes	ii

Notes:

1). E = early, M = mid, L = late.

2). i = automatic pumping system, liquid volume recorded from accumulating flow meter; ii = periodic pumping if liquid is present, volume recorded from accumulating flow meter; iii = periodic pumping if liquid present in sump, volume estimated from change in liquid level; iv = periodic pumping if liquid is present in sump to a holding tank, volume transferred from holding tank measured.

NA: not applicable. Landfill J is situated below an MSW overflow landfill and has no sideslopes. The unit at Landfill M has intermediate cover soil only.

Table II-4A. Annual average LCRS and LDS flow rate (gpad) at Landfills B, J, and R

Landfill unit	Sampling		Years into post-closure care program																															
	Layer	Interval	Total Data	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29		
B-1	LCRS	Monthly	641																															
	LDS	Monthly	536																															
B-2	LCRS	Monthly	644																															
	LDS	Monthly	452																															
B-3	LCRS	Monthly	675																															
	LDS	Monthly	401																															
B-4	LCRS	Monthly	669																															
	LDS	Monthly	205																															
B-5	LCRS	Monthly	663																															
	LDS	Monthly	370																															
B-6	LCRS	Monthly	675																															
	LDS	Monthly	207																															
B-7	LCRS	Monthly	420																															
	LDS	Monthly	228																															
B-8	LCRS	Monthly	138																															
	LDS	Monthly	126																															
J-1	LCRS	Monthly	20																															
	LDS	Monthly	85																															
J-2	LCRS	Monthly	21																															
	LDS	Monthly	21																															
J-3	LCRS	Monthly	26																															
	LDS	Monthly	26																															
R-1	LCRS	Monthly	148																															
	LDS	Monthly	148																															
R-2	LCRS	Monthly	156																															
	LDS	Monthly	156																															
R-3	LCRS	Monthly	156																															
	LDS	Monthly	156																															
R-4	LCRS	Monthly	156																															
	LDS	Monthly	156																															
R-5	LCRS	Monthly	156																															
	LDS	Monthly	156																															

Table II-2. Landfill base liner system details

Designation		Area	LCRS		Primary liner			LDS		Secondary liner ⁽⁴⁾				
Landfill	Sub-group	Liner area of landfill units (acres)	Drainage layer material ⁽¹⁾	Soil thickness (inches)	Type of liner system ⁽²⁾	Geomembrane material ⁽³⁾	Geomembrane thickness (mil)	Compacted clay liner thickness (inches)	Drainage layer material ⁽¹⁾	Soil thickness (inches)	Type of liner system ⁽²⁾	Geomembrane material ⁽³⁾	Geomembrane thickness (mil)	Compacted clay liner thickness (inches)
B	B-1 to B-8	4 – 9	S	12	GM/CCL	HDPE	80	60	S	12	GM/CCL	HDPE	80	36
T	T-1 to T-18	1.5 – 4	S	12	GM/CCL	HDPE	60	36	S	12	GM/CCL	HDPE	60	36
J	J-1 to J-3	9.5 – 11.5	S	12	GM/CCL	HDPE	60	18	G/GN	12	GM/CCL	HDPE	60	36
R	R-1	5 – 7.5	GN	--	GM/CCL	PVC	40	36	GN	--	GM/CCL	PVC	40	36
	R-2 to R-5		GC	--	GM/CCL	HDPE	80	36	GC	--	GM/CCL	HDPE	80	36
P	P-1 to P-4	10	GN	--	GM	HDPE	60	--	GN	--	GM/CCL	HDPE	60	36
Y	Y-1	1.5 – 2	S	12	GM	HDPE	60	--	S	12	GM/CCL	HDPE	40	36
	Y-2 and Y-3		S	12	GM	HDPE	80	--	S/GC	12	GM/CCL	HDPE	60	36
M	M-1	9	S/GC	12	GM/CCL	HDPE	60	18	GN	--	GM/CCL	HDPE	60	36
D	D-1 and D-2	3 – 6	GC	--	GM	HDPE	60	--	GC	--	GM/GCL	HDPE	60	--
F	F-1	6	GN	--	GM	HDPE	80	--	GN	--	GM/CCL	HDPE	80	36

Notes:

- 1) GN = geonet, GT = geotextile, GC = geocomposite, G = gravel, S = sand
- 2) GM = geomembrane, GCL = geosynthetic clay liner, CCL = compacted clay liner
- 3) HDPE = high-density polyethylene, PVC = polyvinyl chloride
- 4) The LDS for Landfill P is overlain by a tertiary liner and drainage layer, not listed in the table due to space constraints. As such, Landfill P features two LDS layers from which data are available: LDS1 (upper LDS above tertiary liner and LDS2) and LDS2 (lower LDS above secondary liner).

Table II-3. Landfill cover system details

Designation		Protective soil thickness (inches)	Drainage layer(s)		Barrier layer			
Landfill	Sub-group of study units		Type ⁽¹⁾	Soil thickness (inches)	Type ⁽²⁾	Material ⁽³⁾	Thickness (mil)	Compacted clay liner thickness (inches)
B	B-1 to B-6	6	S	18	CCL/GM	HDPE	30	24
	B-7 to B-8	6	S	18	GM/CCL	HDPE	60	24
T	T-1 to T-18	24	GN	--	GM/CCL	HDPE	60	24
J ⁽⁴⁾	J-1 to J-3	24	GC	--	GM	HDPE	60	--
R	R-1 to R-5	24	GT	--	GM/CCL	HDPE	80	24
P	P-1	36	GN	--	GM/CCL	HDPE	60	24
	P-2 to P-4	36	GN	--	GM/GCL	HDPE	60	--
Y	Y-1 to Y-3	30	GC	--	GM/GCL	HDPE	40	--
M ⁽⁵⁾	M-1	-	GF	18	--	--	--	--
D	D-1 to D-2	24	GC	--	GM/GCL	LLDPE	40	--
F	F-1	12	S	12	GM/GCL	LLDPE	40	--

Notes:

- 1) GN = geonet, GT = geotextile, GC = geocomposite, S = sand, GF = general fill
- 2) GM = geomembrane, GCL = geosynthetic clay liner, CCL = compacted clay liner
- 3) HDPE = high-density polyethylene, LLDPE = linear low density polyethylene
- 4) Landfill J is situated below an MSW overfill landfill; the liner system of the overfill landfill is integrated with the cover system of Landfill J units.
- 5) Landfill M unit is currently inactive with intermediate soil cover pending final cover system construction; the final cover will comprise a 36-inch all-soil evapotranspirative cover system.

Table II-4B. Annual average LCRS and LDS flow rate (gpad) at Landfill T

Landfill unit	Sampling		Years into post-closure care program																																		
	Layer	Interval	Total Data	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29					
T-1	LCRS	Monthly	58	16	9.1	0.3	0.3	0.3	0.1	0.1																											
	LDS		58	2.5	2.7	3.0	4.4	2.4	0.8	0.5																											
T-2	LCRS	Monthly	57	69	34	23	14	5.1	6.4	4.0																											
	LDS		63	4.1	3.6	2.7	1.5	1.3	1.7	1.2																											
T-3	LCRS	Monthly	55	34	8	51	43	19	12	6.5																											
	LDS		70	8.7	10	5.3	5.3	3.1	4.3	3.9																											
T-4	LCRS	Monthly	51	50	7.1	17	7.3	14	0.1	0.2																											
	LDS		54	5.9	4.3	4.6	3.6	2.4	0.1	0.2																											
T-5	LCRS	Monthly	48	352	125	73	48	30	1.0	0.1																											
	LDS		70	5.9	4.5	7.0	10	13	15	16																											
T-6	LCRS	Monthly	59	21	12	6.8	7.5	4.0	3.7	0.9																											
	LDS		50	12	6.4	4.3	3.2	2.7	1.3	0.0																											
T-7	LCRS	Daily	1591	40																																	
	LDS		390	7.7																																	
T-8	LCRS	Daily	1519	32																																	
	LDS		1356	26																																	
T-9	LCRS	Daily	1365																																		
	LDS		979																																		
T-10	LCRS	Daily	937																																		
	LDS		1108																																		
T-11	LCRS	Daily	1615																																		
	LDS		1047																																		
T-12	LCRS	Daily	1497																																		
	LDS		592																																		
T-13	LCRS	Daily	1457																																		
	LDS		401																																		
T-14	LCRS	Daily	1276																																		
	LDS		587																																		
T-15	LCRS	Daily	1322																																		
	LDS		235																																		
T-16	LCRS	Daily	1535																																		
	LDS		1008																																		
T-17	LCRS	Daily	1535																																		
	LDS		175																																		
T-18	LCRS	Daily	1310																																		
	LDS		309																																		

Table II-4C. Annual average LCRS and LDS flow rate (gpad) at Landfills P, Y, M, D, and F

Landfill unit	Sampling		Years into post-closure care program																																		
	ID	Layer	Interval	Total Data	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29				
P-1	LCRS			1909				5.9	3.5	2.0	1.8	2.0	1.5	1.9	1.9	1.6	1.8	0.7	0.4	0.7	0.6	0.5	0.8	0.4	0.2												
	LDS1	Weekly		3577				11	19	23	18	14	24	34	32	6.5	3.7	3.3	2.6	3.0	1.5	2.0	1.1	1.0	4.9												
	LDS2			3258				12	17	19	13	17	11	22	16	5.6	2.7	1.9	1.8	1.6	6.4	1.1	6.8	1.3	1.2												
P-2	LCRS			1793	8.1	4.8	2.7	2.2	3.6	1.8	1.0	0.2	0.1	0.1	0.0	0.2	0.3	0.5	0.6	0.2	0.1																
	LDS1	Weekly		3399	21	18	11	6.3	6.4	8.4	10	1.8	1.5	1.4	1.1	1.5	8.2	8.6	8.8	8.0	3.4																
	LDS2			3268	17	27	18	25	16	27	17	5	21	20	15	14	6.0	8.4	8.9	8.8	10																
P-3	LCRS			1520	7.0	3.8	3.9	2.4	2.4	2.2	2.0	1.6	1.3	1.1	1.0	0.8	1.0	0.7	0.8	0.6	0.6																
	LDS1	Weekly		3389	3.7	2.8	3.4	1.8	1.1	3.1	1.5	0.8	2.8	1.3	1.9	6.5	2.0	4.9	4.0	2.4	1.6																
	LDS2			3160	4.7	8.0	8.3	10	5.4	10	5.2	1.1	8.4	10	7.2	6.1	2.7	5.3	6.1	5.9	5.7																
P-4	LCRS			1157	19	13	8.2	6.9	6.6	3.9	2.4	3.6	3.6	2.1	3.1	2.0	2.5																				
	LDS1	Weekly		3239	2.4	1.4	3.9	3.9	1.9	1.0	0.8	0.8	2.0	1.5	1.8	2.2	1.7																				
	LDS2			3211	4.2	8.7	6.6	1.2	1.1	4.8	7.5	6.6	2.2	6.2	5.9	5.1	5.5																				
Y-1	LCRS			99	4.4	3.8	3.3	2.4	2.7	2.3	2.3	2.8	2.5																								
	LDS	Monthly		99	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																					
Y-2	LCRS			102	5.9	5.0	4.4	3.6	4.1	2.6	2.2	2.3	2.1																								
	LDS	Monthly		102	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																					
Y-3	LCRS			101	5.1	4.3	3.4	2.3	3.4	2.2	2.1	2.3	2.2																								
	LDS	Monthly		101	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																					
M-1	LCRS			48	20	14	14	12																													
	LDS	Monthly		6	0.1	0.1	0.1	0.1																													
D-1	LCRS			103	40	40	25	22	30	23	22	21	20																								
	LDS	Monthly		19	1.7	0.0	0.0	0.8	5.9	4.3	1.8	3.5	2.6																								
D-2	LCRS			72	14	6.0	3.4	5.1	4.5	3.6																											
	LDS	Monthly		72	27	29	31	24	20	17																											
F-1	LCRS			288	179	310	164	160	148	126	275	206	100	49	58	24																					
	LDS	Monthly		141	58	27	33	13	12	7.2	2.4	2.1	1.8	1.6	2.3	5.0																					

Table II-5A. Annual peak LCRS and LDS flow rate (gpad) at Landfills B, J, and R

Landfill unit	Years into post-closure care program																													
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	
B-1	LCRS									22	22	24	18	16	16	15	13	36	16	7.3	10	10	51	19	15	12	10	8.8	7.9	
	LDS									4.6	6.7	6.3	4.4	4.6	4.7	3.9	5.6	4.3	10.2	3.0	6.1	3.3	4.6	3.6	5.5	6.3	6.9	4.0	3.8	
B-2	LCRS									13	14	11	12	10	12	17	16	27	10	6.3	9.0	6.9	6.5	5.7	20	5.5	5.7	5.6	6.4	
	LDS									4.5	7.7	9.4	7.2	8.9	6.0	10	5.7	4.1	5.6	2.4	3.1	1.2	1.6	2.8	1.7	1.7	2.1	2.0	1.1	
B-3	LCRS									32	31	29	22	26	20	17	12	11	10	7	9	9	18	17	15	16	14	11		
	LDS									7.0	8.5	10	4.8	4.9	6.4	4.4	5.4	5.1	10	5.3	3.4	2.6	3.2	2.5	2.5	2.6	3.0	3.2	3.1	
B-4	LCRS									31	33	26	24	23	19	17	19	20	22	10	11	17	17	22	10	11	10	8.7		
	LDS									4.8	6.3	6.7	10	13	10	8.0	5.2	8.0	4.0	16	10	4.0	1.9	2.5	2.0	2.6	1.8	2.0	1.0	
B-5	LCRS									12	12	12	9.4	8.3	9.4	5.6	5.1	11	14	11	6.8	4.6	8.4	7.6	5.3	3.3	6.1	3.6	3.8	
	LDS									8.2	50	43	25	21	17	26	22	24	8.8	8.9	6.5	8.1	10	5.7	12	10	19	11	14	
B-6	LCRS									75	94	101	69	42	49	85	184	836	429	264	202	118	79	79	70	47	33	31	28	
	LDS									30	28	23	19	22	19	7.6	32	24	12	5.8	11	8.3	10	8.2	4.6	4.3	7.6	3.5	3.0	
B-7	LCRS	1495	1147	771	1018	718	179	131	142	118	122	163	119	129	124	144	137	132												
	LDS	23	7.5	19	5.8	6.7	6.7	11	13	22	46	37	58	48	60	60	39	41												
B-8	LCRS	76	34	16	16	28	58	24	21	15	4.2	3.4																		
	LDS	5.7	5.5	5.2	6.5	7.3	6.7	5.4	3.4	103	188	188																		
J-1	LCRS	2.3	1.3	4.0	2.7	3.5	0.0	1.7	2.6	6.8	9.1	8.7	9.3	4.2	5.8	10.3	8.4	0.0												
	LDS	3.2	2.2	2.9	2.3	2.3	2.4	4.5	5.7	8.4	19.3	5.4	6.2	5.6	5.7	5.4	5.0	5.0												
J-2	LCRS	0.0	0.0	0.8	0.2	0.1	0.0	0.0	0.0	0.0	4.9	0.0	0.0	3.2	0.0	0.0	7.0	0.0												
	LDS	2.2	2.3	2.5	1.9	1.8	1.9	0.3	2.7	0.0	5.9	6.3	6.5	5.4	7.0	0.0	7.0	0.0												
J-3	LCRS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.3	0.0	0.8	4.5	0.0	0.0	0.0	0.0												
	LDS	2.2	2.1	2.5	2.2	2.1	2.2	2.0	3.3	4.3	3.5	4.6	3.7	4.3	4.0	4.2	3.8	0.0												
R-1	LCRS	15	12	7.1	9.3	6.2	4.4	3.9	3.1	11	7.7	5.7	2.7	3.2																
	LDS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	6.7	0.0	0.0	0.0																
R-2	LCRS	8.7	7.2	7.2	7.3	6.2	5.9	9.6	4.2	7.7	6.3	5.6	4.0	12																
	LDS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																
R-3	LCRS	4.4	7.6	7.9	3.9	3.7	4.8	3.0	3.5	4.7	2.6	2.3	3.3	4.1																
	LDS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																
R-4	LCRS	6.2	6.5	5.6	4.8	4.6	4.7	4.6	4.3	6.3	4.8	6.6	3.9	4.5																
	LDS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																
R-5	LCRS	4.4	7.7	3.5	2.9	4.4	2.5	5.3	2.0	3.2	6.0	1.8	3.3	1.8																
	LDS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																

Table II-5C. Annual peak LCRS and LDS flow rate (gpad) at Landfills P, Y, M, D, and F

Landfill unit	Years into post-closure care program																																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29				
P-1	LCRS			12	5.5	3.4	3.4	2.9	2.4	5.1	6.7	3.0	6.5	2.3	2.3	2.7	2.2	1.8	1.5	2.0	0.9												
	LDS1			24	36	36	36	32	51	68	101	11	99	50	49	37	26	32	19	16	8.9												
	LDS2			33	30	27	25	34	22	44	35	15	52	29	33	27	18	20	15	19	15												
P-2	LCRS	12	13	5.1	6.7	21	6.0	2.2	0.7	1.6	0.8	0.0	0.9	1.2	1.4	1.3	0.6	0.5															
	LDS1	42	28	23	15	11	21	25	3.2	38	19	21	22	19	15	16	11	5.5															
	LDS2	44	39	45	51	32	55	33	17	40	31	36	27	21	17	20	14	13															
P-3	LCRS	10	5.9	7.9	3.4	4.0	3.2	4.5	2.5	1.8	2.3	3.6	2.9	2.9	1.4	1.7	1.1	2.0															
	LDS1	5.5	3.9	10	3.5	2.4	11	4.6	1.6	6.9	2.6	7.4	12	4.9	8.5	10	5.6	2.5															
	LDS2	6.5	17	21	22	13	24	18	3.2	15	20	16	10	12	12	13	9.5	8.6															
P-4	LCRS	38	21	13	14	10	15	5.2	11	6.1	3.9	6.1	5.1	4.3																			
	LDS1	38	4.0	10	8.5	3.4	2.3	2.3	3.1	3.4	2.6	3.4	4.4	2.9																			
	LDS2	11	15	14	3.1	24	8.5	16	15	6.3	13	11	10	13																			
Y-1	LCRS		5.3	4.6	4.0	4.8	6.9	3.7	5.3	4.4	5.2																						
	LDS		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																						
Y-2	LCRS		7.2	5.9	5.5	5.7	7.2	6.5	4.5	3.7	4.4																						
	LDS		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0																						
Y-3	LCRS		6.3	5.6	5.2	3.6	9.1	4.4	4.7	3.9	4.6																						
	LDS		0.1	0.1	0.1	0.1	0.2	0.0	0.0	0.0	0.0																						
M-1	LCRS	34	25	14	14																												
	LDS	1.6	0.8	0.8	0.8																												
D-1	LCRS	104	65	67	39	46	45	35	32	33																							
	LDS	21	0.0	0.0	9.3	20	22	19	15	11																							
D-2	LCRS	23	13	5.1	9.2	9.2	8.2																										
	LDS	33	35	34	31	26	42																										
F-1	LCRS	214	536	200	220	193	142	333	290	121	90	107	34																				
	LDS	129	49	248	19	17	9.2	3.7	2.5	2.9	1.8	3.0	19																				

Table II-6. Summary of leachate data availability for constituents of interest

Chemical constituent	Data availability at landfill ⁽²⁾																		
	B		T		J		R		P			Y		M		D		F	
	L CRS	L DS	L CRS	L DS	L CRS	L DS	L CRS	L DS	L CRS	L DS1	L DS2	L CRS	L DS						
pH	N	N	Y	Y	Y	Y	Y	NF	Y	Y	Y	N	NF	Y	NF	N	N	N	N
Specific conductance	N	N	Y	Y	Y	Y	Y	NF	Y	Y	Y	N	NF	Y	NF	N	N	N	N
Total dissolved solids	N	Y	N	N	N	N	Y	NF	N	N	N	N	NF	N	NF	N	N	N	N
COD ⁽¹⁾	N	N	N	N	N	N	N	NF	N	N	N	N	NF	N	NF	N	N	N	N
BOD ⁽¹⁾	N	N	N	N	N	N	N	NF	N	N	N	N	NF	N	NF	N	N	N	N
Total organic carbon	N	N	Y	Y	N	N	N	NF	Y	Y	Y	N	NF	N	NF	N	N	N	N
Alkalinity	N	N	N	N	N	N	N	NF	N	N	N	N	NF	N	NF	N	N	N	N
Chloride	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Sulfate	N	N	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	N	NF	N	N	N	N
Calcium	Y	N	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	N	NF	N	N	Y	Y
Magnesium	Y	N	N	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Sodium	N	N	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	N	NF	N	N	N	N
Potassium	N	N	N	N	Y	Y	Y	NF	N	N	N	N	NF	N	NF	N	N	N	N
Arsenic	Y	N	Y	Y	Y	Y	Y	NF	Y	Y	Y	N	NF	Y	NF	N	N	Y	Y
Cadmium	Y	N	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Chromium	Y	N	Y	Y	N	N	Y	NF	Y	Y	N	N	NF	Y	NF	N	N	N	N
Lead	Y	N	Y	Y	Y	Y	Y	NF	Y	Y	Y	N	NF	Y	NF	N	N	N	N
Nickel	Y	N	N	N	Y	Y	Y	NF	Y	Y	N	N	NF	Y	NF	N	N	Y	Y
Benzene	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	Y	Y
1,1-Dichloroethane	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
1,2-Dichloroethane	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
cis-1,2-Dichloroethene	Y	Y	N	N	N	N	N	NF	N	N	N	N	NF	Y	NF	N	N	N	N
trans-1,2-Dichloroethene	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Ethylbenzene	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Methylene chloride	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
1,1,1-Trichloroethane	Y	N	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Trichloroethylene	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Toluene	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Vinyl chloride	Y	Y	Y	Y	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N
Xylenes (total)	Y	Y	N	N	Y	Y	Y	NF	N	N	N	N	NF	Y	NF	N	N	N	N

Notes:
 1). COD = chemical oxygen demand, BOD = biochemical oxygen demand.
 2). Data availability: Y = leachate data are available (these cells are colored green for ease of identification), N = leachate data are not collected, although sufficient flow in the LCRS and/or LDS exists to enable a sample to be collected, NF = no flow in the LDS, which means sample collection and analysis cannot be conducted (these cells are colored gray for ease of identification).

11. APPENDIX III

Landfill		Landfill B						
Cells								
Years into PCC		19	19	19	20	20	20	21
Modules		B11	B12	B13	B11	B12	B13	B11
Parameter	Units							
pH	pH units							
Specific Conductance	µmhos/cm							
TDS	mg/L							
COD	mg/L							
BOD5	mg/L							
TOC	mg/L							
Alkalinity	mg/L							
Chloride	mg/L	14700		6860	11200	53.2	5750	9860
Sulfate	mg/L							
Calcium	mg/L				16.7	2280	811	55.9
Magnesium	mg/L				15.3	263	81.7	18.1
Sodium	mg/L							
Potassium	mg/L							
Arsenic	µg/l				2440	201	646	426
Cadmium	µg/l							ND
Chromium	µg/l							138
Lead	µg/l							ND
Nickel	µg/l							
Benzene	µg/l	ND	ND	ND	ND	48600	ND	324
1,1-Dichloroethane	µg/l	ND	ND	ND	ND	453000	194	1050
1,2-Dichloroethane	µg/l	ND	ND	914	ND	104000	726	ND
cis-1,2-Dichloroethylene	µg/l	1980	ND	ND	470	68300	121	3900
trans-1,2-Dichloroethylene	µg/l	ND	ND	ND	ND	ND	ND	ND
Ethylbenzene	µg/l	1690	4510	ND	ND	1090000	101	189
Methylene Chloride	µg/l	ND	289000	13600	ND	3490000	6730	ND
1,1,1-Trichloroethane	µg/l	ND	4130	ND	ND	481000	ND	ND
Trichloroethylene	µg/l	ND	ND	ND	ND	561000	69	ND
Toluene	µg/l	14800	28900	123	388	8670000	230	10600
Vinyl Chloride	µg/l	ND	ND	ND	ND	ND	75.1	850
Xylenes (total)	µg/l	16300	21300	154	ND	5040000	444	ND

17	17	18	18	18	19	19	19	20	20
B22	B23	B21	B22	B23	B21	B22	B23	B21	B22
7,330	4,470	8,450	6,310	3,900	8,560	5,150	3,290	11,800	802
		11.6	137	653	10.5	85.6	509	12.8	19
		30.1	136	290	30.3	49	273	38.4	6.83
		675	1550	1260	500	203	6120	404	25.3
					ND	ND	14.5	ND	ND
					270	97	ND	171	11.5
					ND	ND	ND	ND	ND
ND	ND	5.18	57.8	10.2	3.02	21.5	6.52	2.98	2.04
192	ND	4.8	207	65	2.21	197	49.7	2.76	20.5
ND	ND	ND	12.4	ND	ND	ND	ND	ND	ND
ND	ND	19.4	16.2	5.7	ND	8.71	2.33	ND	1.49
ND	ND	ND	ND	ND	ND	2.58	ND	ND	ND
130	138	15	186	163	5.21	51.2	63	7.53	1.54
449	ND	ND	313	5.36	4.04	10.4	6.57	ND	ND
ND	ND	ND	8.76	1.37	ND	9.55	0.946	ND	0.412
ND	ND	ND	10	3.97	ND	1.47	1.14	ND	0.275
737	136	91.6	1050	112	56.8	295	45.8	51.9	13.3
ND	ND	ND	21.5	2.53	ND	22.5	ND	ND	2.49
519	415	57.8	767	539	17.4	202	108	24.8	12.9

21	22	22	22	23	23	23	14	14	14
B33	B31	B32	B33	B31	B32	B33	B41	B42	B43
9,680	15,000	10,000	12,000	14,000	9,900	11,000	7,340	7,030	8,620
1.44				51	190	ND			
2.38				25	90	0.6			
263	290	630	510	220	1000	850			
10	6	73	30	0.006	43	0.03			
242	160	200	190	480	54	280			
33	19	19	95	160	210	450			
	1500	1700	950	1400	1300	600			
1.5	20	1	1	1.7	0.71	0.001	0.72	0.351	ND
1.5	30	3.8	1.5	8.3	3.1	0.0015	1.53	0.659	2.94
1.55	30	1.5	1.5	0.0015	0.0003	0.0015	ND	ND	ND
1.5	20	1	1	0.0042	0.00075	0.001	0.747	ND	ND
1.5	40	2	2	0.002	0.0004	0.002	ND	ND	ND
5.75	170	8.5	8.5	0.0085	0.0017	0.0085	4.81	1.67	ND
31.2	170	180	23	12	62000	21	12.9	2.38	12.4
1.63	20	1	1	0.001	0.3	0.001	ND	ND	ND
1.25	30	1.5	1.5	0.0016	0.0011	0.0015	ND	ND	ND
6	170	8.5	8.5	17	2.3	0.0085	6.24	ND	1.58
2.5	10	0.5	0.5	0.97	0.31	0.0005	ND	ND	ND
52.7	170	8.5	46	24	3.1	36	32.6	8.74	1.73

B-4									
15	15	15	16	16	16	17	17	17	18
B41	B42	B43	B41	B42	B43	B41	B42	B43	B41
6,080	5,580	11,900	4,620	5,030	8,220	3,990	4,210		5,520
52.3	97.9	20.2	50	144	43.1	0.369	52.2	26.2	41.1
30.8	57	22.8	26.6	59.8	21.4	27.4	51	13.2	28.9
779	125	1230	919	115	725	908	91.6	206	1140
			14.1	ND	ND	22	3.15	ND	15.8
			23.5	ND	243	24	20.1	121	35.6
			ND	ND	36.6	ND	ND	ND	ND
0.386	0.367	0.874	0.6	ND	ND	ND	ND	ND	0.93
1.88	1.93	4.51	1.57	0.37	ND	1.11	0.31	7.14	1.82
ND	ND	ND	ND	ND	ND	ND	ND	ND	ND
ND	ND	ND	ND	ND	ND	ND	ND	ND	ND
ND	ND	ND	ND	ND	ND	ND	ND	ND	ND
3.12	5.15	1.57	2.08	0.818	ND	0.607	0.478	ND	2.64
ND	ND	ND	ND	ND	ND	ND	ND	ND	5.25
ND	ND	ND	ND	ND	ND	ND	ND	ND	ND
0.375	ND	ND	0.347	ND	ND	0.277	ND	ND	0.92
1.15	0.474	20.9	0.956	ND	ND	ND	ND	3.39	5.71
ND	ND	ND	ND	ND	ND	ND	ND	ND	ND
25.8	29.3	8.31	16.2	4.13	ND	4.73	1.77	ND	8.59

18	18	19	19	19	20	20	20	21	21
B42	B43	B41	B42	B43	B41	B42	B43	B41	B42
5,820	12,200	4,750	5,390	10,100	5,900	6,300	13,000	5,200	5,800
157	9.46	27.2	97.5	13.2				23	150
61.1	12.9	24.7	49.7	16.8				21	64
131	446	1740	101	652	820	140	1200	760	140
ND	ND	30.3	10	10	25	6	6	25	0.006
19.8	95.1	81.5	30	351	76	810	230	71	46
ND	ND	33	33	33	19	19	19	38	49
					2700	1200	560	2500	910
ND	0.74	0.3	0.3	30	0.2	0.4	20	0.24	0.0002
0.48	2.22	1.96	0.43	30	2.2	5.2	30	2.3	3.5
ND	ND	0.25	0.25	25	0.3	0.3	30	0.0003	0.0003
ND	ND	0.3	0.3	30	0.2	0.2	20	0.0002	0.0002
ND	ND	0.3	0.3	30	0.4	0.4	40	0.0004	0.0004
0.78	1.56	1.65	0.83	46	2.8	1.7	170	2.8	0.0017
ND	5.62	7.22	2	200	1.7	1.7	170	3	0.0017
ND	ND	0.325	0.325	32.5	0.2	0.2	20	0.0002	0.0002
ND	ND	0.73	0.25	25	1.4	0.3	30	0.0012	0.0003
ND	6.16	0.26	0.25	110	1.7	1.7	170	3.3	0.0017
ND	ND	0.5	0.5	50	0.1	0.1	10	0.0001	0.0001
ND	12.2	9.19	0.3	314	9.7	1.7	170	11	0.0017

21	13	13	13	14	14	14	15	15	15
B43	B51	B52	B53	B51	B52	B53	B51	B52	B53
11,000	8,650	6,700	4,760	8,830	1,960	4,260	6,240	2,670	11,500
20				96.6	361	354	673	341	162
30				40.2	67.9	49.4	54.5	75.5	43.7
1100				783	434	189	253	96.6	80.1
0.006							52.7	10.3	ND
230							ND	ND	ND
130							ND	29	ND
560									
0.001	1.36	ND	1.29	0.535	ND	2.97	ND	ND	1.45
1.9	3.53	ND	1.51	1.8	1.14	2.79	0.646	0.407	1.71
0.0015	ND	ND	ND	ND	0.286	ND	ND	ND	ND
0.001	1.08	ND							
0.002	ND								
0.0085	1.07	ND	15.2	0.254	ND	17.4	ND	ND	4.16
0.0085	5.05	10.5	ND						
0.001	ND								
0.0015	ND	ND	0.341	ND	ND	0.341	ND	ND	ND
0.0085	4.2	ND	6.43	1.6	0.3	11.2	0.887	ND	2.47
0.0005	ND								
0.0085	7.24	5.89	282	1.45	1.01	140	0.756	0.557	41.8

16	17	17	17	18	18	18	19	19	19
B63	B61	B62	B63	B61	B62	B63	B61	B62	B63
15,200		9,930	11,100	5,100	12,000	13,000	11,000	5,100	6,400
37.3	72.9	26.5	28.2				93	83	46
23.1	40.5	18.9	18.6				34	41	30
183	330	195	213	320	90	190	100	380	250
ND	1	1	1	0.6	0.6	0.6	0.003	9.8	0.003
135	250	143	111	220	72	250	76	270	150
ND	5.25	3.3	23.8	1.9	1.9	20	24	12	52
				1400	230	270	350	2000	350
15.2	7.4	7.56	6.5	12	1.4	8.9	3.7	5.4	5.6
8.94	6.7	3.13	5	8.8	3.2	5.9	3.4	7	6.6
ND	2.5	0.25	1.25	3	0.3	1.5	0.0015	0.0015	0.0015
1.23	3	0.3	1.5	4.1	0.37	1.6	0.001	0.001	0.001
ND	3	0.3	1.5	4	0.4	2	0.002	0.002	0.002
10.6	13.3	2.89	6.6	17	1.7	8.5	0.0085	13	0.0085
ND	20	2	10	17	1.7	8.5	0.0085	0.0085	0.0085
ND	3.25	0.325	1.63	2	9.7	1	0.001	0.001	0.001
0.62	2.5	0.25	1.25	3	0.7	1.5	0.0015	0.0015	0.0015
53.9	58	8.48	24.6	87	3.1	21	0.0085	54	15
1.88	5	0.5	2.5	9.5	0.1	2.8	1.1	1.5	1.2
24.9	166	7.33	16.6	99	4.7	16	0.0085	150	11

F-1									Landfill J
4	5	6	7	8	9	10	11	12	0
								7	7.6
									9750
676	650	552	287	354	342	380	333	386	
ND	13.2	14	13	10	4	5	8	4	10
									5
ND	ND	ND	ND	18	ND	3	ND	ND	
ND	ND	ND	14	26	44	2.6	ND	ND	5
ND	ND	ND	ND	ND	ND	ND	ND	ND	12.0
									5
ND	ND	ND				ND	ND	ND	
ND	ND	ND	ND	ND	ND	ND	ND	ND	10
ND	ND	ND	ND	ND	ND	ND	ND	ND	5
									5
ND	ND	ND	ND	ND	ND	ND	ND	ND	1.1
ND	ND	ND	ND	ND	ND	ND	ND	ND	3.7
ND	ND	ND	ND	ND	ND	ND	ND	ND	5
ND	ND	ND	ND	ND	ND	ND	ND	ND	10
									10

J-1									
1	2	3	4	5	6	7	8	9	10
7.6	7.2	7.1	6.9	7.1	6.6	7.3	6.2	6.8	6.9
9890	11360		12640	13938	17410	15324	25509	34700	33810
2540	2770	2840	3300	3400	3300	4100	7200	7200	8500
1730	1710	1880	1900	2100	2400	2900	4500	5200	6000
352	390	439	480	460	400	410	630	530	640
823	830	952	930	1000	920	1100	1800	1600	1900
22	22	25	29	29	36	45	130	140	300
952	936	1190	1400	1400	1500	1900	4200	3800	5800
10	10	10	10	10	10	11	28	9	32
5	5	5	5	5	5	5	5	1	3
3	8	3	3	18	5	8	15	9	5
40	40	40	42	270	340	160	230	260	330
10	10	10	5	5	5	5	5	1	1
13.0	25.0	31.0	18.0	23.0	12.0	5.0	15.0	13.0	11.0
10	10	10	5	5	5	5	5	1	1
20	20	20	10	10	10	10	10	1	1
10	10	10	5	5	5	5	5	1	1
10	10	10	5	5	5.0	5.0	5.0	5.0	5.0
10	10	10	5	5	5.0	5.0	5.0	1.0	1.0
10	10	10	5	5	5.3	5.0	6.3	5.4	6.2
10	10	10	5	5	5	5	5	1	1
20	20	20	10	10	10	10	10	1	1
20	20	20	10	10	10	10	10	2	2

11	12	13	14	15	16	0	1	2	3
6.9	7.3	7.1	7.2	7.5	7.1			7.5	7.4
38400	42900	43161	43700	44748	45563			1630	1675
9200	9400	9500	10000	10000	11000			209	207
6600	7200	8700	7800	7800	7700			203	200
760	810	860	840	810	790			69.4	50.7
2000	2000	2000	2100	2100	2100			94.4	104
480	560	740	720	800	890			5.58	5.94
7300	8600	7800	8000	8700	8600			143	159
40	57	60	63	80	96			10	10
1	4	25	10	6	4			5	5
9	9	45	140	13	43			21	5
240	380	1100	2100	1000	410			219	55
1	1	1	1	1	1			100	10
3.5	7.1	7.7	5.0	8.2	5.9			100	24
1	1	1	1	1	1			100	10
1	1	1	1	1	1			200	20
1	1	1	1	1	1			100	10
5.0	0.6	2.0	2.0	2.0	2.9			100	10
1.0	1.0	1.0	1.0	1.0	1.0			100	10
	0.9	1.1	1.3	2.1	1.5			100	41
1	1	1	1	0.57	1			100	10
1	1	1	1	1	1			200	20
2	2	2		2	2			200	20

J-2									
4	5	6	7	8	9	10	11	12	13
7.6	7.0		7.2	7.0	6.4	7.3	7.1	7.0	7.3
1560	7374		6966	6356	17134	17450	18500	14190	19186
220	510		610	4200	4700	4000	4100	4400	4300
190	2400		3000	5800	6500	5700	5700	6200	6600
45	170		130	520	500	510	470	500	480
100	470		600	1800	1600	1600	1400	1800	1900
6	13		16	46	40	31	38	34	39
160	690		900	2300	2000	2000	2100	2100	2400
10	10		10	10	6	15	6	15	15
5	5		5	5	2	2	2	2	1
3	3		68	8	34	9	9	3	9
40	220		3700	590	480	250	290	510	990
25	40		10	20	0	1	1	1	1
25	40		10	20	6	4	3	3	4
25	40		10	20	1	1	1	1	1
50	80		20	40	1	1	1	1	1
25	40		10	20	1	1	1	1	1
25	40		10	20	1	0	5	5	1
25	40		10	20	1	1	1	1	1
43	40		10	72	5	5	5	6	8
25	40		10	20	1	1	1	1	1
50	80		20	40	1	1	1	1	1
50	80		20	40	2	2	2	2	2

14	15	16	0	1	2	3	4	5	6
7.5	7.4	7.5							6.5
19760	19676	19009							14070
4700	5200	4400							4700
7200	8200	6400							25
450	510	490							760
1800	1700	1600							1100
38	37	39							29
2500	2600	2500							570
5	5	10							10
1	5	1							5
4	9	14							3
950	1300	1200							370
1	1	1							100
3	6	3							100
1	1	1							100
1	1	1							200
1	1	1							100
2	2	2							100
1	1	1							100
7	12	8							100
1	1	1							100
1	1	1							200
2	2	2							200

J-3									
7	8	9	10	11	12	13	14	15	16
6.8		6.7							
12286		10040							
4100		3500							
25		20							
560		430							
1000		770							
34		23							
540		450							
23		17							
5		1							
3.5		9							
510		400							
130		120							
130		36							
130		20							
270		20							
130		6							
130		9							
130		20							
130		20							
130		8							
270		20							
270		9							

Landfill M					Landfill P			
M-1								
0	1	2	3	4	5	6	7	8
8.0	7.7	7.5	7.5	7.5				
29615	19891	16387	14948	15994	66,900	70,500		
					546	516	2,010	337
5423	2544	2431	2625	2783				
				2400				
			322.5	340				
	440							
	1.5							
	33							
	290							
	0.35							
	6.3							
	1.1							
	0.82							
	16							
	0.22							
	0.57							

9	10	11	12	13	1	2	3	4	5
7.6									
35,300									
20,500									
7,850	9,600	10,000	8,900	9,400	14,000	15,000	15,000		13,000
2,810	3,400.00	3,400	3,500	3,600	5,500	2,600	2,700		5,500
1,200	1,200.00	1,300	1,100	1,100	880	1,400	2,700		710.00
382	390	490	430	460	690	400	780		390
5,070	5,600	5,700	5,000	5,700	12,000	4,800	10,000		7,200
16.8	18.0	20.0	22.0	20.0	39	34	36		30
0	8.20	15	9	11	10	ND	ND		0.03
0	1	1	1	ND	ND	ND	ND		10
0	290	8	2	11	ND	ND	ND		50
0	ND	ND	ND	ND	110	ND	67		ND
0	180	140	180	170	110	ND	ND		130
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	1	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND
ND	ND	ND	ND	ND	ND	ND	ND		ND

R-2 to R-5								Landfill T	
6	7	8	9	10	11	12	13	0	1
			7.0					8.8	8.5
			14,131					39004	47100
			8,207						
								2167	3508
16,000			12,100	13,500	13,500	13,500	14,250	11590	14120
5,600			4,730	4,850	4,875	4,825	5,100	8933	9700
1,500			1,211	1,140	1,210	1,105	1,095	4270	2283
620			620	505	600	530	555		
7,900			9,153	8,875	9,150	8,600	9,450		
42			23	51	33	38	37		
20			0.04	32.00	39.00	17.75	14.15	102467	112550
10			0.01	7.90	9.45	4.65	4.76	549	483
50			0.02	2.17	6.70	1.80	2.39	407	143
50			0.00	3.95				50	ND
78			0.13	113.50	121.75	116.25	119.50		
ND			ND	0.40	0.79	1.30	ND	5	31
ND			ND	0.44	0.50	0.37	0.35	7	ND
ND								3	ND
ND									
ND								ND	ND
ND			ND	ND	ND	ND	ND	ND	ND
ND								718	307
ND								ND	ND
ND								11	35
ND			ND	ND	ND	ND	ND		
ND								2020	4424
ND			ND	ND	0.54	ND	ND		

T-1					T-2			
2	3	4	5	6	0	1	2	3
	8.7	9.0	8.7	9.0	7.7	5.9	5.6	8.2
	44000	41500	28050	32250				
	2925	8373	1352	702	4,498	3,818	3,438	3,311
	13200	14500	14500	14275	27,233	29,620	30,933	28,025
	9970	9505	10215	10488				
	1110	3895	4450	3658	4,557	17,482	23,467	14,150
					23,600	25,760	22,900	18,900
	78000	71750	109425	82800	154,333	52,580	63,933	125,000
	ND	ND	ND	11	935	186	321	110
	460	275	334	328	610	1,706	1,440	555
	ND	51	ND	610	35	51	92	151
	ND	26	18	10	135	95	56	176
	ND	1020	73	35	117	74	25	189
	ND	ND	16	ND	94	62	35	120
	ND	ND	5	ND	37	33	7	64
	ND	ND	ND	ND	168	113	45	288
	ND	38	37	ND	193	156	20	125
	ND	1708	344	ND	89	55	18	152
	ND	ND	4	ND	68	28	9	76
					224	146	91	240
	3590	4434	421	38	245	146	47	880

			T-3				
4	5	6	0	1	2	3	4
7.8	7.8	7.9	8.8	8.6	7.8	8.1	8.1
3,719	2,955	2,097	7,053	5,061	4,957	4,840	5,891
26,575	30,550	31,867	21,448	31,300	33,733	34,320	32,150
15,400	16,600	19,167	7,397	6,592	9,363	9,722	9,063
21,600	24,800	24,567	19,667	19,480	15,100	19,780	17,800
83,400	103,975	214,667	234,333	171,140	121,733	200,200	112,950
2	5	9	1,208	585	831	124	4
598	582	747	310	604	873	1,030	1,233
60	36	21	28	55	167	151	60
105	69	19	541	219	335	248	495
114	79	83	499	174	345	267	603
67	45	27	521	106	205	158	315
38	32	45	170	67	118	90	180
170	111	26	883	366	619	405	810
108	48	47	1,647	654	334	195	315
90	62	31	393	137	279	214	428
46	29	20	429	87	145	107	215
143	93	18	1,224	816	727	405	827
238	155	49	5,476	1,582	733	876	5,800

		T-5						
5	6	0	1	2	3	4	5	6
7.7	7.5	12.1	11.9	12.0	12.0	12.0	11.8	11.7
3,787	3,199	8,356	7,144	7,323	7,542	10,579	8,739	7,739
30,625	31,300	12,233	13,300	12,918	13,133	14,375	13,325	12,975
5,723	5,300	4,743	5,878	9,028	9,668	10,205	7,798	8,643
16,675	15,900	7,483	6,168	7,013	9,420	6,813	6,873	6,613
164,025	201,250	12,167	15,160	11,450	12,250	11,975	13,000	11,438
27	14	56	49	34	17	2	5	17
948	1,104	117	84	78	85	51	54	101
36	58	13	43	126	151	60	44	35
170	39	1,496	6,257	508	260	276	200	69
195	99	1,615	6,806	1,107	629	1,373	369	395
105	26	3,945	4,388	403	158	88	140	75
75	46	544	2,440	175	90	85	106	144
275	31	2,448	10,224	744	405	225	318	119
110	49	1,338	12,612	301	158	143	153	156
153	32	1,292	5,396	390	214	119	187	100
65	23	662	2,766	296	107	223	89	81
230	25	2,096	8,892	1,528	745	1,420	601	105
385	65	22,154	150,900	24,350	17,530	28,425	3,350	408

T-6							T-7	
0	1	2	3	4	5	6	0	1
12.0	12.2	10.8	12.1	9.4	11.5	11.7	9.4	9.6
							45250	30100
5,745	6,933	7,664	7,694	8,275	9,962	8,799	6,438	4,928
11,000	9,850	18,092	14,150	11,629	16,700	17,100	13,525	13,850
							7,215	6,235
3,300	4,710	7,516	7,545	5,483	7,198	7,928	6,970	7,338
4,930	5,998	9,706	6,953	5,058	8,515	8,575		
18,500	19,050	44,460	22,575	14,593	19,975	16,100	53,500	32,247
4	72	54	26	2	5	17		
17	40	128	61	26	50	66	122	201
5	45	126	134	72	39	15	55	20
490	2,361	340	328	224	170	113	584	460
470	2,438	285	324	172	225	565		
4,000	1,880	644	385	146	179	144	160	
160	827	89	110	58	125	256		
720	3,720	389	495	262	250	178	170	145
2,720	5,117	1,115	204	102	354	273	777	
380	1,963	199	261	139	169	176		
190	1,029	161	131	126	73	129		53
1,200	3,350	1,063	544	478	214	113		
15,100	19,000	9,660	5,658	3,881	4,580	733	1,027	435

12. APPENDIX IV

APPENDIX IV

LANDFILL B

CELLS B-1 TO B-6

INTERMEDIATE COVER

PERCOLATION/LEAKAGE THROUGH LAYER 9						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

=====

AVERAGE OF MONTHLY AVERAGED DAILY HEADS (INCHES)

=====

DAILY AVERAGE HEAD ON TOP OF LAYER 5						
AVERAGES	1.5041	1.8790	2.9000	2.9467	3.2444	2.9432
	2.1392	1.3659	1.0773	1.1179	1.4290	1.5142
STD. DEVIATIONS	1.0202	1.0153	1.0973	1.0597	1.2654	1.4057
	1.5756	1.2827	1.0547	1.0478	1.3296	1.3531

DAILY AVERAGE HEAD ON TOP OF LAYER 8						
AVERAGES	0.0068	0.0088	0.0073	0.0080	0.0089	0.0095
	0.0097	0.0092	0.0084	0.0076	0.0075	0.0070
STD. DEVIATIONS	0.0026	0.0023	0.0025	0.0024	0.0023	0.0024
	0.0029	0.0028	0.0029	0.0027	0.0025	0.0029

=====

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

=====

	INCHES	CU. FEET	PERCENT
PRECIPITATION	50.13 (8.994)	1294302.9	100.00
RUNOFF	0.241 (0.2687)	6511.62	0.481
EVAPOTRANSPIRATION	33.332 (3.6434)	899834.00	66.419
LATERAL DRAINAGE COLLECTED FROM LAYER 4	16.70790 (5.45715)	451233.621	31.3160
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.00071 (0.00197)	181.138	0.01337
AVERAGE HEAD ON TOP OF LAYER 5	3.977 (0.620)		
LATERAL DRAINAGE COLLECTED FROM LAYER 7	0.00665 (0.00669)	179.510	0.08326
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.00005 (0.00001)	1.360	0.00010
AVERAGE HEAD ON TOP OF LAYER 8	0.008 (0.000)		
CHANGE IN WATER STORAGE	-0.114 (4.5321)	-3077.20	-0.237

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	5.02	163319.281
RUNOFF	1.207	32601.972
DRAINAGE COLLECTED FROM LAYER 4	0.34818	4007.25708
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000050	1.36097
AVERAGE HEAD ON TOP OF LAYER 5	5.032	
MAXIMUM HEAD ON TOP OF LAYER 5	6.994	
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	23.3 FEET	
DRAINAGE COLLECTED FROM LAYER 7	0.00007	0.00595
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000000	0.00654
AVERAGE HEAD ON TOP OF LAYER 8	0.013	
MAXIMUM HEAD ON TOP OF LAYER 8	0.030	
LOCATION OF MAXIMUM HEAD IN LAYER 7 (DISTANCE FROM DRAIN)	0.6 FEET	
SNOW WATER	4.02	108976.0000
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.3837
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0580

*** Maximum heads are computed using McEnroe's equation. ***
 Reference: Maximum Saturated Depth over Landfill Liner
 by Bruce M. McEnroe, University of Kansas
 ASCE Journal of Environmental Engineering
 Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100

LAYER	(INCHES)	(VOL/VOL)
1	3.3683	0.1403
2	117.8800	0.2920
5	3.4587	0.3012
4	0.7313	0.3069
5	0.0000	0.0000
6	25.0200	0.4270

F	1.5740	0.1311
B	0.0000	0.0000
9	11.3770	0.4270
SNOW WATER	0.000	

FINAL COVER

EVAPORATIVE ZONE DEPTH	=	22.0	INCHES
INITIAL WATER IN EVAPORATIVE ZONE	=	3.039	INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE	=	0.414	INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	0.626	INCHES
INITIAL SNOW WATER	=	0.000	INCHES
INITIAL WATER IN LAYER MATERIALS	=	221.686	INCHES
TOTAL INITIAL WATER	=	221.686	INCHES
TOTAL SUBSURFACE INFLOW	=	0.00	INCHES/YEAR

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100							
	JAN/FEB	FEB/APR	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC	
PRECIPITATION							
TOTALS	4.47	4.15	4.75	3.89	3.30	4.73	
STD. DEVIATIONS	2.25	2.14	2.43	1.99	1.92	2.16	
	2.40	1.00	1.17	2.24	1.35	1.55	
RUNOFF							
TOTALS	0.010	0.011	0.017	0.009	0.007	0.003	
STD. DEVIATIONS	0.027	0.018	0.094	0.026	0.001	0.002	
	0.054	0.041	0.057	0.033	0.041	0.027	
	0.108	0.082	0.102	0.132	0.088	0.013	
EVAPOTRANSPIRATION							
TOTALS	1.572	1.050	1.483	2.827	3.086	4.054	
STD. DEVIATIONS	4.464	4.217	2.981	1.520	1.008	1.203	
	0.282	0.373	0.678	0.846	1.119	1.485	
	1.415	1.304	1.049	0.459	0.224	0.222	
LATERAL DRAINAGE COLLECTED FROM LAYER 2							
TOTALS	2.8080	2.4807	2.3439	1.4679	1.0370	0.5018	
STD. DEVIATIONS	0.8930	1.0717	1.3363	1.0936	1.1624	1.4977	
	1.4882	1.8222	1.7014	0.9755	0.8907	0.5010	
	1.0207	1.1718	1.4497	1.7515	1.2401	1.1549	
PERCOLATION/LEAKAGE THROUGH LAYER 4							
TOTALS	0.0067	0.0061	0.0066	0.0061	0.0062	0.0059	
STD. DEVIATIONS	0.0061	0.0062	0.0062	0.0064	0.0060	0.0063	
	0.0005	0.0005	0.0004	0.0003	0.0003	0.0002	
	0.0003	0.0003	0.0004	0.0005	0.0004	0.0004	
LATERAL DRAINAGE COLLECTED FROM LAYER 8							
TOTALS	0.0018	0.0012	0.0019	0.0019	0.0045	0.0051	
STD. DEVIATIONS	0.0062	0.0062	0.0018	0.0019	0.0059	0.0060	
	0.0012	0.0010	0.0012	0.0012	0.0012	0.0012	
	0.0012	0.0012	0.0011	0.0011	0.0010	0.0011	
PERCOLATION/LEAKAGE THROUGH LAYER 10							

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TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 11							
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 13							
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)							
DAILY AVERAGE HEAD ON TOP OF LAYER 3							
AVERAGES	1.2077	2.4446	3.0100	2.0511	1.4157	0.8106	
STD. DEVIATIONS	1.8738	2.1144	1.8448	1.2604	1.1112	0.6997	
	1.2418	1.3564	1.6740	1.9767	1.5709	1.4674	
DAILY AVERAGE HEAD ON TOP OF LAYER 5							
AVERAGES	0.0042	0.0032	0.0084	0.0087	0.0089	0.0090	
STD. DEVIATIONS	0.0039	0.0086	0.0083	0.0084	0.0083	0.0081	
	0.0017	0.0016	0.0017	0.0017	0.0017	0.0017	
	0.0017	0.0016	0.0016	0.0016	0.0015	0.0013	
DAILY AVERAGE HEAD ON TOP OF LAYER 12							
AVERAGES	0.0001	0.0001	0.0001	0.0001	0.0001	0.0001	
STD. DEVIATIONS	0.0001	0.0001	0.0001	0.0001	0.0001	0.0001	
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.15 (8.594)	1354507.9	100.00
RUNOFF	0.211 (0.2718)	5708.79	0.421
EVAPOTRANSPIRATION	11.200 (3.6099)	802064.81	68.944
LATERAL DRAINAGE COLLECTED	17.93918 (5.93241)	485837.437	35.86857

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FROM LAYER 2			
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.07468 (0.00276)	2016.871	0.14890
AVERAGE HEAD ON TOP OF LAYER 3	1.997 (0.599)		
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.07098 (0.01507)	1916.682	0.14152
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00005 (0.00001)	1.443	0.00011
AVERAGE HEAD ON TOP OF LAYER 9	0.009 (0.002)		
LATERAL DRAINAGE COLLECTED FROM LAYER 11	0.00005 (0.00001)	1.384	0.00010
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.00000 (0.00000)	0.083	0.00001
AVERAGE HEAD ON TOP OF LAYER 12	0.000 (0.000)		
CHANGE IN WATER STORAGE	-0.038 (1.2376)	-1026.50	-0.076

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	6.02	162817.281
RUNOFF	1.107	29905.7129
DRAINAGE COLLECTED FROM LAYER 2	0.88222	23626.28370
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000554	0.61437
AVERAGE HEAD ON TOP OF LAYER 3	21.136	
MAXIMUM HEAD ON TOP OF LAYER 3	24.007	
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	15.7 FEET	
DRAINAGE COLLECTED FROM LAYER 8	0.00015	6.85668
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000	0.00490
AVERAGE HEAD ON TOP OF LAYER 9	0.011	
MAXIMUM HEAD ON TOP OF LAYER 9	0.022	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.4 FEET	
DRAINAGE COLLECTED FROM LAYER 11	0.00000	0.00420
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.000000	0.00073
AVERAGE HEAD ON TOP OF LAYER 12	0.000	

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MAXIMUM HEAD ON TOP OF LAYER 12	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 11 (DISTANCE FROM DRAIN)	0.9 FEET	
SNOW WATER	4.02	108376.0000
MAXIMUM VEG. SOIL WATER (VOL/VOL)	0.4062	
MINIMUM VEG. SOIL WATER (VOL/VOL)	0.0289	

*** Maximum heads are computed using McInroe's equations. ***
 Reference: Maximum Saturated Depth over Landfill Liner by Bruce M. McInroe, University of Kansas, ASCE Journal of Environmental Engineering, Vol. 119, No. 2, March 1993, pp. 362-370.

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)
1	0.2504	0.0584
2	1.3563	0.0754
3	10.2480	0.4270
4	0.0000	0.0000
5	3.4297	0.1429
6	119.0800	0.2920
7	1.7114	0.1428
8	0.5433	0.0453
9	0.0000	0.0000
10	25.6200	0.4270
11	1.5770	0.1340
12	0.0000	0.0000
13	13.3720	0.4270
SNOW WATER	0.000	

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30-YEAR DURATION

```

#
#*****
#
# HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
# HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
# DEVELOPED BY ENVIRONMENTAL LABORATORY
# USEA WATERWAYS EXPERIMENT STATION
# FOR US EPA RISK REDUCTION ENGINEERING LABORATORY
#*****
#
PRECIPITATION DATA FILE: C:\B0\U\DATA1.D4
TEMPERATURE DATA FILE: C:\B0\U\DATA7.D7
SOLAR RADIATION DATA FILE: C:\B0\U\DATA11.D13
EVAPOTRANSPIRATION DATA: C:\B0\U\DATA11.D11
SOIL AND DESIGN DATA FILE: C:\B0\U\DATA10.D10
OUTPUT DATA FILE: C:\B0\U\OUT\OUT

TIME: 17:16 DATE: 7/24/2016

-----
TITLE: B1 Pre-Closure
-----

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE
COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

LAYER 1
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 5
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2257 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

LAYER 2
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1B
THICKNESS = 540.00 INCHES
Page 1
    
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```

POROSITY = 0.0710 VOL/VOL
FIELD CAPACITY = 0.2930 VOL/VOL
WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.3079 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

LAYER 3
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 5
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2257 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

LAYER 4
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
WILTING POINT = 0.0150 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1074 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.99999997800E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 30.0 FEET

LAYER 5
-----
TYPE 2 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 15
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999900E-12 CM/SEC
FML PIMPLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

LAYER 6
-----
TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 60.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
Page 2
    
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WILTING POINT = 0.1360 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-08 CM/SEC

LAYER 7
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 5
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1311 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 10.0 FEET

LAYER 8
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 15
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999900E-12 CM/SEC
FML PIMPLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

LAYER 9
-----
TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 16.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.3970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-08 CM/SEC

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
SOIL DATA BASE USING SOIL TEXTURE # 2 WITH A
FAIR STAND OF GRASS, A SURFACE SLOPE OF 0.3%
AND A SLOPE LENGTH OF 50. FEET.

SCS RUNOFF CURVE NUMBER = 69.10
Page 3
    
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FRACTION OF AREA ALLOWING RUNOFF = 100.0 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE = 7.840 ACRES
EVAPORATIVE ZONE DEPTH = 21.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 4.953 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 10.954 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE = 1.376 INCHES
INITIAL SNOW WATER = 0.000 INCHES
INITIAL WATER IN LAYER MATERIALS = 218.178 INCHES
TOTAL INITIAL WATER = 218.178 INCHES
TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

-----
ANNUAL TOTALS FOR YEAR 1
-----
PRECIPITATION INCHES CU. FEET PERCENT
16.48 1523360.820 100.00
RUNOFF 0.106 1851.938 0.19
EVAPOTRANSPIRATION 57.545 1624792.876 62.18
DRAINAGE COLLECTED FROM LAYER 4 18.4016 496977.281 32.58
PERC./LEAKAGE THROUGH LAYER 5 0.007350 199.516 0.21
AVG. HEAD ON TOP OF LAYER 5 2.1870
DRAINAGE COLLECTED FROM LAYER 7 0.0084 171.180 0.08
PERC./LEAKAGE THROUGH LAYER 8 0.000040 1.172 0.00
AVG. HEAD ON TOP OF LAYER 8 0.0077
CHANGE IN WATER STORAGE 0.021 171.167 0.04
SOIL WATER AT START OF YEAR 218.178 5892981.000
SOIL WATER AT END OF YEAR 218.199 5892982.000
SNOW WATER AT START OF YEAR 0.000 0.000 0.00
SNOW WATER AT END OF YEAR 0.000 0.000 0.00
ANNUAL WATER BUDGET BALANCE 0.0000 -0.083 0.00

-----
ANNUAL TOTALS FOR YEAR 2
-----
PRECIPITATION INCHES CU. FEET PERCENT
18.55 1581271.500 100.00
RUNOFF 0.334 9010.978 0.57
EVAPOTRANSPIRATION 17.012 909196.187 60.21
DRAINAGE COLLECTED FROM LAYER 4 28.8562 779225.002 49.29
Page 4
    
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PERC./LEAKAGE THROUGH LAYER 6	0.013029	297.863	0.02
AVG. HEAD ON TOP OF LAYER 5	2.3561		
DRAINAGE COLLECTED FROM LAYER 7	0.0094	234.020	0.02
PERC./LEAKAGE THROUGH LAYER 8	0.000068	1.829	0.00
AVG. HEAD ON TOP OF LAYER 8	0.0213		
CHANGE IN WATER STORAGE	-7.062	-200810.469	-13.09
SOIL WATER AT START OF YEAR	218.139	5892512.000	
SOIL WATER AT END OF YEAR	210.338	5686093.300	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.164	0.00

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 2

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	2.26	3.84	3.86	4.73	3.07	8.36
STD. DEVIATIONS	0.91	0.90	0.47	4.06	2.47	3.45
RANGE						
TOTALS	0.000	0.000	0.083	0.001	0.083	0.001
STD. DEVIATIONS	0.000	0.000	0.117	0.000	0.000	0.000
EVAPOTRANSPIRATION						
TOTALS	1.321	2.036	2.793	2.803	4.045	6.232
STD. DEVIATIONS	0.148	0.038	0.046	1.083	0.343	0.093
LATERAL DRAINAGE COLLECTED FROM LAYER 4						
TOTALS	1.4600	2.7213	2.1891	2.9321	2.8612	3.1370
STD. DEVIATIONS	1.9413	1.9259	1.6010	1.1000	1.2930	0.8383
PERC./LEAKAGE THROUGH LAYER 6						
TOTALS	0.0079	0.3805	0.4278	0.3228	0.1828	0.3823
STD. DEVIATIONS	1.0805	2.3730	1.8130	0.8996	0.3415	0.3185

PERC./LEAKAGE THROUGH LAYER 6

TOTALS

0.0006

0.0010

0.0008

0.0010

0.0011

0.0013

STD. DEVIATIONS	0.0008	0.0007	0.0006	0.0003	0.0005	0.0004
LATERAL DRAINAGE COLLECTED FROM LAYER 7						
TOTALS	0.0004	0.0004	0.0006	0.0006	0.0007	0.0008
STD. DEVIATIONS	0.0008	0.0008	0.0008	0.0007	0.0007	0.0007
PERCOLATION/LEAKAGE THROUGH LAYER 9						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 5

AVERAGES	2.0059	4.1033	3.0371	3.6005	2.8188	4.3084
STD. DEVIATIONS	2.6380	2.5693	2.2628	1.6869	1.8335	1.1930

DAILY AVERAGE HEAD ON TOP OF LAYER 8

AVERAGES	0.0073	0.0067	0.0111	0.0206	0.0099	0.0093
STD. DEVIATIONS	0.0037	0.0013	0.0096	0.0005	0.0007	0.0007

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 2

	INCHES	CU. FEET	PERCENT
PRECIPITATION	17.31	(1.464)	1533219.1
RUNOFF	0.220	(0.1912)	5931.90
EVAPOTRANSPIRATION	37.479	(0.6597)	1028194.50
LATERAL DRAINAGE COLLECTED FROM LAYER 4	27.6289	(7.39252)	438150.187
PERCOLATION/LEAKAGE THROUGH LAYER 6	0.00919	(0.00760)	748.191
AVERAGE HEAD ON TOP OF LAYER 5	2.772	(0.8273)	
LATERAL DRAINAGE COLLECTED	0.00701	(0.00212)	213.606

FROM LAYER 7

PERCOLATION/LEAKAGE THROUGH LAYER 9

0.00000 (0.00001)

1.380

0.00010

AVERAGE HEAD ON TOP OF LAYER 8

0.010 (0.009)

CHANGE IN WATER STORAGE

-3.820 (1.4321)

-103172.65

-6.642

PEAK DAILY VALUES FOR YEARS 1 THROUGH 2

	(INCHES)	(CU. FT.)
PRECIPITATION	3.17	96415.702
RUNOFF	0.171	4812.0000
DRAINAGE COLLECTED FROM LAYER 4	0.1458	3939.87842
PERCOLATION/LEAKAGE THROUGH LAYER 6	0.000050	1.34738
AVERAGE HEAD ON TOP OF LAYER 5	3.768	
MAXIMUM HEAD ON TOP OF LAYER 5	6.993	
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	23.2 FEET	
DRAINAGE COLLECTED FROM LAYER 7	0.00007	0.80709
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000000	0.00636
AVERAGE HEAD ON TOP OF LAYER 8	0.031	
MAXIMUM HEAD ON TOP OF LAYER 8	0.029	
LOCATION OF MAXIMUM HEAD IN LAYER 7 (DISTANCE FROM DRAIN)	0.1 FEET	
SNOW WATER	1.35	41878.1710
MAXIMUM VEG. SOIL WATER (VOL./VOL.)		0.3140
MINIMUM VEG. SOIL WATER (VOL./VOL.)		0.0580

*** Maximum heads are computed using McInroe's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner by Bruce M. McInroe, University of Kansas, ASCE Journal of Environmental Engineering, Vol. 119, No. 3, March 1993, pp. 250-270.

FINAL WATER STORAGE AT END OF YEAR 2

LAYER	(INCHES)	(VOL./VOL.)
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1	6.2819	0.2617
2	136.6528	0.2988
3	2.3600	0.1567
4	0.0751	0.0163
5	0.0000	0.0000
6	23.6200	0.4270
7	1.9730	0.1313
8	0.0000	0.0000
9	15.3720	0.4270
SNOW WATER	0.000	

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#
#*****
#
#
# HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
# HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
# DEVELOPED BY ENVIRONMENTAL LABORATORY
# USAC WATERWAYS EXPERIMENT STATION
# FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
#*****
#
PRECIPITATION DATA FILE: C:\BLL\UDATA1.04
TEMPERATURE DATA FILE: C:\BLL\UDATA1.07
SOLAR RADIATION DATA FILE: C:\BLL\UDATA1.013
EVAPORATION DATA FILE: C:\BLL\UDATA1.011
SOIL AND DESIGN DATA FILE: C:\BLL\UDATA1.010
OUTPUT DATA FILE: C:\BLL\UDOUT.001

TIME: 16:53 DATE: 7/24/2016

-----
TITLE: B1 Post-Closure
-----

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER
WERE SPECIFIED BY THE USER.

LAYER 1
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 3
THICKNESS = 4.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1691 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

LAYER 2
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 18.00 INCHES
Page 1

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WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2918 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-02 CM/SEC

LAYER 7
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 3
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1967 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-02 CM/SEC

LAYER 8
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.0450 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0383 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.0000000000E-02 CM/SEC
SLOPE = 3.00 PERCENT
DRAINAGE LENGTH = 30.0 FEET

LAYER 9
-----
TYPE 4 - FLEXIBLE MEMBRANE LAYER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.09 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-12 CM/SEC
FML PENETRATION DEFECTS = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

LAYER 10
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 24.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.3970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
Page 3

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POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2609 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.0000000000E-02 CM/SEC
SLOPE = 3.00 PERCENT
DRAINAGE LENGTH = 30.0 FEET

LAYER 3
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 24.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.3970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-06 CM/SEC

LAYER 4
-----
TYPE 2 - FLEXIBLE MEMBRANE LAYER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.09 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-12 CM/SEC
FML PENETRATION DEFECTS = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

LAYER 5
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 3
THICKNESS = 24.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2627 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-02 CM/SEC

LAYER 6
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1R
THICKNESS = 18.00 INCHES
POROSITY = 0.0710 VOL/VOL
FIELD CAPACITY = 0.2970 VOL/VOL
Page 2

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EFFECTIVE SAT. HYD. COND. = 0.1000000000E-06 CM/SEC

LAYER 11
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1R
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1813 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-02 CM/SEC
SLOPE = 3.00 PERCENT
DRAINAGE LENGTH = 30.0 FEET

LAYER 12
-----
TYPE 4 - FLEXIBLE MEMBRANE LAYER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.09 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-12 CM/SEC
FML PENETRATION DEFECTS = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

LAYER 13
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 36.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.3970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-06 CM/SEC

GENERAL DESIGN AND EVAPORATIVE ZONE DATA
-----
NOTE: SCS NUMBER CURVE NUMBER WAS COMPUTED FROM DEPART
SOIL DATA BASE USING SOIL TEXTURE # 3 WITH A
FAIR STAND OF GRASS, A SURFACE SLOPE OF 2 %
AND A SLOPE LENGTH OF 30. FEET.

SCS RUNOFF CURVE NUMBER = 158.10
FRACTION OF AREA ALLOWING RUNOFF = 100.00 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE = 7.640 ACRES
EVAPORATIVE ZONE DEPTH = 22.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 3.322 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 8.414 INCHES
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LOWER LIMIT OF EVAPORATIVE STORAGE = 0.636 INCHES
 INITIAL SNOW WATER = 0.000 INCHES
 INITIAL WATER IN LAYER MATERIALS = 226.656 INCHES
 TOTAL INITIAL WATER = 226.656 INCHES
 TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

ANNUAL TOTALS FOR YEAR 1			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	16.48	152306.620	100.00
RUNOFF	0.102	2751.089	0.18
EVAPOTRANSPIRATION	16.345	986997.500	64.70
DRAINAGE COLLECTED FROM LAYER 2	19.7503	533399.312	34.87
PERC./LEAKAGE THROUGH LAYER 4	0.075341	2034.753	0.13
AVG. HEAD ON TOP OF LAYER 3	5.2263		
DRAINAGE COLLECTED FROM LAYER 8	4.0499	109272.320	7.16
PERC./LEAKAGE THROUGH LAYER 10	0.001819	49.600	0.00
AVG. HEAD ON TOP OF LAYER 9	0.1020		
DRAINAGE COLLECTED FROM LAYER 11	0.0048	129.540	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000038	1.024	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0058		
CHANGE IN WATER STORAGE	5.999	-107178.234	-7.07
SOIL WATER AT START OF YEAR	226.656	6131342.500	
SOIL WATER AT END OF YEAR	222.687	6024164.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.348	0.00

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ANNUAL TOTALS FOR YEAR 2			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	18.55	1591271.500	100.00
RUNOFF	0.281	7576.341	0.48
EVAPOTRANSPIRATION	15.376	953399.187	60.42

SOIL WATER AT START OF YEAR	211.817	3990864.500	
SOIL WATER AT END OF YEAR	220.100	3944297.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.802	0.00

ANNUAL TOTALS FOR YEAR 4			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	44.27	1191608.870	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	14.500	92638.187	77.48
DRAINAGE COLLECTED FROM LAYER 2	11.3829	307421.562	25.71
PERC./LEAKAGE THROUGH LAYER 4	0.072929	1969.610	0.16
AVG. HEAD ON TOP OF LAYER 3	1.3285		
DRAINAGE COLLECTED FROM LAYER 8	0.1487	408.373	0.34
PERC./LEAKAGE THROUGH LAYER 10	0.000102	2.750	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0179		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	0.149	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000004	0.101	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.561	-42170.727	-3.53
SOIL WATER AT START OF YEAR	220.100	3944297.500	
SOIL WATER AT END OF YEAR	218.539	3902126.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.210	0.00

ANNUAL TOTALS FOR YEAR 5			
	INCHES	CU. FEET	PERCENT

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DRAINAGE COLLECTED FROM LAYER 2	21.3669	611075.125	39.91
PERC./LEAKAGE THROUGH LAYER 4	0.076270	2058.771	0.13
AVG. HEAD ON TOP OF LAYER 3	2.5568		
DRAINAGE COLLECTED FROM LAYER 8	0.1962	10701.041	0.68
PERC./LEAKAGE THROUGH LAYER 10	0.000242	6.158	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0480		
DRAINAGE COLLECTED FROM LAYER 11	0.0007	19.521	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000009	0.242	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0009		
CHANGE IN WATER STORAGE	-0.870	-23499.842	-1.49
SOIL WATER AT START OF YEAR	222.687	6024164.000	
SOIL WATER AT END OF YEAR	221.817	5990664.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.188	0.00

ANNUAL TOTALS FOR YEAR 3			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	67.70	1828387.370	100.00
RUNOFF	1.424	38471.645	2.10
EVAPOTRANSPIRATION	53.262	952326.000	52.69
DRAINAGE COLLECTED FROM LAYER 2	32.5133	878091.937	48.09
PERC./LEAKAGE THROUGH LAYER 4	0.078498	2117.592	0.12
AVG. HEAD ON TOP OF LAYER 3	3.1029		
DRAINAGE COLLECTED FROM LAYER 8	0.2369	5959.557	0.32
PERC./LEAKAGE THROUGH LAYER 10	0.000142	0.394	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0262		
DRAINAGE COLLECTED FROM LAYER 11	0.0003	0.483	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000005	0.133	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0000		
CHANGE IN WATER STORAGE	-1.717	-46367.113	-2.54

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PRECIPITATION	60.46	1632854.870	100.00
RUNOFF	0.040	1068.238	0.07
EVAPOTRANSPIRATION	34.301	626561.500	56.73
DRAINAGE COLLECTED FROM LAYER 2	24.4389	660025.125	40.42
PERC./LEAKAGE THROUGH LAYER 4	0.076681	2076.947	0.13
AVG. HEAD ON TOP OF LAYER 3	2.7156		
DRAINAGE COLLECTED FROM LAYER 8	0.1180	3187.298	0.20
PERC./LEAKAGE THROUGH LAYER 10	0.000093	0.246	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0143		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	2.732	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000009	0.095	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	1.568	42111.527	2.59
SOIL WATER AT START OF YEAR	218.539	3902126.500	
SOIL WATER AT END OF YEAR	220.100	3944297.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-1.199	0.00

ANNUAL TOTALS FOR YEAR 6			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.54	1418958.370	100.00
RUNOFF	0.001	87.108	0.01
EVAPOTRANSPIRATION	31.820	956604.375	67.42
DRAINAGE COLLECTED FROM LAYER 2	16.9360	437934.375	32.27
PERC./LEAKAGE THROUGH LAYER 4	0.074389	2094.451	0.14
AVG. HEAD ON TOP OF LAYER 3	1.9860		
DRAINAGE COLLECTED FROM LAYER 8	0.1000	2700.373	0.19
PERC./LEAKAGE THROUGH LAYER 10	0.000071	1.944	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0121		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.981	0.00

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PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.090	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	0.090	1629.432	0.11
SOIL WATER AT START OF YEAR	210.104	394338.000	
SOIL WATER AT END OF YEAR	220.162	3941967.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.714	0.00

ANNUAL TOTALS FOR YEAR 7

	INCHES	CU. FEET	PERCENT
PRECIPITATION	44.71	1207492.250	100.00
RUNOFF	0.112	3012.307	0.25
EVAPOTRANSPIRATION	75.743	803264.437	66.52
DRAINAGE COLLECTED FROM LAYER 2	15.0012	403540.281	33.55
PERC./LEAKAGE THROUGH LAYER 4	0.073884	1995.664	0.17
AVG. HEAD ON TOP OF LAYER 3	1.7547		
DRAINAGE COLLECTED FROM LAYER 8	0.0972	2357.107	0.20
PERC./LEAKAGE THROUGH LAYER 10	0.000064	1.728	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0105		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.717	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.087	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-0.233	-6284.070	-0.52
SOIL WATER AT START OF YEAR	220.162	3945967.500	
SOIL WATER AT END OF YEAR	219.959	3939689.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.451	0.00

ANNUAL TOTALS FOR YEAR 8

	INCHES	CU. FEET	PERCENT
PRECIPITATION	50.52	1384408.620	100.00
RUNOFF	0.204	5312.488	0.40
EVAPOTRANSPIRATION	51.615	833829.687	67.58
DRAINAGE COLLECTED FROM LAYER 2	18.3883	502017.909	36.79
PERC./LEAKAGE THROUGH LAYER 4	0.074918	2029.321	0.15
AVG. HEAD ON TOP OF LAYER 3	2.0133		
DRAINAGE COLLECTED FROM LAYER 8	0.0810	2240.498	0.16
PERC./LEAKAGE THROUGH LAYER 10	0.000061	1.654	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0100		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.596	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.086	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	0.030	801.117	0.06
SOIL WATER AT START OF YEAR	219.930	3939689.500	
SOIL WATER AT END OF YEAR	219.959	3940484.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.728	0.00

ANNUAL TOTALS FOR YEAR 9

	INCHES	CU. FEET	PERCENT
PRECIPITATION	71.59	1933446.000	100.00
RUNOFF	0.545	14721.951	0.76
EVAPOTRANSPIRATION	87.424	1101719.100	57.28
DRAINAGE COLLECTED FROM LAYER 2	32.9707	890436.000	46.05
PERC./LEAKAGE THROUGH LAYER 4	0.078768	2126.770	0.11
AVG. HEAD ON TOP OF LAYER 3	3.4385		

DRAINAGE COLLECTED FROM LAYER 8	0.0792	2130.944	0.11
PERC./LEAKAGE THROUGH LAYER 10	0.000059	1.389	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0096		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.514	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.081	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	0.371	15426.445	0.80
SOIL WATER AT START OF YEAR	219.959	3940484.500	
SOIL WATER AT END OF YEAR	320.570	3955911.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.553	0.00

ANNUAL TOTALS FOR YEAR 10

	INCHES	CU. FEET	PERCENT
PRECIPITATION	51.10	1380667.750	100.00
RUNOFF	0.307	8283.198	0.60
EVAPOTRANSPIRATION	32.624	881073.125	63.84
DRAINAGE COLLECTED FROM LAYER 2	19.0704	513688.094	37.22
PERC./LEAKAGE THROUGH LAYER 4	0.074676	2016.787	0.15
AVG. HEAD ON TOP OF LAYER 3	1.9935		
DRAINAGE COLLECTED FROM LAYER 8	0.0756	2040.616	0.15
PERC./LEAKAGE THROUGH LAYER 10	0.000056	1.576	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.481	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-0.926	-23018.832	-1.81
SOIL WATER AT START OF YEAR	220.530	3955911.000	
SOIL WATER AT END OF YEAR	219.604	3930802.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00

SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.013	0.00

ANNUAL TOTALS FOR YEAR 11

	INCHES	CU. FEET	PERCENT
PRECIPITATION	44.65	1205872.000	100.00
RUNOFF	0.220	5939.685	0.49
EVAPOTRANSPIRATION	34.174	922947.437	76.54
DRAINAGE COLLECTED FROM LAYER 2	11.4359	308852.094	25.63
PERC./LEAKAGE THROUGH LAYER 4	0.072674	1962.719	0.16
AVG. HEAD ON TOP OF LAYER 3	1.3114		
DRAINAGE COLLECTED FROM LAYER 8	0.0751	2029.768	0.17
PERC./LEAKAGE THROUGH LAYER 10	0.000056	1.518	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.431	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000001	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.255	-33887.879	-2.81
SOIL WATER AT START OF YEAR	219.604	3930802.000	
SOIL WATER AT END OF YEAR	218.349	3906994.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.702	0.00

ANNUAL TOTALS FOR YEAR 12

	INCHES	CU. FEET	PERCENT
PRECIPITATION	38.18	1021125.000	100.00
RUNOFF	0.000	11.433	0.00
EVAPOTRANSPIRATION	25.340	684374.437	66.97

DRAINAGE COLLECTED FROM LAYER 2	11.4778	300983.719	30.00
PERC./LEAKAGE THROUGH LAYER 4	0.072984	1971.091	0.19
AVG. HEAD ON TOP OF LAYER 3	1.3494		
DRAINAGE COLLECTED FROM LAYER 8	0.0718	2046.789	0.20
PERC./LEAKAGE THROUGH LAYER 10	0.000057	1.510	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.441	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	1.285	34717.125	3.37
SOIL WATER AT START OF YEAR	218.349	3866904.500	
SOIL WATER AT END OF YEAR	219.634	3921711.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.017	0.00

ANNUAL TOTALS FOR YEAR 13			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	-44.14	1192097.710	100.00
RUNOFF	0.959	23904.738	2.17
EVAPOTRANSPIRATION	27.143	733057.437	61.49
DRAINAGE COLLECTED FROM LAYER 2	17.3593	474227.150	39.78
PERC./LEAKAGE THROUGH LAYER 4	0.074276	2005.592	0.17
AVG. HEAD ON TOP OF LAYER 3	1.8903		
DRAINAGE COLLECTED FROM LAYER 8	0.0766	2069.607	0.17
PERC./LEAKAGE THROUGH LAYER 10	0.000057	1.544	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.450	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.568	-43162.645	-3.62

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SOIL WATER AT START OF YEAR	219.634	3931711.500
SOIL WATER AT END OF YEAR	219.078	3885450.000
SNOW WATER AT START OF YEAR	0.000	0.000
SNOW WATER AT END OF YEAR	0.000	0.000
ANNUAL WATER BUDGET BALANCE	0.0000	-0.334

ANNUAL TOTALS FOR YEAR 14			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	48.17	1300936.870	100.00
RUNOFF	0.001	22.173	0.00
EVAPOTRANSPIRATION	32.998	891184.875	68.50
DRAINAGE COLLECTED FROM LAYER 2	10.7853	291551.408	22.41
PERC./LEAKAGE THROUGH LAYER 4	0.072631	1961.517	0.15
AVG. HEAD ON TOP OF LAYER 3	1.2422		
DRAINAGE COLLECTED FROM LAYER 8	0.0764	2082.090	0.16
PERC./LEAKAGE THROUGH LAYER 10	0.000057	1.540	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.468	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	4.299	116134.643	8.93
SOIL WATER AT START OF YEAR	218.036	3883450.000	
SOIL WATER AT END OF YEAR	221.897	3992828.000	
SNOW WATER AT START OF YEAR	0.000	0.000	
SNOW WATER AT END OF YEAR	0.438	11835.506	
ANNUAL WATER BUDGET BALANCE	0.0000	0.121	

ANNUAL TOTALS FOR YEAR 15			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	-48.32	1310389.120	100.00
RUNOFF	0.400	10811.490	0.83
EVAPOTRANSPIRATION	29.851	779448.812	59.48
DRAINAGE COLLECTED FROM LAYER 2	21.2566	574080.187	43.82
PERC./LEAKAGE THROUGH LAYER 4	0.075678	2043.858	0.16
AVG. HEAD ON TOP OF LAYER 3	2.3409		
DRAINAGE COLLECTED FROM LAYER 8	0.0745	2012.351	0.15
PERC./LEAKAGE THROUGH LAYER 10	0.000058	1.508	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.424	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-2.072	-53964.934	-4.27
SOIL WATER AT START OF YEAR	221.997	3992828.000	
SOIL WATER AT END OF YEAR	220.283	3948968.500	
SNOW WATER AT START OF YEAR	0.438	11835.506	
SNOW WATER AT END OF YEAR	0.000	0.000	
ANNUAL WATER BUDGET BALANCE	0.0000	-0.444	

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DRAINAGE COLLECTED FROM LAYER 2	17.3593	474227.150	39.78
PERC./LEAKAGE THROUGH LAYER 4	0.074276	2005.592	0.17
AVG. HEAD ON TOP OF LAYER 3	1.8903		
DRAINAGE COLLECTED FROM LAYER 8	0.0766	2069.607	0.17
PERC./LEAKAGE THROUGH LAYER 10	0.000057	1.544	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.450	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.568	-43162.645	-3.62

ANNUAL TOTALS FOR YEAR 16			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	48.32	1310389.120	100.00
RUNOFF	0.069	1868.316	0.14
EVAPOTRANSPIRATION	26.077	704257.750	53.74
DRAINAGE COLLECTED FROM LAYER 2	17.3593	474080.187	36.19
PERC./LEAKAGE THROUGH LAYER 4	0.073392	1982.120	0.15
AVG. HEAD ON TOP OF LAYER 3	1.8900		
DRAINAGE COLLECTED FROM LAYER 8	0.0751	2027.876	0.15
PERC./LEAKAGE THROUGH LAYER 10	0.000058	1.518	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		

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DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.446	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.607	-43396.719	-4.28
SOIL WATER AT START OF YEAR	220.263	3948968.500	
SOIL WATER AT END OF YEAR	218.477	3905302.000	
SNOW WATER AT START OF YEAR	0.000	0.000	
SNOW WATER AT END OF YEAR	0.000	0.000	
ANNUAL WATER BUDGET BALANCE	0.0000	0.510	

ANNUAL TOTALS FOR YEAR 17			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	46.31	1250701.750	100.00
RUNOFF	0.297	8016.974	0.64
EVAPOTRANSPIRATION	29.410	787511.875	61.37
DRAINAGE COLLECTED FROM LAYER 2	16.1856	442528.375	35.38
PERC./LEAKAGE THROUGH LAYER 4	0.074281	2000.119	0.16
AVG. HEAD ON TOP OF LAYER 3	1.8809		
DRAINAGE COLLECTED FROM LAYER 8	0.0733	1932.158	0.15
PERC./LEAKAGE THROUGH LAYER 10	0.000054	1.436	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0086		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.382	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.083	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	1.137	30733.189	2.46
SOIL WATER AT START OF YEAR	218.637	3905302.000	
SOIL WATER AT END OF YEAR	219.794	3936015.000	
SNOW WATER AT START OF YEAR	0.000	0.000	
SNOW WATER AT END OF YEAR	0.000	0.000	
ANNUAL WATER BUDGET BALANCE	0.0000	-0.809	

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ANNUAL TOTALS FOR YEAR 18

	INCHES	CU. FEET	PERCENT
PRECIPITATION	55.91	1451958.000	100.00
RUNOFF	0.336	9068.000	0.62
EVAPOTRANSPIRATION	34.469	920915.873	63.94
DRAINAGE COLLECTED FROM LAYER 2	18.6420	503468.056	34.58
PERC./LEAKAGE THROUGH LAYER 4	0.074416	2009.778	0.14
AVG. HEAD ON TOP OF LAYER 3	1.9061		
DRAINAGE COLLECTED FROM LAYER 8	0.0757	2044.586	0.14
PERC./LEAKAGE THROUGH LAYER 10	0.000017	1.526	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0092		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.422	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	0.387	10459.438	0.72
SOIL WATER AT START OF YEAR	219.794	5936021.000	
SOIL WATER AT END OF YEAR	220.181	5946474.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.051	0.00

ANNUAL TOTALS FOR YEAR 19

	INCHES	CU. FEET	PERCENT
PRECIPITATION	42.35	1149256.000	100.00
RUNOFF	0.100	2912.117	0.26
EVAPOTRANSPIRATION	27.217	731032.887	63.96
DRAINAGE COLLECTED FROM LAYER 2	15.0092	403854.375	35.27
PERC./LEAKAGE THROUGH LAYER 4	0.073901	1995.821	0.17
AVG. HEAD ON TOP OF LAYER 3	1.7868		

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DRAINAGE COLLECTED FROM LAYER 8	0.0725	1958.538	0.17
PERC./LEAKAGE THROUGH LAYER 10	0.000055	1.477	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0088		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.408	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000009	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	0.143	3955.582	0.34
SOIL WATER AT START OF YEAR	220.181	5946474.500	
SOIL WATER AT END OF YEAR	220.124	5910330.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.039	0.00

ANNUAL TOTALS FOR YEAR 20

	INCHES	CU. FEET	PERCENT
PRECIPITATION	72.40	1915221.500	100.00
RUNOFF	0.151	4092.245	0.21
EVAPOTRANSPIRATION	29.927	1078306.750	55.15
DRAINAGE COLLECTED FROM LAYER 2	32.3141	872713.625	44.88
PERC./LEAKAGE THROUGH LAYER 4	0.079178	2132.038	0.11
AVG. HEAD ON TOP OF LAYER 3	3.4950		
DRAINAGE COLLECTED FROM LAYER 8	0.0746	2014.877	0.10
PERC./LEAKAGE THROUGH LAYER 10	0.000056	1.509	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0090		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.415	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-0.007	-1804.573	-0.09
SOIL WATER AT START OF YEAR	220.324	5950330.000	
SOIL WATER AT END OF YEAR	220.257	5948321.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00

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SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.114	0.00

ANNUAL TOTALS FOR YEAR 21

	INCHES	CU. FEET	PERCENT
PRECIPITATION	46.82	1259345.870	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	30.311	824298.000	65.45
DRAINAGE COLLECTED FROM LAYER 2	16.0271	432576.531	34.35
PERC./LEAKAGE THROUGH LAYER 4	0.074253	2005.373	0.16
AVG. HEAD ON TOP OF LAYER 3	1.8696		
DRAINAGE COLLECTED FROM LAYER 8	0.0728	1985.823	0.16
PERC./LEAKAGE THROUGH LAYER 10	0.000055	1.478	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0088		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.418	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	0.019	503.171	0.04
SOIL WATER AT START OF YEAR	220.257	5948521.500	
SOIL WATER AT END OF YEAR	220.276	5949028.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.289	0.00

ANNUAL TOTALS FOR YEAR 22

	INCHES	CU. FEET	PERCENT
PRECIPITATION	55.27	1492887.750	100.00
RUNOFF	0.021	829.172	0.06

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EVAPOTRANSPIRATION	31.638	862488.062	64.48
DRAINAGE COLLECTED FROM LAYER 2	20.7108	559341.625	37.47
PERC./LEAKAGE THROUGH LAYER 4	0.075618	2042.235	0.14
AVG. HEAD ON TOP OF LAYER 3	2.1610		
DRAINAGE COLLECTED FROM LAYER 8	0.0732	1977.807	0.13
PERC./LEAKAGE THROUGH LAYER 10	0.000055	1.485	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0088		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.388	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000001	0.082	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.181	-31949.482	-2.14
SOIL WATER AT START OF YEAR	220.276	5949028.500	
SOIL WATER AT END OF YEAR	219.093	5917079.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.283	0.00

ANNUAL TOTALS FOR YEAR 23

	INCHES	CU. FEET	PERCENT
PRECIPITATION	58.95	1592874.500	100.00
RUNOFF	0.188	5797.917	0.23
EVAPOTRANSPIRATION	34.128	921700.375	57.89
DRAINAGE COLLECTED FROM LAYER 2	22.5314	605509.812	38.22
PERC./LEAKAGE THROUGH LAYER 4	0.073753	2043.388	0.13
AVG. HEAD ON TOP OF LAYER 3	2.1953		
DRAINAGE COLLECTED FROM LAYER 8	0.0764	2062.848	0.13
PERC./LEAKAGE THROUGH LAYER 10	0.000057	1.540	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0092		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.434	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000001	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		

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CHANGE IN WATER STORAGE	2.076	56061.699	3.32
SOIL WATER AT START OF YEAR	319.093	3937079.000	
SOIL WATER AT END OF YEAR	321.168	3993141.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.455	0.00

ANNUAL TOTALS FOR YEAR 24			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	31.98	1407834.500	100.00
RUNOFF	0.026	890.024	0.05
EVAPOTRANSPIRATION	13.007	891431.375	63.20
DRAINAGE COLLECTED FROM LAYER 2	17.7308	478856.344	34.13
PERC./LEAKAGE THROUGH LAYER 4	0.074942	2021.200	0.14
AVG. HEAD ON TOP OF LAYER 3	2.0448		
DRAINAGE COLLECTED FROM LAYER 8	0.0740	1999.192	0.14
PERC./LEAKAGE THROUGH LAYER 10	0.000056	1.500	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0099		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.430	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	1.142	30859.715	2.20
SOIL WATER AT START OF YEAR	223.168	573341.000	
SOIL WATER AT END OF YEAR	222.311	600194.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.264	0.00

ANNUAL TOTALS FOR YEAR 25			
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	INCHES	CU. FEET	PERCENT
PRECIPITATION	41.72	1126740.370	100.00
RUNOFF	0.183	4450.080	0.39
EVAPOTRANSPIRATION	29.399	794687.312	70.49
DRAINAGE COLLECTED FROM LAYER 2	19.8695	363773.344	32.29
PERC./LEAKAGE THROUGH LAYER 4	0.073312	1979.903	0.18
AVG. HEAD ON TOP OF LAYER 3	1.5277		
DRAINAGE COLLECTED FROM LAYER 8	0.0760	2033.166	0.18
PERC./LEAKAGE THROUGH LAYER 10	0.000057	1.534	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0092		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.433	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.389	-37425.840	-3.32
SOIL WATER AT START OF YEAR	225.311	600394.500	
SOIL WATER AT END OF YEAR	220.925	396508.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.435	0.00

ANNUAL TOTALS FOR YEAR 26			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	46.66	1260156.000	100.00
RUNOFF	0.225	6089.497	0.49
EVAPOTRANSPIRATION	27.021	737855.875	58.55
DRAINAGE COLLECTED FROM LAYER 2	19.3439	527771.000	41.89
PERC./LEAKAGE THROUGH LAYER 4	0.074630	2014.542	0.16
AVG. HEAD ON TOP OF LAYER 3	1.9988		
DRAINAGE COLLECTED FROM LAYER 8	0.0783	2050.945	0.16
PERC./LEAKAGE THROUGH LAYER 10	0.000057	1.538	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0092		

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DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.432	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-0.504	-13401.460	-1.08
SOIL WATER AT START OF YEAR	320.923	3960566.500	
SOIL WATER AT END OF YEAR	320.421	3952947.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.417	0.00

ANNUAL TOTALS FOR YEAR 27			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	55.71	1504571.120	100.00
RUNOFF	0.100	2890.318	0.18
EVAPOTRANSPIRATION	37.275	1006889.750	66.92
DRAINAGE COLLECTED FROM LAYER 2	18.7414	523260.312	34.84
PERC./LEAKAGE THROUGH LAYER 4	0.075302	2093.682	0.14
AVG. HEAD ON TOP OF LAYER 3	1.2257		
DRAINAGE COLLECTED FROM LAYER 8	0.0792	1977.938	0.13
PERC./LEAKAGE THROUGH LAYER 10	0.000015	1.493	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0088		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.423	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-1.478	-39947.463	-2.66
SOIL WATER AT START OF YEAR	320.421	3952947.000	
SOIL WATER AT END OF YEAR	318.942	3952999.500	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.217	0.00

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ANNUAL TOTALS FOR YEAR 28			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	40.76	1100923.870	100.00
RUNOFF	0.156	4202.913	0.38
EVAPOTRANSPIRATION	29.804	804926.312	73.12
DRAINAGE COLLECTED FROM LAYER 2	11.7328	305523.000	27.75
PERC./LEAKAGE THROUGH LAYER 4	0.072818	1960.620	0.18
AVG. HEAD ON TOP OF LAYER 3	1.2897		
DRAINAGE COLLECTED FROM LAYER 8	0.0780	2107.257	0.19
PERC./LEAKAGE THROUGH LAYER 10	0.000058	1.569	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0094		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.468	0.00
PERC./LEAKAGE THROUGH LAYER 13	0.000002	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-0.590	-15947.749	-1.45
SOIL WATER AT START OF YEAR	318.942	3912999.500	
SOIL WATER AT END OF YEAR	318.351	3897052.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.189	0.00

ANNUAL TOTALS FOR YEAR 29			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	48.38	1305606.370	100.00
RUNOFF	0.001	20.141	0.00
EVAPOTRANSPIRATION	31.817	859298.000	65.77
DRAINAGE COLLECTED FROM LAYER 2	11.0241	276051.750	20.78
PERC./LEAKAGE THROUGH LAYER 4	0.073474	1084.320	0.15

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AVG. HEAD ON TOP OF LAYER 3	1.5822		
DRAINAGE COLLECTED FROM LAYER 3	0.0741	2900.544	0.15
PERC./LEAKAGE THROUGH LAYER 10	0.000058	1.500	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.428	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	2.364	69236.859	1.30
SOIL WATER AT START OF YEAR	218.351	5097952.000	
SOIL WATER AT END OF YEAR	270.915	5966289.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.397	0.00

ANNUAL TOTALS FOR YEAR 30			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	42.79	1154936.320	100.00
RUNOFF	0.042	3128.536	0.10
EVAPOTRANSPIRATION	15.465	487740.873	39.51
DRAINAGE COLLECTED FROM LAYER 2	18.0946	487063.000	42.15
PERC./LEAKAGE THROUGH LAYER 4	0.074946	2024.075	0.18
AVG. HEAD ON TOP OF LAYER 3	2.1033		
DRAINAGE COLLECTED FROM LAYER 8	0.0753	2093.229	0.18
PERC./LEAKAGE THROUGH LAYER 10	0.000056	1.521	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0091		
DRAINAGE COLLECTED FROM LAYER 11	0.0001	1.432	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000003	0.084	0.00
AVG. HEAD ON TOP OF LAYER 12	0.0001		
CHANGE IN WATER STORAGE	-0.827	-37329.480	-1.95
SOIL WATER AT START OF YEAR	270.915	5966289.000	
SOIL WATER AT END OF YEAR	270.088	5943959.500	

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SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.441	0.00

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 30						
	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	4.49	3.61	4.54	4.00	3.24	5.06
STD. DEVIATIONS	1.47	0.10	4.78	3.38	2.50	3.61
PERC./LEAKAGE THROUGH LAYER 2	2.71	1.43	2.68	2.37	2.03	2.13
STD. DEVIATIONS	2.70	3.18	2.69	2.55	1.91	1.79
RUNOFF						
TOTALS	0.015	0.001	0.024	0.013	0.006	0.001
STD. DEVIATIONS	0.036	0.027	0.039	0.045	0.003	0.001
EVAPOTRANSPIRATION						
TOTALS	1.545	1.853	2.498	2.833	3.437	4.201
STD. DEVIATIONS	4.409	4.367	3.008	1.498	1.022	1.192
PERC./LEAKAGE THROUGH LAYER 4	0.250	0.326	0.607	0.912	1.230	1.324
STD. DEVIATIONS	1.478	1.217	0.955	0.503	0.202	0.247
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	3.0841	1.9603	2.2723	1.5016	1.1177	0.5679
STD. DEVIATIONS	0.6877	1.9087	1.4737	1.7763	1.1669	1.5761
PERC./LEAKAGE THROUGH LAYER 4	0.0068	0.0060	0.0066	0.0062	0.0062	0.0059
STD. DEVIATIONS	0.0061	0.0063	0.0066	0.0064	0.0060	0.0064
LATERAL DRAINAGE COLLECTED FROM LAYER 8						
TOTALS	0.0221	0.0413	0.0388	0.0191	0.0147	0.0024
STD. DEVIATIONS	0.0136	0.0107	0.0097	0.0094	0.0091	0.0091
PERC./LEAKAGE THROUGH LAYER 10	0.0738	0.2027	0.1655	0.1091	0.0340	0.0273
STD. DEVIATIONS	0.0190	0.0158	0.0131	0.0117	0.0100	0.0093
PERC./LEAKAGE THROUGH LAYER 11						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

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STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 11						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERC./LEAKAGE THROUGH LAYER 13						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	2.7020	2.8666	2.8838	2.1093	1.1951	0.8195
STD. DEVIATIONS	0.9129	1.6877	1.9412	2.7630	1.6271	2.1337
DAILY AVERAGE HEAD ON TOP OF LAYER 9						
AVERAGES	0.0313	0.0902	0.0532	0.0280	0.0209	0.0187
STD. DEVIATIONS	0.0166	0.0153	0.0143	0.0137	0.0134	0.0130
DAILY AVERAGE HEAD ON TOP OF LAYER 12						
AVERAGES	0.0005	0.0004	0.0004	0.0004	0.0003	0.0003
STD. DEVIATIONS	0.0001	0.0002	0.0001	0.0002	0.0002	0.0001

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 30			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.77 (9.081)	137115.6	100.00
RUNOFF	0.215 (0.3043)	5796.04	0.42
EVAPOTRANSPIRATION	22.054 (7.9314)	651675.94	81.135
LATERAL DRAINAGE COLLECTED FROM LAYER 2	18.4924 (6.9044)	499421.031	36.42337

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PERC./LEAKAGE THROUGH LAYER 4	0.07882 (0.00171)	2020.776	0.14738
AVERAGE HEAD ON TOP OF LAYER 3	2.044 (0.603)		
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.22827 (0.72388)	6165.010	0.44862
PERC./LEAKAGE THROUGH LAYER 10	0.00013 (0.00009)	3.458	0.00028
AVERAGE HEAD ON TOP OF LAYER 9	0.028 (0.090)		
LATERAL DRAINAGE COLLECTED FROM LAYER 11	0.00024 (0.00087)	6.557	0.00048
PERC./LEAKAGE THROUGH LAYER 13	0.00000 (0.00001)	0.123	0.00001
AVERAGE HEAD ON TOP OF LAYER 12	0.000 (0.001)		
CHANGE IN WATER STORAGE	-0.219 (1.6160)	-3912.77	-0.451

PEAK DAILY VALUES FOR YEARS 1 THROUGH 30		
	(INCHES)	(CU. FT.)
PRECIPITATION	6.02	162311.281
RUNOFF	0.949	25024.2383
DRAINAGE COLLECTED FROM LAYER 2	0.88143	23804.86330
PERC./LEAKAGE THROUGH LAYER 4	0.000356	0.01024
AVERAGE HEAD ON TOP OF LAYER 3	21.177	
MAXIMUM HEAD ON TOP OF LAYER 3	24.899	
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	35.7 FEET	
DRAINAGE COLLECTED FROM LAYER 8	0.07232	1947.80977
PERC./LEAKAGE THROUGH LAYER 10	0.000029	0.77906
AVERAGE HEAD ON TOP OF LAYER 9	3.111	
MAXIMUM HEAD ON TOP OF LAYER 9	4.052	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	18.2 FEET	
DRAINAGE COLLECTED FROM LAYER 11	0.00002	0.07455
PERC./LEAKAGE THROUGH LAYER 13	0.000000	0.00490
AVERAGE HEAD ON TOP OF LAYER 12	0.001	

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MAXIMUM HEAD ON TOP OF LAYER 12 0.022
 LOCATION OF MAXIMUM HEAD IN LAYER 11
 (DISTANCE FROM DRAIN) 0.0 FEET
 SNOW WATER 1.97 33212.68/F
 MAXIMUM VEG. SOIL WATER (VOL/VOL) 0.4038
 MINIMUM VEG. SOIL WATER (VOL/VOL) 0.0209

*** Maximal heads are computed using McEnroe's equations. ***

Reference: Maximum saturated depth over Landfill liner
 by Bruce M. McEnroe, University of Kansas
 ASCE Journal of Environmental Engineering
 Vol. 110, No. 2, March 1995, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 50

LAYER	(INCHES)	(VOL/VOL)
1	1.2883	0.2147
2	2.0262	0.3459
3	10.2480	0.4270
4	0.0000	0.0000
5	7.4256	0.3427
6	157.6980	0.2920
7	1.7123	0.3427
8	0.7633	0.0453
9	0.0000	0.0000
10	25.6200	0.4270
11	1.5770	0.1310
12	0.0000	0.0000
13	15.8720	0.4270
SNOW WATER	0.000	

CELLS B-7 AND B-8

INTERMEDIATE COVER

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#
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#
# HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
# HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
# DEVELOPED BY ENVIRONMENTAL LABORATORY
# USAC WATERWAYS EXPERIMENT STATION
# FOR USFPA RISK REDUCTION ENGINEERING LABORATORY
#
*****

PRECIPITATION DATA FILE: C:\DATA\04
TEMPERATURE DATA FILE: C:\DATA\07
SOLAR RADIATION DATA FILE: C:\DATA\03
EVAPOTRANSPIRATION DATA: C:\DATA\05
SOIL AND DESIGN DATA FILE: C:\DATA\010
OUTPUT DATA FILE: C:\OUT\OUT

TIME: 2018 DATE: 9/ 9/2013

-----
TITLE: 87-Without Cover
-----

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE
COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

LAYER 1
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 3
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2257 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

LAYER 2
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1B
THICKNESS = 1184.00 INCHES
Page 1
    
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POROSITY = 0.6710 VOL/VOL
FIELD CAPACITY = 0.2930 VOL/VOL
WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2969 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

LAYER 3
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 3
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2257 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

LAYER 4
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0430 VOL/VOL
WILTING POINT = 0.0280 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0815 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 45.0 FEET

LAYER 5
-----
TYPE 2 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

LAYER 6
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 60.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
Page 2
    
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WILTING POINT = 0.0360 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-08 CM/SEC

LAYER 7
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0430 VOL/VOL
WILTING POINT = 0.0280 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0815 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 45.0 FEET

LAYER 8
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

LAYER 9
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 16.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.2970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-08 CM/SEC

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
SOIL DATA BASE USING SOIL TEXTURE # 2 WITH A
FAIR STAND OF GRASS, A SURFACE SLOPE OF 0.3%
AND A SLOPE LENGTH OF 45. FEET.

SCS RUNOFF CURVE NUMBER = 65-10
Page 3
    
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FRACTION OF AREA ALLOWING RUNOFF = 100.0 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE = 3.870 ACRES
EVAPORATIVE ZONE DEPTH = 21.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 4.103 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 10.054 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE = 1.376 INCHES
INITIAL SNOW WATER = 0.000 INCHES
INITIAL WATER IN LAYER MATERIALS = 399.147 INCHES
TOTAL INITIAL WATER = 399.147 INCHES
TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

-----
AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100
-----
PRECIPITATION
TOTALS JAN/JUL FEB/AUG MAR/SEP APR/OCT MAY/NOV JUN/DEC
4.47 4.15 4.75 5.69 3.50 4.73
5.63 5.57 4.85 5.05 3.49 3.90

STD. DEVIATIONS 2.35 2.14 2.41 1.99 1.81 2.16
2.49 3.00 3.17 2.74 1.59 1.55

RUNOFF
TOTALS 0.023 0.012 0.019 0.021 0.009 0.004
0.023 0.012 0.023 0.030 0.002 0.009

STD. DEVIATIONS 0.064 0.051 0.059 0.041 0.046 0.016
0.24 0.062 0.207 0.224 0.021 0.027

EVAPOTRANSPIRATION
TOTALS 1.547 1.890 2.018 2.853 4.209 4.383
4.709 4.468 5.013 5.703 9.980 1.170

STD. DEVIATIONS 0.243 0.325 0.506 0.849 0.923 1.263
1.470 1.316 1.051 0.420 0.187 0.206

LATERAL DRAINAGE COLLECTED FROM LAYER 4
TOTALS 0.9957 1.0056 1.9189 1.7014 2.1423 2.0768
1.9189 1.2882 1.0072 0.9464 1.0017 1.0847

STD. DEVIATIONS 0.6545 0.3192 0.6886 0.6460 0.7659 0.8996
1.1564 1.1110 0.9193 0.7124 0.8733 0.9240

PERCOLATION/LEAKAGE THROUGH LAYER 8
TOTALS 0.0004 0.0004 0.0009 0.0006 0.0008 0.0007
0.0000 0.0003 0.0004 0.0004 0.0004 0.0004

STD. DEVIATIONS 0.0002 0.0003 0.0007 0.0002 0.0002 0.0003
0.0004 0.0004 0.0003 0.0003 0.0003 0.0003

LATERAL DRAINAGE COLLECTED FROM LAYER 7
TOTALS 0.0004 0.0004 0.0005 0.0005 0.0007 0.0007
0.0007 0.0005 0.0004 0.0004 0.0004 0.0004

STD. DEVIATIONS 0.0003 0.0002 0.0003 0.0002 0.0003 0.0003
0.0003 0.0003 0.0003 0.0003 0.0003 0.0003
Page 4
    
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PERCOLATION/LEAKAGE THROUGH LAYER 9						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 5						
AVERAGES	1.2589	1.4907	1.9239	2.2971	2.6787	2.6698
	2.2545	1.6085	1.3277	1.1988	1.1082	1.1426
STD. DEVIATIONS	0.8213	0.7702	0.8466	0.8179	0.9247	1.1225
	1.8059	1.3613	1.2168	1.0346	1.1223	1.1445

DAILY AVERAGE HEAD ON TOP OF LAYER 8						
AVERAGES	0.0003	0.0005	0.0007	0.0008	0.0009	0.0010
	0.0009	0.0007	0.0003	0.0003	0.0003	0.0003
STD. DEVIATIONS	0.0003	0.0003	0.0003	0.0003	0.0003	0.0003
	0.0004	0.0005	0.0004	0.0004	0.0003	0.0004

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	INCHES	CU. FEET	PERCENT
PRECIPITATION	50.13 (8.994)	704559.9	100.00
RUNOFF	0.249 (0.1055)	3505.00	0.497
EVAPOTRANSPIRATION	53.312 (3.6091)	467972.22	66.421
LATERAL DRAINAGE COLLECTED FROM LAYER 4	16.70163 (5.31745)	234676.156	33.30109
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.00014 (0.00178)	86.747	0.01224
AVERAGE HEAD ON TOP OF LAYER 5	1.781 (0.551)		
LATERAL DRAINAGE COLLECTED FROM LAYER 7	0.00013 (0.00178)	86.151	0.02123
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.00003 (0.00006)	0.166	0.00002
AVERAGE HEAD ON TOP OF LAYER 8	0.001 (0.000)		
CHANGE IN WATER STORAGE	-0.116 (5.1706)	-1629.64	-0.231

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	5.02	84429.078
RUNOFF	1.223	17178.6409
DRAINAGE COLLECTED FROM LAYER 4	0.17558	1764.33433
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000041	0.57056
AVERAGE HEAD ON TOP OF LAYER 5	4.598	
MAXIMUM HEAD ON TOP OF LAYER 5	5.584	
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	19.9 FEET	
DRAINAGE COLLECTED FROM LAYER 7	0.00004	0.54587
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000003	0.00052
AVERAGE HEAD ON TOP OF LAYER 8	0.002	
MAXIMUM HEAD ON TOP OF LAYER 8	0.003	
LOCATION OF MAXIMUM HEAD IN LAYER 7 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	4.02	56877.0273
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.3831
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0580

*** Maximum heads are computed using McEnroe's equation. ***
 Reference: Maximum Saturated Depth over Landfill Liner
 by Bruce M. McEnroe, University of Kansas
 ASCE Journal of Environmental Engineering
 Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100

LAYER	(INCHES)	(VOL/VOL)
1	3.3719	0.1405
2	339.8880	0.2920
5	3.4588	0.2012
4	0.7161	0.0597
5	0.0000	0.0000
6	25.0200	0.4270

7	0.5461	0.0450
8	0.0000	0.0000
9	11.3776	0.4270
SNOW WATER	0.000	

FINAL COVER

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#
#*****
#
# HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
# HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
# DEVELOPED BY ENVIRONMENTAL LABORATORY
# USAC WATERWAYS EXPERIMENT STATION
# FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
#*****
#
PRECIPITATION DATA FILE: C:\DATA\04
TEMPERATURE DATA FILE: C:\DATA\07
SOLAR RADIATION DATA FILE: C:\DATA\03.D33
EVAPORATION DATA FILE: C:\DATA\01
SOIL AND DESIGN DATA FILE: C:\DATA\010
OUTPUT DATA FILE: C:\OUT\OUT

TIME: 18:42 DATE: 9/ 9/2013

-----
TITLE: 87-4th cover
-----

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER BECC
      COMPUTED AS NEARLY STEADY-STATE VALUE BY THE PROGRAM.

-----
LAYER 1
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 5
THICKNESS = 9.00 INCHES
POROSITY = 0.4570 VOL/VOL
FIELD CAPACITY = 0.1380 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1691 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
      FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

-----
LAYER 2
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 18.00 INCHES
Page 1

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WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2970 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

-----
LAYER 7
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 5
THICKNESS = 17.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1310 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1330 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

-----
LAYER 8
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0450 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0453 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.00000000000E-02 CM/SEC
SLOPE = 3.00 PERCENT
DRAINAGE LENGTH = 45.0 FEET

-----
LAYER 9
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-12 CM/SEC
FML PENHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

-----
LAYER 10
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 36
THICKNESS = 50.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.3970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
Page 3

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POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2607 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.00000000000E-02 CM/SEC
SLOPE = 3.00 PERCENT
DRAINAGE LENGTH = 45.0 FEET

-----
LAYER 3
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-12 CM/SEC
FML PENHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

-----
LAYER 4
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 36
THICKNESS = 24.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.3970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-06 CM/SEC

-----
LAYER 5
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 5
THICKNESS = 24.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.1320 VOL/VOL
WILTING POINT = 0.0380 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1320 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

-----
LAYER 6
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 18
THICKNESS = 116.00 INCHES
POROSITY = 0.0710 VOL/VOL
FIELD CAPACITY = 0.2970 VOL/VOL
Page 2

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EFFECTIVE SAT. HYD. COND. = 0.10000000000E-08 CM/SEC

-----
LAYER 11
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.0450 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0453 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.00000000000E-02 CM/SEC
SLOPE = 3.00 PERCENT
DRAINAGE LENGTH = 45.0 FEET

-----
LAYER 12
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-12 CM/SEC
FML PENHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 4 - POOR

-----
LAYER 13
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 36
THICKNESS = 36.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.3970 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-08 CM/SEC

-----
UNSATURATED ZONE AND EVAPORATIVE ZONE DATA
-----
NOTE: CCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
      SOIL DATA BASE USING SOIL TEXTURE # 5 WITH A
      FAIR STAND OF GRASS, A SURFACE SLOPE OF 2.5%
      AND A SLOPE LENGTH OF 45 FEET.

CCS RUNOFF CURVE NUMBER = 58.30
FRACTION OF AREA ALLOWING RUNOFF = 100.0 PERCENT
AREA PROHIBITED ON HORIZONTAL PLANE = 3.870 ACRES
Page 4

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EVAPORATIVE ZONE DEPTH	=	22.0	INCHES
INITIAL WATER IN EVAPORATIVE ZONE	=	4.861	INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE	=	0.414	INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	0.636	INCHES
INITIAL SNOW WATER	=	0.000	INCHES
INITIAL WATER IN LAYER MATERIALS	=	462.624	INCHES
TOTAL INITIAL WATER	=	462.624	INCHES
TOTAL SUBSURFACE INFLOW	=	0.00	INCHES/YEAR

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100							
	JAN/FEB	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC	
PRECIPITATION							
TOTALS	4.47	4.15	4.75	3.89	3.30	4.71	
STD. DEVIATIONS	2.55	2.14	2.43	1.99	1.92	2.16	
	2.40	1.00	1.17	1.24	1.15	1.57	
RUNOFF							
TOTALS	0.016	0.012	0.017	0.009	0.007	0.009	
STD. DEVIATIONS	0.026	0.019	0.009	0.026	0.001	0.003	
	0.109	0.042	0.051	0.031	0.042	0.013	
	0.109	0.053	0.185	0.134	0.058	0.013	
EVAPOTRANSPIRATION							
TOTALS	1.372	1.008	1.496	2.857	3.070	4.001	
STD. DEVIATIONS	4.455	1.207	1.970	1.509	1.021	1.203	
	0.294	0.377	0.677	0.847	1.116	1.484	
	1.416	1.197	1.050	0.459	0.121	0.123	
LATERAL DRAINAGE COLLECTED FROM LAYER 3							
TOTALS	2.8331	2.4808	2.3375	1.4425	1.0710	0.1629	
STD. DEVIATIONS	0.8581	1.0980	1.3099	1.0018	1.1559	1.3019	
	1.7378	1.8412	1.7520	0.9735	0.9119	0.1302	
	1.0526	1.1540	1.4917	1.7571	1.2488	1.1703	
PERCOLATION/LEAKAGE THROUGH LAYER 4							
TOTALS	0.0008	0.0008	0.0008	0.0005	0.0004	0.0002	
STD. DEVIATIONS	0.0003	0.0004	0.0004	0.0005	0.0004	0.0005	
	0.0004	0.0005	0.0004	0.0003	0.0003	0.0002	
	0.0003	0.0003	0.0004	0.0005	0.0004	0.0004	
LATERAL DRAINAGE COLLECTED FROM LAYER 8							
TOTALS	0.0007	0.0008	0.0008	0.0006	0.0003	0.0003	
STD. DEVIATIONS	0.0003	0.0003	0.0004	0.0004	0.0004	0.0005	
	0.0003	0.0004	0.0004	0.0003	0.0003	0.0002	
	0.0002	0.0003	0.0002	0.0004	0.0004	0.0003	
PERCOLATION/LEAKAGE THROUGH LAYER 10							

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TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 11						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 13						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	1.0012	1.0831	1.6827	1.8068	1.7587	0.7277
STD. DEVIATIONS	1.0159	1.3077	1.5829	1.8117	1.4239	1.3194
	1.2154	1.9168	1.8726	1.1274	1.0333	0.6407
	1.1413	1.2468	1.1264	1.7643	1.4134	1.1270
DAILY AVERAGE HEAD ON TOP OF LAYER 8						
AVERAGES	0.0009	0.0011	0.0020	0.0008	0.0006	0.0004
STD. DEVIATIONS	0.0003	0.0004	0.0005	0.0007	0.0006	0.0006
	0.0004	0.0003	0.0001	0.0004	0.0003	0.0002
	0.0003	0.0004	0.0004	0.0003	0.0003	0.0004
DAILY AVERAGE HEAD ON TOP OF LAYER 12						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.13 (8.594)	708559.9	100.00
RUNOFF	0.217 (0.2756)	3046.09	0.432
EVAPOTRANSPIRATION	17.878 (1.7992)	447819.71	68.560
LATERAL DRAINAGE COLLECTED	18.0907 (1.94448)	754175.187	96.0754

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FROM LAYER 1		
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00012 (0.00109)	85.811 0.01218
AVERAGE HEAD ON TOP OF LAYER 3	1.796 (0.515)	
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.00012 (0.00167)	85.788 0.01218
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00002 (0.00009)	0.101 0.00004
AVERAGE HEAD ON TOP OF LAYER 9	0.001 (0.000)	
LATERAL DRAINAGE COLLECTED FROM LAYER 11	0.00000 (0.00000)	0.070 0.00001
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.00000 (0.00000)	0.035 0.00000
AVERAGE HEAD ON TOP OF LAYER 12	0.000 (0.000)	
CHANGE IN WATER STORAGE	-0.040 (1.2024)	-564.71 -0.080

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	5.01	84429.078
RUNOFF	1.117	1566.1709
DRAINAGE COLLECTED FROM LAYER 2	0.96273	13524.4720
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000158	2.21531
AVERAGE HEAD ON TOP OF LAYER 3	19.368	
MAXIMUM HEAD ON TOP OF LAYER 3	23.044	
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	37.3 FEET	
DRAINAGE COLLECTED FROM LAYER 8	0.00008	1.18887
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000	0.00099
AVERAGE HEAD ON TOP OF LAYER 9	0.003	
MAXIMUM HEAD ON TOP OF LAYER 9	0.007	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 11	0.00000	0.00064
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.000000	0.00010
AVERAGE HEAD ON TOP OF LAYER 12	0.000	

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MAXIMUM HEAD ON TOP OF LAYER 12	0.000
LOCATION OF MAXIMUM HEAD IN LAYER 11 (DISTANCE FROM DRAIN)	0.0 FEET
SNOW WATER	4.02 36477.0273
MAXIMUM VEG. SOIL WATER (VOL/VOL)	0.3927
MINIMUM VEG. SOIL WATER (VOL/VOL)	0.0299

*** Maximum heads are computed using McInroe's equations. ***
 Reference: Maximum Saturated Depth over Landfill Liner by Bruce M. McInroe, University of Kansas Agric Journal of Environmental Engineering Vol. 119, No. 2, March 1991, pp. 362-370.

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)
1	0.3506	0.0584
2	1.2294	0.0739
3	0.0000	0.0000
4	10.2480	0.1270
5	3.1440	0.1310
6	319.6800	0.2920
7	1.1270	0.1330
8	0.9401	0.9450
9	0.0000	0.0000
10	25.6200	0.4270
11	0.1400	0.0450
12	0.0000	0.0000
13	13.3720	0.4270
SNOW WATER	0.000	

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LANDFILL T

INTERMEDIATE COVER

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AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FEET)	PERCENT
PRECIPITATION	38.14 (0.040)	291475.6	100.00
RUNOFF	0.028 (0.0050)	143.80	0.049
EVAPOTRANSPIRATION	4.0377 (4.0899)	208194.06	69.445
LATERAL DRAINAGE COLLECTED FROM LAYER 2	17.73285 (6.31681)	90316.336	30.49941
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00112 (0.00038)	5.690	0.00191
AVERAGE HEAD ON TOP OF LAYER 3	1.706 (0.596)		
LATERAL DRAINAGE COLLECTED FROM LAYER 3	0.00113 (0.00017)	5.471	0.00183
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000 (0.00000)	0.004	0.00000
AVERAGE HEAD ON TOP OF LAYER 6	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.003 (5.6122)	13.73	0.005

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PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	6.14	81203.479
RUNOFF	0.436	2216.3388
DRAINAGE COLLECTED FROM LAYER 2	0.14734	810.44690
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000008	0.04794
AVERAGE HEAD ON TOP OF LAYER 3	3.591	
MAXIMUM HEAD ON TOP OF LAYER 3	6.861	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	20.9 FEET	
DRAINAGE COLLECTED FROM LAYER 3	0.00001	0.01672
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000	0.00004
AVERAGE HEAD ON TOP OF LAYER 6	0.000	
MAXIMUM HEAD ON TOP OF LAYER 6	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	

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Page 5

SHOW WATER 5.7% 29026.4746

MAXIMUM VEG. SOIL WATER (VOL/VOL) 0.4972

MINIMUM VEG. SOIL WATER (VOL/VOL) 0.0770

*** Maximum heads are computed using McInnes's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner by Bruce W. McInnes, University of Kansas, ASCE Journal of Environmental Engineering, Vol. 119, No. 2, March 1993, pp. 262-270.

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FINAL WATER STORAGE AT END OF YEAR 100

LAYER	(INCHES)	(VOL/VOL)
1	231.1384	0.2990
2	1.0752	0.0899
3	0.0000	0.0000
4	13.3720	0.4270
5	0.5400	0.0450
6	0.0000	0.0000
7	15.3720	0.4270
SHOW WATER	0.000	

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Page 6

FINAL COVER

STD. DEVIATIONS	0.029 0.001	0.109 0.010	0.015 0.004	0.013 0.004	0.030 0.009	0.002 0.001
EVAPOTRANSPIRATION						
TOTALS	0.310 1.151	0.700 1.078	0.761 0.816	0.908 0.172	0.777 0.131	0.887 0.462
STD. DEVIATIONS	0.457 0.894	0.577 0.449	0.716 0.516	0.479 0.709	0.387 0.789	0.394 0.301
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	4.2640 5.3104	4.5157 5.0445	3.7356 4.7234	3.4848 2.4210	3.9391 3.3339	3.0596 5.0260
STD. DEVIATIONS	2.2608 1.9462	2.2457 2.2890	2.1840 2.3752	2.3500 1.6228	2.1759 2.0249	1.7431 2.5229
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 6						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 8						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 9						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 11						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
AVERAGES OR MONTHLY AVERAGED DAILY HEADS (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 3						

AVERAGES	0.0008 0.0010	0.0009 0.0009	0.0007 0.0009	0.0006 0.0004	0.0007 0.0006	0.0007 0.0009
STD. DEVIATIONS	0.0004 0.0003	0.0004 0.0004	0.0004 0.0004	0.0004 0.0003	0.0004 0.0004	0.0003 0.0004
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 10						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES		CU. FEET		PERCENT	
PRECIPITATION	19.14	(0.060)	21105462.0		100.00	
RUNOFF	0.078	(0.1510)	28207.20		0.134	
EVAPOTRANSPIRATION	8.390	(2.3747)	1045605.73		24.410	
LATERAL DRAINAGE COLLECTED FROM LAYER 2	49.67404	(7.70858)	18031670.000		85.4360	
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000	(0.00000)		1.246	0.00001	
AVERAGE HEAD ON TOP OF LAYER 5	0.001	(0.000)				
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00000	(0.00000)		0.246	0.00000	
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00000	(0.00000)		0.900	0.00000	
AVERAGE HEAD ON TOP OF LAYER 9	0.000	(0.000)				
LATERAL DRAINAGE COLLECTED FROM LAYER 9	0.00000	(0.00000)		0.000	0.00000	
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.00000	(0.00000)		0.900	0.00000	
AVERAGE HEAD ON TOP OF LAYER 10	0.000	(0.000)				
CHANGE IN WATER STORAGE	0.000	(0.8300)		-87.91	0.000	

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	6.14	2228820.000
RUNOFF	0.762	276776.1560
DRAINAGE COLLECTED FROM LAYER 2	3.92077	1423239.62000
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000000	0.00281
AVERAGE HEAD ON TOP OF LAYER 5	0.021	
MAXIMUM HEAD ON TOP OF LAYER 5	0.043	
LOCATION OF MAXIMUM HEAD IN LAYER 5 (DISTANCE FROM DRAIN)	0.2 FEET	
DRAINAGE COLLECTED FROM LAYER 6	0.00000	0.00308
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.000000	0.00247
AVERAGE HEAD ON TOP OF LAYER 7	0.000	
MAXIMUM HEAD ON TOP OF LAYER 7	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 9	0.00000	0.00004
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.000000	0.00247
AVERAGE HEAD ON TOP OF LAYER 10	0.000	
MAXIMUM HEAD ON TOP OF LAYER 10	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN)	0.0 FEET	
SHOW WATER	5.71	2073319.6200
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.3220
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0380
*** Maximum heads are computed using McCorrie's equations. ***		
Reference: Maximum saturated depth over Landfill Liner By Bruce M. McCorrie, University of Kansas ASCE Journal of Environmental Engineering vol. 119, No. 2, March 1993, pp. 262-270.		
FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)

1	3.2682	0.9415
2	0.0021	0.0006
3	0.0000	0.0000
4	10.2480	0.4270
5	241.2800	0.7920
6	0.5400	0.0450
7	0.0000	0.0000
8	13.3720	0.4270
9	0.5400	0.0450
10	0.0000	0.0000
11	13.3720	0.4270
SHOW WATER	0.000	

LANDFILL J

INTERMEDIATE COVER

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**
HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
DEVELOPED BY ENVIRONMENTAL LABORATORY
USAE WATERWAYS EXPERIMENT STATION
FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
**
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*****

PRECIPITATION DATA FILE: C:\JNC\DATA1.04
TEMPERATURE DATA FILE: C:\JNC\DATA1.07
SOLAR RADIATION DATA FILE: C:\JNC\DATA1.03A
EVAPORATION DATA FILE: C:\JNC\DATA1.03B
SOIL AND DESIGN DATA FILE: C:\JNC\DATA10.010
OUTPUT DATA FILE: C:\JNC\OUT.OUT

TIME: 15:18 DATE: 4/26/2016

-----
TITLE: 3) Without cover
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NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER DEPTH
COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

-----
LAYER 1
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1B
THICKNESS = 180.00 INCHES
POROSITY = 0.6710 VOL/VOL
FIELD CAPACITY = 0.2929 VOL/VOL
WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2873 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.1000000000E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

-----
LAYER 2
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
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Page 1

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POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0679 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC

-----
LAYER 3
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0457 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 10.0 FEET

-----
LAYER 4
-----
TYPE 2 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.09 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.199999999999E-12 CM/SEC
PML PINHOLE DENSITY = 2.00 HOLES/ACRE
PML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
PML PLACEMENT QUALITY = 3 - GOOD

-----
LAYER 5
-----
TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 1C
THICKNESS = 18.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.2670 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.100000000000E-08 CM/SEC

-----
LAYER 6
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
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Page 2

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WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0410 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 10.0 FEET

-----
LAYER 7
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.09 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.199999999999E-12 CM/SEC
PML PINHOLE DENSITY = 2.00 HOLES/ACRE
PML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
PML PLACEMENT QUALITY = 3 - GOOD

-----
LAYER 8
-----
TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 1C
THICKNESS = 18.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.2670 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.100000000000E-08 CM/SEC

-----
GENERAL DESIGN AND EVAPORATIVE ZONE DATA
-----
NOTE: 80% RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
SOIL DATA BASE USING SOIL TEXTURE #18 WITH A
FAIR STAND OF GRASS, A SURFACE SLOPE OF 3%
AND A SLOPE LENGTH OF 50 FEET.

80% RUNOFF CURVE NUMBER = 61.90
FRACTION OF AREA ALLOWING RUNOFF = 100.0 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE = 100.000 ACRES
EVAPORATIVE ZONE DEPTH = 32.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 2.158 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 21.427 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE = 3.464 INCHES
INITIAL SNOW WATER = 0.000 INCHES
INITIAL WATER IN LAYER MATERIALS = 438.153 INCHES
TOTAL INITIAL WATER = 438.153 INCHES
TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

-----
AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100
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Page 3

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	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.10	1.11	0.93	0.72	0.24	0.08
STD. DEVIATIONS	0.01	0.01	0.14	0.27	0.70	0.65
RUNOFF						
TOTALS	0.000	0.000	0.000	0.000	0.000	0.000
STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000
EVAPORATION						
TOTALS	0.021	0.032	1.416	1.401	0.404	0.077
STD. DEVIATIONS	0.218	0.219	0.471	0.944	0.446	0.106
LATERAL DRAINAGE COLLECTED FROM LAYER 3						
TOTALS	0.003	0.003	0.003	0.003	0.000	0.000
STD. DEVIATIONS	0.003	0.003	0.003	0.003	0.000	0.000
PERCOLATION/LEAKAGE THROUGH LAYER 1						
TOTALS	0.000	0.000	0.000	0.000	0.000	0.000
STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000
LATERAL DRAINAGE COLLECTED FROM LAYER 6						
TOTALS	0.000	0.000	0.000	0.000	0.000	0.000
STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000
PERCOLATION/LEAKAGE THROUGH LAYER 8						
TOTALS	0.000	0.000	0.000	0.000	0.000	0.000
STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000

AVERAGES OF MONTHLY AVERAGES DAILY HEADS (INCHES)

(DATE) AVERAGE HEAD ON TOP OF LAYER 4

Page 4

AVERAGES	0.0004	0.0004	0.0004	0.0004	0.0005	0.0004
STD. DEVIATIONS	0.0017	0.0015	0.0014	0.0013	0.0013	0.0011
DAILY AVERAGE HEAD ON TOP OF LAYER 7	0.0011	0.0010	0.0010	0.0009	0.0009	0.0008
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	6.00 (1.691)	2176875.0	100.000
RUNOFF	0.000 (0.0000)	0.00	0.000
EVAPOTRANSPIRATION	3.991 (1.5815)	2174883.00	99.999
LATERAL DRAINAGE COLLECTED FROM LAYER 3	0.00319 (0.00939)	1137.340	0.05317
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.00000 (0.00000)	0.001	0.00005
AVERAGE HEAD ON TOP OF LAYER 4	0.000 (0.001)		
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00000 (0.00000)	0.178	0.00001
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00000 (0.00000)	0.448	0.00002
AVERAGE HEAD ON TOP OF LAYER 7	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.002 (0.7491)	1681.79	0.047

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	1.62	588060.000
RUNOFF	0.000	0.0000
DRAINAGE COLLECTED FROM LAYER 3	0.00016	571.50139
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.000000	0.01866
AVERAGE HEAD ON TOP OF LAYER 4	0.010	

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MAXIMUM HEAD ON TOP OF LAYER 4	0.032
LOCATION OF MAXIMUM HEAD IN LAYER 7 (DISTANCE FROM DRAIN)	0.6 FEET
DRAINAGE COLLECTED FROM LAYER 6	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.000000
AVERAGE HEAD ON TOP OF LAYER 7	0.000
MAXIMUM HEAD ON TOP OF LAYER 7	0.001
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET
SNOW WATER	0.73
MAXIMUM VEG. SOIL WATER (VOL/VOL)	0.2355
MINIMUM VEG. SOIL WATER (VOL/VOL)	0.0776

*** Maximum heads are computed using McCorde's equations. ***

REFERENCE: Maximum saturated depth over landfill liner by Bruce H. McCorde, University of Kansas AGES Journal of Environmental Engineering Vol. 119, No. 3, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)
1	414.1881	0.2876
2	0.3312	0.0439
3	0.5403	0.0450
4	0.0000	0.0000
5	7.6860	0.4270
6	0.1400	0.0450
7	0.0000	0.0000
8	15.3720	0.4270
SNOW WATER	0.000	

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LANDFILL R

CELL R-1

INTERMEDIATE COVER

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AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FEET)	PERCENT
PRECIPITATION	6.00 (1.831)	137243.1	100.00
RUNOFF	0.000 (0.0000)	0.00	0.000
EVAPOTRANSPIRATION	5.478 (1.2370)	123271.71	91.344
LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.33375 (0.59714)	11748.908	8.56690
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00002 (0.00001)	0.469	0.00034
AVERAGE HEAD ON TOP OF LAYER 3	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 3	0.00000 (0.00001)	0.452	0.00033
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000 (0.00000)	0.087	0.00001
AVERAGE HEAD ON TOP OF LAYER 6	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.005 (0.7180)	121.99	0.089

=====

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	1.62	37047.781
RUNOFF	0.000	0.0000
DRAINAGE COLLECTED FROM LAYER 2	0.12675	2896.70117
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000001	0.02500
AVERAGE HEAD ON TOP OF LAYER 3	0.001	
MAXIMUM HEAD ON TOP OF LAYER 3	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 3	0.00000	0.01540
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000	0.00001
AVERAGE HEAD ON TOP OF LAYER 6	0.000	
MAXIMUM HEAD ON TOP OF LAYER 6	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	

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Page 5

SHOW WATER 6.75 16786.6875

MAXIMUM VEG. SOIL WATER (VOL/VOL) 0.1974

MINIMUM VEG. SOIL WATER (VOL/VOL) 0.0770

*** Maximum heads are computed using Meinrose's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner by Bruce W. Meinrose, University of Kansas, ASCE Journal of Environmental Engineering, Vol. 119, No. 2, March 1993, pp. 262-270.

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FINAL WATER STORAGE AT END OF YEAR 100

LAYER	(INCHES)	(VOL/VOL)
1	235.4840	0.2844
2	0.0020	0.0100
3	0.0000	0.0000
4	13.3720	0.4270
5	0.0030	0.0100
6	0.0000	0.0000
7	15.3720	0.4270
SHOW WATER	0.000	

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Page 6

FINAL COVER

STD. DEVIATIONS	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000
EVAPOTRANSPIRATION						
TOTALS	0.073 0.033	0.099 0.072	0.108 0.071	0.142 0.027	0.088 0.058	0.026 0.045
STD. DEVIATIONS	0.052 0.016	0.071 0.061	0.069 0.051	0.104 0.044	0.087 0.019	0.036 0.035
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	1.048 0.0031	1.040 0.0030	0.827 0.0969	0.5814 0.2282	0.1776 0.6356	0.0548 0.0046
STD. DEVIATIONS	0.5911 0.0282	0.7578 0.1174	0.3441 0.1656	0.4935 0.3951	0.2224 0.5826	0.1781 0.4019
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 6						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 8						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 9						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 11						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
AVERAGES (OR MONTHLY AVERAGED DAILY HEADS) (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 3						

AVERAGES	0.0044 0.0000	0.0010 0.0001	0.0036 0.0004	0.0026 0.0010	0.0008 0.0029	0.0002 0.0026
STD. DEVIATIONS	0.0026 0.0001	0.0018 0.0006	0.0024 0.0007	0.0021 0.0017	0.0010 0.0026	0.0008 0.0017
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 10						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES		CU. FEET		PERCENT	
PRECIPITATION	6.00	(1.635)	137148.1	100.00		
RUNOFF	0.000	(0.0000)	0.00	0.000		
EVAPOTRANSPIRATION	8.796	(0.2081)	16147.19	11.774		
LATERAL DRAINAGE COLLECTED FROM LAYER 2	5.29085	(1.48820)	120991.896	89.23600		
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000	(0.00000)	0.000	0.00000		
AVERAGE HEAD ON TOP OF LAYER 5	0.002	(0.001)				
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00000	(0.00000)	0.000	0.00000		
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00000	(0.00000)	0.000	0.00000		
AVERAGE HEAD ON TOP OF LAYER 7	0.000	(0.000)				
LATERAL DRAINAGE COLLECTED FROM LAYER 9	0.00000	(0.00000)	0.000	0.00000		
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.00000	(0.00000)	0.000	0.00000		
AVERAGE HEAD ON TOP OF LAYER 10	0.000	(0.000)				
CHANGE IN WATER STORAGE	0.000	(0.0047)	0.00	0.000		

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	1.62	37047.781
RUNOFF	0.000	0.0000
DRAINAGE COLLECTED FROM LAYER 2	1.27306	29113.53128
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 5	0.172	
MAXIMUM HEAD ON TOP OF LAYER 3	0.777	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	2.3 FEET	
DRAINAGE COLLECTED FROM LAYER 6	0.00000	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 7	0.000	
MAXIMUM HEAD ON TOP OF LAYER 7	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 9	0.00000	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 10	0.000	
MAXIMUM HEAD ON TOP OF LAYER 10	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	0.71	16786.6878
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.1118
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0655
*** Maximum heads are computed using McCorr's equations. ***		
Reference: Maximum saturated depth over Landfill Liner By Bruce M. McCorr, University of Kansas ASCE Journal of Environmental Engineering vol. 119, No. 2, March 1993, pp. 262-270.		

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)

1	1.6488	0.0687
2	0.0250	0.1250
3	0.0000	0.0000
4	10.2480	0.4270
5	241.7760	0.7920
6	0.0040	0.0100
7	0.0000	0.0000
8	13.3720	0.4270
9	0.0070	0.0100
10	0.0000	0.0000
11	13.3720	0.4270
SNOW WATER	0.000	

CELLS R-2 TO R-5

INTERMEDIATE COVER


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AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100
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	INCHES	CU. FEET	PERCENT
PRECIPITATION	6.00 (1.631)	35137.4	100.00
RUNOFF	0.000 (0.0000)	0.00	0.000
EVAPOTRANSPIRATION	5.478 (1.2370)	48179.31	91.344
LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.31377 (0.59715)	43505.492	8.56723
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000 (0.00000)	0.000	0.00000
AVERAGE HEAD ON TOP OF LAYER 3	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 5	0.00000 (0.00000)	0.000	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000 (0.00000)	0.001	0.00000
AVERAGE HEAD ON TOP OF LAYER 6	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.005 (0.7180)	472.46	0.089

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PEAK DAILY VALUES FOR YEARS 1 THROUGH 100
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	(INCHES)	(CU. FT.)
PRECIPITATION	1.62	143486.641
RUNOFF	0.000	0.0000
DRAINAGE COLLECTED FROM LAYER 2	0.12744	11287.60210
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000000	0.00078
AVERAGE HEAD ON TOP OF LAYER 3	0.001	
MAXIMUM HEAD ON TOP OF LAYER 3	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	0.8 FEET	
DRAINAGE COLLECTED FROM LAYER 5	0.00000	0.00077
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000	0.00018
AVERAGE HEAD ON TOP OF LAYER 6	0.000	
MAXIMUM HEAD ON TOP OF LAYER 6	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	

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SHOW WATER 6.75 43021.1004
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MAXIMUM VEG. SOIL WATER (VOL/VOL)	0.1974
MINIMUM VEG. SOIL WATER (VOL/VOL)	0.0770

*** Maximum heads are computed using McInroe's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner by Bruce W. McInroe, University of Kansas, ASCE Journal of Environmental Engineering, Vol. 119, No. 2, March 1993, pp. 262-270.

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FINAL WATER STORAGE AT END OF YEAR 100
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LAYER	(INCHES)	(VOL/VOL)
1	863.1320	0.2871
2	0.0020	0.0100
3	0.0000	0.0000
4	13.3720	0.4270
5	0.0030	0.0100
6	0.0000	0.0000
7	15.3720	0.4270
SHOW WATER	0.000	

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FINAL COVER

STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000
EVAPOTRANSPIRATION						
TOTALS	0.073	0.099	0.108	0.142	0.088	0.026
STD. DEVIATIONS	0.003	0.002	0.013	0.027	0.058	0.045
STD. DEVIATIONS	0.052	0.071	0.069	0.104	0.087	0.036
STD. DEVIATIONS	0.016	0.003	0.015	0.044	0.019	0.035
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	1.0448	1.0400	0.8227	0.5814	0.1776	0.0548
STD. DEVIATIONS	0.0031	0.0030	0.0069	0.2282	0.6356	0.0046
STD. DEVIATIONS	0.5931	0.7578	0.3441	0.4935	0.2224	0.1781
STD. DEVIATIONS	0.0282	0.1174	0.1656	0.3951	0.5820	0.4019
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 6						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 8						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 9						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 11						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGES OR MONTHLY AVERAGED DAILY HEADS (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 3						

AVERAGES	0.0044	0.0010	0.0036	0.0026	0.0008	0.0002
STD. DEVIATIONS	0.0000	0.0001	0.0004	0.0010	0.0009	0.0002
STD. DEVIATIONS	0.0026	0.0018	0.0024	0.0021	0.0010	0.0008
STD. DEVIATIONS	0.0001	0.0006	0.0007	0.0017	0.0016	0.0017
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 10						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES	CU. FEET	PERCENT			
PRECIPITATION	6.00 (1.635)	51117.4	100.00			
RUNOFF	0.000 (0.0000)	0.00	0.000			
EVAPOTRANSPIRATION	8.706 (0.2081)	62138.22	11.774			
LATERAL DRAINAGE COLLECTED FROM LAYER 2	5.29085 (1.49820)	468038.969	88.2600			
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000 (0.00000)	0.000	0.00000			
AVERAGE HEAD ON TOP OF LAYER 5	0.002 (0.001)					
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00000 (0.00000)	0.000	0.00000			
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00000 (0.00000)	0.000	0.00000			
AVERAGE HEAD ON TOP OF LAYER 7	0.000 (0.000)					
LATERAL DRAINAGE COLLECTED FROM LAYER 9	0.00000 (0.00000)	0.000	0.00000			
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.00000 (0.00000)	0.000	0.00000			
AVERAGE HEAD ON TOP OF LAYER 10	0.000 (0.000)					
CHANGE IN WATER STORAGE	0.000 (0.0047)	0.00	0.000			

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	1.62	143486.641
RUNOFF	0.000	0.00000
DRAINAGE COLLECTED FROM LAYER 2	1.27306	112757.17000
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 5	0.172	
MAXIMUM HEAD ON TOP OF LAYER 5	0.377	
LOCATION OF MAXIMUM HEAD IN LAYER 5 (DISTANCE FROM DRAIN)	2.3 FEET	
DRAINAGE COLLECTED FROM LAYER 6	0.00000	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 7	0.000	
MAXIMUM HEAD ON TOP OF LAYER 7	0.002	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 9	0.00000	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 10	0.000	
MAXIMUM HEAD ON TOP OF LAYER 10	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	0.73	65015.1094
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.1118
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0655
*** Maximum heads are computed using McCorrie's equations. ***		
Reference: Maximum saturated depth over Landfill Liner By Bruce M. McCorrie, University of Kansas ASCE Journal of Environmental Engineering vol. 119, No. 2, March 1993, pp. 262-270.		
FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)

1	1.6488	0.0687
2	0.0250	0.1250
3	0.0000	0.0000
4	10.2480	0.4270
5	371.4240	0.7920
6	0.0040	0.0100
7	0.0000	0.0000
8	13.3720	0.4270
9	0.0070	0.0100
10	0.0000	0.0000
11	13.3720	0.4270
SNOW WATER	0.000	

30-YEAR DURATION

	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.70	823719.687	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	8.096	727039.687	87.05
DRAINAGE COLLECTED FROM LAYER 2	0.3096	27436.133	3.33
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.0017	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.017	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.001	0.00
AVG. HEAD ON TOP OF LAYER 6	0.0000		
CHANGE IN WATER STORAGE	0.895	79262.859	9.62
SOIL WATER AT START OF YEAR	395,292	3501800.000	
SOIL WATER AT END OF YEAR	396,187	3502060.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.393	0.00

ANNUAL TOTALS FOR YEAR 4

	INCHES	CU. FEET	PERCENT
PRECIPITATION	8.59	406545.531	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	5.151	450219.844	112.22
DRAINAGE COLLECTED FROM LAYER 2	0.2495	22095.549	5.44
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.028	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.028	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.001	0.00
AVG. HEAD ON TOP OF LAYER 6	0.0000		
CHANGE IN WATER STORAGE	-0.810	-71770.133	-17.85
SOIL WATER AT START OF YEAR	396,182	3502060.000	
SOIL WATER AT END OF YEAR	395,372	3501920.000	

SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.184	0.00

ANNUAL TOTALS FOR YEAR 5

	INCHES	CU. FEET	PERCENT
PRECIPITATION	7.83	693539.750	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	7.536	667437.750	96.24
DRAINAGE COLLECTED FROM LAYER 2	0.2479	21955.844	3.17
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.015	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.015	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.001	0.00
AVG. HEAD ON TOP OF LAYER 6	0.0000		
CHANGE IN WATER STORAGE	0.047	4124.782	0.59
SOIL WATER AT START OF YEAR	395,422	3502436.000	
SOIL WATER AT END OF YEAR	395,422	3502436.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.337	0.00

ANNUAL TOTALS FOR YEAR 6

	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.08	449945.634	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	4.961	436196.531	97.66
DRAINAGE COLLECTED FROM LAYER 2	0.1739	15402.591	3.42

PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.017	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.016	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.001	0.00
AVG. HEAD ON TOP OF LAYER 6	0.0000		
CHANGE IN WATER STORAGE	0.055	-4884.193	-1.08
SOIL WATER AT START OF YEAR	385,472	3502446.000	
SOIL WATER AT END OF YEAR	385,398	3502390.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-1.130	0.00

ANNUAL TOTALS FOR YEAR 7

	INCHES	CU. FEET	PERCENT
PRECIPITATION	6.83	608946.750	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.401	360989.312	93.73
DRAINAGE COLLECTED FROM LAYER 2	0.1384	12344.782	2.04
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.032	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.011	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 6	0.0000		
CHANGE IN WATER STORAGE	0.289	25813.656	4.23
SOIL WATER AT START OF YEAR	395,368	3502850.000	
SOIL WATER AT END OF YEAR	395,658	35044370.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-1.024	0.00

ANNUAL TOTALS FOR YEAR 8

	INCHES	CU. FEET	PERCENT
PRECIPITATION	3.78	334602.125	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	3.597	318190.275	95.16
DRAINAGE COLLECTED FROM LAYER 2	0.1021	9046.833	2.70
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.012	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.013	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.001	0.00
AVG. HEAD ON TOP OF LAYER 6	0.0000		
CHANGE IN WATER STORAGE	0.081	7165.663	2.14
SOIL WATER AT START OF YEAR	395,619	35044176.000	
SOIL WATER AT END OF YEAR	395,738	35051740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.782	0.00

ANNUAL TOTALS FOR YEAR 9

	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.47	395926.906	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	8.795	424739.312	107.28
DRAINAGE COLLECTED FROM LAYER 2	0.1210	10718.074	2.71
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.009	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.008	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.001	0.00
AVG. HEAD ON TOP OF LAYER 6	0.0000		
CHANGE IN WATER STORAGE	-0.448	-16542.239	-4.59

SOIL WATER AT START OF YEAR	395.738	3501340.000
SOIL WATER AT END OF YEAR	395.292	35011800.000
SNOW WATER AT START OF YEAR	0.000	0.000
SNOW WATER AT END OF YEAR	0.000	0.000
ANNUAL WATER BUDGET BALANCE	0.0000	1.771

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 9

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.18 0.00	0.77 0.00	1.10 0.01	0.55 0.26	0.17 1.19	0.08 0.35
STD. DEVIATIONS	0.01 0.01	0.03 0.00	0.75 0.02	0.43 0.37	0.27 0.78	0.08 0.31
RUNOFF						
TOTALS	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000
STD. DEVIATIONS	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000
EVAPOTRANSPIRATION						
TOTALS	0.925 0.007	0.998 0.005	1.052 0.005	0.701 0.130	0.255 0.792	0.041 0.265
STD. DEVIATIONS	0.349 0.034	0.484 0.000	0.782 0.036	0.473 0.293	0.258 0.426	0.073 0.327
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	0.0340 0.0001	0.0323 0.0000	0.0375 0.0001	0.0314 0.0042	0.0044 0.0332	0.0013 0.0313
STD. DEVIATIONS	0.0473 0.0000	0.0189 0.0000	0.0435 0.0004	0.0459 0.0222	0.0064 0.0241	0.0032 0.0498
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 5						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 7						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 6						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 9

	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.80 (1.834)	513520.7	100.00
RUNOFF	0.000 (0.0000)	0.00	0.000
EVAPOTRANSPIRATION	5.610 (1.8817)	496873.00	96.758
LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.19405 (0.06791)	17185.365	3.34658
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000 (0.00000)	0.015	0.00000
AVERAGE HEAD ON TOP OF LAYER 3	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 5	0.00000 (0.00000)	0.014	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000 (0.00000)	0.002	0.00000
AVERAGE HEAD ON TOP OF LAYER 6	0.000 (0.000)		
CHANGE IN WATER STORAGE	-0.000 (0.8683)	-537.90	-0.105

PEAK DAILY VALUES FOR YEARS 1 THROUGH 9

	(INCHES)	(CU. FT.)
PRECIPITATION	0.97	85914.844
RUNOFF	0.000	0.0000
DRAINAGE COLLECTED FROM LAYER 2	0.04235	3751.29815
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 3	0.000	
MAXIMUM HEAD ON TOP OF LAYER 3	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 5	0.00000	0.00005
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000	0.00004
AVERAGE HEAD ON TOP OF LAYER 6	0.000	
MAXIMUM HEAD ON TOP OF LAYER 6	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	0.14	12427.9756
MAXIMUM VOL. SOIL WATER (VOL./VOL.)		0.3414
MINIMUM VOL. SOIL WATER (VOL./VOL.)		0.0770

*** Maximum heads are computed using McCoron's equations. ***
 Reference: Maximum Saturated Depth over Landfill Liner
 by Bruce M. McCoron, University of Kansas
 ASCE Journal of Environmental Engineering
 Vol. 130, No. 3, March 2004, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 9

LAYER	(INCHES)	(VOL./VOL.)
1	304.5439	0.2866
2	0.0000	0.0100
3	0.0000	0.0000
6	35.1770	0.4270
7	0.0000	0.0100

0	0.0000	0.0000
7	11.3710	0.4270
SNOW WATER	0.000	

PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	2.703	0.00
SOIL WATER AT START OF YEAR	407.225	36068736.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.418	0.00

ANNUAL TOTALS FOR YEAR 2			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.78	511946.156	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.618	54732.117	10.69
DRAINAGE COLLECTED FROM LAYER 2	5.1621	457213.791	89.31
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.191	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0019		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00

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ANNUAL WATER BUDGET BALANCE 0.0000 0.250 0.00

ANNUAL TOTALS FOR YEAR 3

	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.30	823719.697	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.988	87499.155	10.62
DRAINAGE COLLECTED FROM LAYER 2	8.3109	736331.750	89.36
PERC./LEAKAGE THROUGH LAYER 4	0.000003	0.295	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0021		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.001	108.170	0.01
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.226	36068848.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.280	0.00

ANNUAL TOTALS FOR YEAR 4

	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.19	405543.533	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.182	11360.190	12.68
DRAINAGE COLLECTED FROM LAYER 2	4.0091	355092.562	87.34

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PERC./LEAKAGE THROUGH LAYER 8	0.000002	0.312	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0015		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
DRAINAGE COLLECTED FROM LAYER 10	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	-0.001	-105.417	-0.03
SOIL WATER AT START OF YEAR	407.226	3606888.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-2.201	0.00

ANNUAL TOTALS FOR YEAR 5			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	7.83	693519.750	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.884	76541.113	11.04
DRAINAGE COLLECTED FROM LAYER 2	6.9458	616977.187	88.96
PERC./LEAKAGE THROUGH LAYER 4	0.000003	0.247	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0026		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	

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SOIL WATER AT END OF YEAR 407.225 36068740.000
 SNOW WATER AT START OF YEAR 0.000 0.000 0.00
 SNOW WATER AT END OF YEAR 0.000 0.000 0.00
 ANNUAL WATER BUDGET BALANCE 0.0000 0.433 0.00

ANNUAL TOTALS FOR YEAR 6

	INCHES	CU. FEET	PERCENT
PRECIPITATION	3.08	449043.030	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.557	49359.320	10.97
DRAINAGE COLLECTED FROM LAYER 2	4.5227	400596.230	89.03
PERC./LEAKAGE THROUGH LAYER 4	0.000004	0.217	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0027		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.090	0.00

ANNUAL TOTALS FOR YEAR 7

	INCHES	CU. FEET	PERCENT
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PRECIPITATION	6.83	604946.750	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.371	50583.484	8.38
DRAINAGE COLLECTED FROM LAYER 2	8.2469	513209.937	91.46
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.219	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0023		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.012	1062.280	0.18
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.237	36069804.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	3.659	0.00

ANNUAL TOTALS FOR YEAR 8			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.78	334802.125	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.530	46957.109	14.02
DRAINAGE COLLECTED FROM LAYER 2	3.2266	285790.281	85.26
PERC./LEAKAGE THROUGH LAYER 4	0.000001	0.121	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0012		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00

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PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.023	2054.282	0.61
SOIL WATER AT START OF YEAR	407.227	36068604.000	
SOIL WATER AT END OF YEAR	407.260	36071856.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.449	0.00

ANNUAL TOTALS FOR YEAR 9			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.47	395916.906	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.311	27571.871	6.96
DRAINAGE COLLECTED FROM LAYER 2	4.1537	371445.926	93.82
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.157	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0015		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	-0.031	-1102.344	-0.78
SOIL WATER AT START OF YEAR	407.260	36071856.000	
SOIL WATER AT END OF YEAR	407.225	36068750.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.573	0.00

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PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	-10.218	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.031	0.00

ANNUAL TOTALS FOR YEAR 10			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	6.54	579260.937	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.722	54042.954	11.09
DRAINAGE COLLECTED FROM LAYER 2	5.8171	51234.219	88.95
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.212	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0021		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	-10.218	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.031	0.00

ANNUAL TOTALS FOR YEAR 11			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.71	505746.002	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.577	51107.449	10.11
DRAINAGE COLLECTED FROM LAYER 2	5.1320	454610.531	89.89
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.185	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0019		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00

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PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.084	0.00

ANNUAL TOTALS FOR YEAR 12			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	6.09	530403.437	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.795	62287.086	11.55
DRAINAGE COLLECTED FROM LAYER 2	5.3898	477116.250	88.45
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.193	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0020		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00

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ANNUAL WATER BUDGET BALANCE	0.0000	0.1111	0.00

ANNUAL TOTALS FOR YEAR 13			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.58	405859.719	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.583	51922.996	12.73
DRAINAGE COLLECTED FROM LAYER 2	7.9272	354936.656	87.27
PERC./LEAKAGE THROUGH LAYER 4	0.000002	-0.155	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0015		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.053	0.00

ANNUAL TOTALS FOR YEAR 14			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	8.34	738690.562	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.801	70929.266	9.60

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DRAINAGE COLLECTED FROM LAYER 2	7.5392	667751.000	90.40
PERC./LEAKAGE THROUGH LAYER 4	0.000001	0.271	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0028		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.117	0.00

ANNUAL TOTALS FOR YEAR 15			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	3.88	343659.312	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.779	68927.977	20.06
DRAINAGE COLLECTED FROM LAYER 2	3.1018	274781.187	79.94
PERC./LEAKAGE THROUGH LAYER 4	0.000001	0.113	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0021		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00

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SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.143	0.00

ANNUAL TOTALS FOR YEAR 16			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.47	484488.957	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.566	50116.070	10.34
DRAINAGE COLLECTED FROM LAYER 2	4.9042	434372.562	89.66
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.175	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0018		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.306	0.00

ANNUAL TOTALS FOR YEAR 17			
	INCHES	CU. FEET	PERCENT

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PRECIPITATION	4.84	428098.931	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.710	62918.406	14.68
DRAINAGE COLLECTED FROM LAYER 2	4.1290	361765.844	85.32
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.134	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0021		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.280	0.00

ANNUAL TOTALS FOR YEAR 18			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.80	423145.162	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.560	49572.629	11.66
DRAINAGE COLLECTED FROM LAYER 2	4.2403	371772.844	88.34
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.134	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0026		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00

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PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	2.703	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-2.597	0.00

ANNUAL TOTALS FOR YEAR 19

	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.19	459088.697	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.458	40595.770	8.82
DRAINAGE COLLECTED FROM LAYER 2	4.7323	419512.719	91.18
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.177	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0018		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.193	0.00

ANNUAL TOTALS FOR YEAR 20

	INCHES	CU. FEET	PERCENT
PRECIPITATION	3.10	274575.136	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.364	32246.084	11.74
DRAINAGE COLLECTED FROM LAYER 2	2.7359	242326.909	89.26
PERC./LEAKAGE THROUGH LAYER 4	0.000001	0.103	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.108	0.00

ANNUAL TOTALS FOR YEAR 21

	INCHES	CU. FEET	PERCENT
PRECIPITATION	3.92	348087.906	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.459	40604.238	11.65
DRAINAGE COLLECTED FROM LAYER 2	3.4706	307391.567	88.31
PERC./LEAKAGE THROUGH LAYER 4	0.000001	0.126	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0013		

DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.106	0.00

ANNUAL TOTALS FOR YEAR 22

	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.65	500431.750	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.649	57493.078	11.49
DRAINAGE COLLECTED FROM LAYER 2	5.0009	442938.594	88.51
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.178	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0018		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00

SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.090	0.00

ANNUAL TOTALS FOR YEAR 23

	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.54	410974.094	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.778	68917.062	16.77
DRAINAGE COLLECTED FROM LAYER 2	3.8619	342056.937	83.75
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.150	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0013		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.116	0.00

ANNUAL TOTALS FOR YEAR 24

	INCHES	CU. FEET	PERCENT
PRECIPITATION	6.59	583689.500	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.797	70612.555	12.10

DRAINAGE COLLECTED FROM LAYER 2	5.7928	513076.687	87.90
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.201	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0022		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0003		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.232	0.00

ANNUAL TOTALS FOR YEAR 25			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.43	831233.873	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	1.602	86758.125	10.43
DRAINAGE COLLECTED FROM LAYER 2	5.4279	746475.437	89.37
PERC./LEAKAGE THROUGH LAYER 4	0.000003	0.288	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0021		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00

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SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.296	0.00

ANNUAL TOTALS FOR YEAR 26			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.59	415402.667	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.478	37909.023	9.13
DRAINAGE COLLECTED FROM LAYER 2	4.2560	177493.625	90.87
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.146	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0016		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.042	0.00

ANNUAL TOTALS FOR YEAR 27			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.59	408545.437	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.447	10193.602	9.64
DRAINAGE COLLECTED FROM LAYER 2	4.1471	167334.312	90.36
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.135	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0016		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	-2.707	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.235	0.00

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PRECIPITATION	4.32	400343.373	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.744	61890.930	16.46
DRAINAGE COLLECTED FROM LAYER 2	3.7760	134446.250	83.54
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.149	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0014		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.216	0.00

ANNUAL TOTALS FOR YEAR 28			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	7.65	678461.562	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.888	78641.695	11.59
DRAINAGE COLLECTED FROM LAYER 2	6.7721	599819.500	88.41
PERC./LEAKAGE THROUGH LAYER 4	0.000003	0.238	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0025		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		

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DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	0.000	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068744.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.317	0.00

ANNUAL TOTALS FOR YEAR 29			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	4.59	408545.437	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.447	10193.602	9.64
DRAINAGE COLLECTED FROM LAYER 2	4.1471	167334.312	90.36
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.135	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0016		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.000	-2.707	0.00
SOIL WATER AT START OF YEAR	407.225	36068744.000	
SOIL WATER AT END OF YEAR	407.225	36068740.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.235	0.00

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ANNUAL TOTALS FOR YEAR 30			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	6.90	697289.500	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	0.664	58910.984	9.77
DRAINAGE COLLECTED FROM LAYER 2	0.1284	542808.875	90.12
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.223	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0023		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 8	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 9	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 11	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 10	0.0000		
CHANGE IN WATER STORAGE	0.007	651.424	0.11
SOIL WATER AT START OF YEAR	407.225	36068740.000	
SOIL WATER AT END OF YEAR	407.232	36069302.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-1.774	0.00

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 30						
	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.15	0.78	0.99	0.62	0.24	0.02
STD. DEVIATIONS	0.70	0.81	0.81	0.42	0.37	0.06
RUNOFF						
TOTALS	0.000	0.000	0.000	0.000	0.000	0.000

STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000
EVAPOTRANSPIRATION						
TOTALS	0.077	0.045	0.110	0.132	0.061	0.014
STD. DEVIATIONS	0.008	0.017	0.027	0.023	0.071	0.058
YTD. DEVIATIONS	0.015	0.011	0.006	0.090	0.080	0.073
STD. DEVIATIONS	0.005	0.013	0.044	0.050	0.050	0.032
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	1.0736	0.7122	0.8766	0.4901	0.1803	0.0545
STD. DEVIATIONS	0.0000	0.0000	0.0383	0.2330	0.8614	0.4649
STD. DEVIATIONS	0.5607	0.5630	0.1456	0.3338	0.2427	0.2456
STD. DEVIATIONS	0.0000	0.0021	0.2293	0.4030	0.8921	0.5267
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 6						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 8						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 9						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 11						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3

AVERAGES	0.0047	0.0034	0.0038	0.0022	0.0008	0.0001
STD. DEVIATIONS	0.0019	0.0017	0.0024	0.0015	0.0011	0.0002
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 10						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 30			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.64 (1.424)	499605.2	100.00
RUNOFF	0.000 (0.0000)	0.00	0.000
EVAPOTRANSPIRATION	0.618 (0.1779)	57350.06	11.40
LATERAL DRAINAGE COLLECTED FROM LAYER 2	4.99286 (1.49339)	442227.187	80.5134
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000 (0.00000)	0.181	0.0004
AVERAGE HEAD ON TOP OF LAYER 3	0.002 (0.001)		
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00000 (0.00000)	0.000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00000 (0.00000)	0.000	0.0000
AVERAGE HEAD ON TOP OF LAYER 7	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 9	0.00000 (0.00000)	0.000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.00000 (0.00000)	0.000	0.0000
AVERAGE HEAD ON TOP OF LAYER 10	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.000 (0.0002)	21.89	0.004

PEAK DAILY VALUES FOR YEARS 1 THROUGH 30		
	(INCHES)	(CU. FT.)
PRECIPITATION	1.54	136400.875
RUNOFF	0.000	0.0000
DRAINAGE COLLECTED FROM LAYER 2	1.28421	118759.92909
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000000	0.03481
AVERAGE HEAD ON TOP OF LAYER 3	0.173	
MAXIMUM HEAD ON TOP OF LAYER 3	0.330	
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	2.3 FEET	
DRAINAGE COLLECTED FROM LAYER 6	0.00000	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 7	0.000	
MAXIMUM HEAD ON TOP OF LAYER 7	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 9	0.00000	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 11	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 10	0.000	
MAXIMUM HEAD ON TOP OF LAYER 10	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	0.14	12427.9750
MAXIMUM VEG. SOIL WATER (VOL./VOL.)		0.0786
MINIMUM VEG. SOIL WATER (VOL./VOL.)		0.0512

*** Maximum heads are computed using McInroe's equations. ***

Reference: Maximum Calculated Depth over Landfill Liner by Bruce M. McInroe, University of Kansas AGU Journal of Environmental Engineering Vol. 119, No. 2, March 1994, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 30		
LAYER	(INCHES)	(VOL./VOL.)

1	1.6558	0.0090
7	0.0254	0.1266
5	0.0000	0.0000
4	20.3480	0.4270
3	964.5553	0.2866
6	0.0020	0.0100
7	0.0000	0.0000
8	15.3770	0.4270
9	0.0070	0.0160
10	0.0000	0.0000
11	15.3720	0.4270
SHOW WATER	0.000	

LANDFILL P

CELL P-1

INTERMEDIATE COVER

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT SOIL DATA BASE USING SOIL TEXTURE #18 WITH A FAIR STAND OF GRASS, A SURFACE SLOPE OF 1.1% AND A SLOPE LENGTH OF 50. FEET.

SCS RUNOFF CURVE NUMBER	=	59.58
FRACTION OF AREA ALLOWING RUNOFF	=	100.0 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE	=	100.0000 ACRES
EVAPORATIVE ZONE DEPTH	=	22.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE	=	3.530 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE	=	14.752 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	1.654 INCHES
INITIAL SNOW WATER	=	0.000 INCHES
INITIAL WATER IN LAYER MATERIALS	=	154.236 INCHES
TOTAL INITIAL WATER	=	154.236 INCHES
TOTAL SUBSURFACE INFLOW	=	0.00 INCHES/YEAR

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.95 2.83	1.35 2.48	2.12 3.28	3.11 2.48	4.01 1.58	4.02 1.29
STD. DEVIATIONS	0.58 1.76	0.53 1.61	1.28 1.84	1.51 1.95	3.01 1.23	2.06 0.91
RUNOFF						
TOTALS	0.091 0.209	0.011 0.200	0.030 0.080	0.050 0.001	0.007 0.200	0.001 0.002
STD. DEVIATIONS	0.009 0.000	0.004 0.000	0.000 0.002	0.004 0.000	0.036 0.000	0.011 0.015
EVAPOTRANSPIRATION						

TOTALS	1.128 3.288	1.316 2.485	2.051 3.286	2.590 1.408	4.332 1.214	4.912 1.009
STD. DEVIATIONS	0.406 1.000	0.460 1.537	0.773 1.978	1.051 0.675	1.373 0.421	1.563 0.372

LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	0.1210 0.1472	0.0613 0.0100	0.0513 0.0004	0.0030 0.1100	0.2104 0.2100	0.3047 0.2170
STD. DEVIATIONS	0.3392 0.3339	0.1635 0.0534	0.1205 0.0305	0.2624 0.3300	0.4601 0.4000	0.0137 0.5290

PERCOLATION/LEAKAGE THROUGH LAYER 3						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0001 0.0000
STD. DEVIATIONS	0.0001 0.0001	0.0000 0.0000	0.0000 0.0000	0.0001 0.0001	0.0001 0.0001	0.0001 0.0001

LATERAL DRAINAGE COLLECTED FROM LAYER 5						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0001	0.0000 0.0001	0.0000 0.0000

PERCOLATION/LEAKAGE THROUGH LAYER 6						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

LATERAL DRAINAGE COLLECTED FROM LAYER 7						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

PERCOLATION/LEAKAGE THROUGH LAYER 8						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	0.0001 0.0001	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0001 0.0001	0.0002 0.0001
STD. DEVIATIONS	0.0001 0.0002	0.0001 0.0000	0.0001 0.0000	0.0001 0.0001	0.0002 0.0002	0.0004 0.0002
DAILY AVERAGE HEAD ON TOP OF LAYER 5						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 6						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

EVAPOTRANSPIRATION	28.689	(4.5956)	50414245.00	94.540
LATERAL DRAINAGE COLLECTED FROM LAYER 2	1.63760	(1.91724)	594447.502	5.39584
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.0000	(0.00000)	119.900	0.00189
AVERAGE HEAD ON TOP OF LAYER 3	0.000	(0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 5	0.00009	(0.00012)	71.326	0.00063
PERCOLATION/LEAKAGE THROUGH LAYER 6	0.00013	(0.00013)	49.477	0.00044
AVERAGE HEAD ON TOP OF LAYER 5	0.000	(0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 7	0.00013	(0.00013)	48.444	0.00044
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00000	(0.00000)	0.833	0.00000
AVERAGE HEAD ON TOP OF LAYER 6	0.000	(0.000)		
CHANGE IN WATER STORAGE	-0.008	(2.0736)	-2754.34	-0.025

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	5.36	1345000.000
RUNOFF	0.245	60020.3510
DRAINAGE COLLECTED FROM LAYER 2	0.16210	50073.65020

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	INCHES	CU. FEET	PERCENT	
PRECIPITATION	30.34	(0.245)	11014725.0	100.00
RUNOFF	0.024	(0.0552)	6660.20	0.072

PERCOLATION/LEAKAGE THROUGH LAYER 3	0.00000	19.19121
AVERAGE HEAD ON TOP OF LAYER 3	0.002	
MAXIMUM HEAD ON TOP OF LAYER 3	0.004	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 5	0.00001	9.61864
PERCOLATION/LEAKAGE THROUGH LAYER 6	0.00000	2.00181
AVERAGE HEAD ON TOP OF LAYER 6	0.000	
MAXIMUM HEAD ON TOP OF LAYER 6	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 7	0.00001	2.00045
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00000	0.00212
AVERAGE HEAD ON TOP OF LAYER 8	0.000	
MAXIMUM HEAD ON TOP OF LAYER 8	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	2.01	837825.3128
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.4650
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0770

*** Maximum heads are computed using McBratne's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner
by Bruce R. McBratne, University of Kansas
ASCE Journal of Environmental Engineering
Vol. 115, No. 2, March 1989, pp. 292-270.

FINAL WATER STORAGE AT END OF YEAR 199		
LAYER	(INCHES)	(VOL/VOL)
1	126.5874	0.2884
2	0.0020	0.0100
3	0.0000	0.0000
4	1.0000	0.0450
5	0.0020	0.0100
6	0.0000	0.0000
7	0.0020	0.0100
8	0.0000	0.0000
9	15.0844	0.4270
SNOW WATER	0.000	

FINAL COVER

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+-----+
+
+ HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
+ HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
+ DEVELOPED BY ENVIRONMENTAL LABORATORY
+ USAE WATERWAYS EXPERIMENTAL STATION
+ FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
+
+-----+

PRECIPITATION DATA FILE: C:\PIC\DATA4.D4
TEMPERATURE DATA FILE: C:\PIC\DATA7.G7
SOLAR RADIATION DATA FILE: C:\PIC\DATA3.DL3
EVAPOTRANSPIRATION DATA: C:\PIC\DATA11.DL1
SOIL AND DESIGN DATA FILE: C:\PIC\DATA10.D4
OUTPUT DATA FILE: C:\PIC\OUT.OUT

TIME: 11:56 DATE: 4/10/2016

+-----+
+ TITLE: P1-0408
+-----+

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE
      COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

      LAYER 1
      -----
      TYPE 1 - VERTICAL PERCOLATION LAYER
      MATERIAL TEXTURE NUMBER 1
      Page 1
  
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THICKNESS = 26.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0450 VOL/VOL
WILTING POINT = 0.0130 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.9999999999E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
      FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

      LAYER 2
      -----
      TYPE 2 - LATERAL DRAINAGE LAYER
      MATERIAL TEXTURE NUMBER 34
      THICKNESS = 0.20 INCHES
      POROSITY = 0.0500 VOL/VOL
      FIELD CAPACITY = 0.0100 VOL/VOL
      WILTING POINT = 0.0050 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 33.0000000000 CM/SEC
      SLOPE = 0.50 PERCENT
      DRAINAGE LENGTH = 50.0 FEET

      LAYER 3
      -----
      TYPE 4 - FLEXIBLE MEMBRANE LINER
      MATERIAL TEXTURE NUMBER 35
      THICKNESS = 0.00 INCHES
      POROSITY = 0.0000 VOL/VOL
      FIELD CAPACITY = 0.0000 VOL/VOL
      WILTING POINT = 0.0000 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 0.1000000000E-12 CM/SEC
      PML PINHOLE DENSITY = 2.00 HOLES/ACRE
      PML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
      PML PLACEMENT QUALITY = 3 - GOOD

      LAYER 4
      -----
      Page 2
  
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      TYPE 3 - BARRIER SOIL LINER
      MATERIAL TEXTURE NUMBER 16
      THICKNESS = 24.00 INCHES
      POROSITY = 0.4270 VOL/VOL
      FIELD CAPACITY = 0.0120 VOL/VOL
      WILTING POINT = 0.0070 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 0.1000000000E-06 CM/SEC

      LAYER 5
      -----
      TYPE 1 - VERTICAL PERCOLATION LAYER
      MATERIAL TEXTURE NUMBER 1E
      THICKNESS = 480.00 INCHES
      POROSITY = 0.4710 VOL/VOL
      FIELD CAPACITY = 0.2500 VOL/VOL
      WILTING POINT = 0.0770 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.2500 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 0.1000000000E-02 CM/SEC

      LAYER 6
      -----
      TYPE 2 - LATERAL DRAINAGE LAYER
      MATERIAL TEXTURE NUMBER 34
      THICKNESS = 0.20 INCHES
      POROSITY = 0.0500 VOL/VOL
      FIELD CAPACITY = 0.0100 VOL/VOL
      WILTING POINT = 0.0050 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 33.0000000000 CM/SEC
      SLOPE = 0.50 PERCENT
      DRAINAGE LENGTH = 50.0 FEET

      LAYER 7
      -----
      TYPE 4 - FLEXIBLE MEMBRANE LINER
      Page 3
  
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      MATERIAL TEXTURE NUMBER 35
      THICKNESS = 0.00 INCHES
      POROSITY = 0.0000 VOL/VOL
      FIELD CAPACITY = 0.0000 VOL/VOL
      WILTING POINT = 0.0000 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 0.1000000000E-12 CM/SEC
      PML PINHOLE DENSITY = 2.00 HOLES/ACRE
      PML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
      PML PLACEMENT QUALITY = 3 - GOOD

      LAYER 8
      -----
      TYPE 1 - VERTICAL PERCOLATION LAYER
      MATERIAL TEXTURE NUMBER 1
      THICKNESS = 24.00 INCHES
      POROSITY = 0.4170 VOL/VOL
      FIELD CAPACITY = 0.0450 VOL/VOL
      WILTING POINT = 0.0130 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.0450 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 0.9999999999E-02 CM/SEC

      LAYER 9
      -----
      TYPE 2 - LATERAL DRAINAGE LAYER
      MATERIAL TEXTURE NUMBER 34
      THICKNESS = 0.20 INCHES
      POROSITY = 0.0500 VOL/VOL
      FIELD CAPACITY = 0.0100 VOL/VOL
      WILTING POINT = 0.0050 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 33.0000000000 CM/SEC
      SLOPE = 0.50 PERCENT
      DRAINAGE LENGTH = 50.0 FEET

      LAYER 10
      -----
      Page 4
  
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TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35

THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
MELTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.199999999999999E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 3 - GOOD
FML PLACEMENT QUALITY = 3 - GOOD

LAYER 11

TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34

THICKNESS = 0.20 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
MELTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.0000000000000E-12 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 50.0 FEET

LAYER 12

TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35

THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
MELTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.199999999999999E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 3 - GOOD
FML PLACEMENT QUALITY = 3 - GOOD

Page 5

LAYER 13

TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 16

THICKNESS = 37.20 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
MELTING POINT = 0.3670 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.100000000000000E-06 CM/SEC

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT SOIL DATA BASE USING SOIL TEXTURE # 1 WITH A FAIR STAND OF GRASS, A SURFACE SLOPE OF 1:1 AND A SLOPE LENGTH OF 50. FEET.

SCS RUNOFF CURVE NUMBER = 46.40
FRACTION OF AREA ALLOWING RUNOFF = 100.0 PERCENT
AREA PROTECTED ON HORIZONTAL PLANE = 100.0000 ACRES
EVAPORATIVE ZONE DEPTH = 27.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 0.396 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 5.174 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE = 0.396 INCHES
INITIAL SNOW WATER = 0.000 INCHES
INITIAL WATER IN LAYER MATERIALS = 169.176 INCHES
TOTAL INITIAL WATER = 169.176 INCHES
TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100

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	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.05 2.03	1.36 2.40	2.12 3.20	3.11 2.48	4.91 1.54	4.02 1.29
STD. DEVIATIONS	0.60 1.76	0.83 1.61	1.20 1.94	1.91 1.95	1.81 1.28	2.06 0.91
RUNOFF						
TOTALS	0.001 0.000	0.012 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000
STD. DEVIATIONS	0.009 0.000	0.047 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.016
EVAPOTRANSPIRATION						
TOTALS	0.700 2.921	0.898 1.940	1.400 1.961	1.988 1.357	2.205 0.984	3.195 0.057
STD. DEVIATIONS	0.437 1.411	0.403 1.090	0.734 1.090	0.943 0.896	1.200 0.566	1.419 0.479
LATERAL DRAINAGE COLLECTED FROM LAYER 12						
TOTALS	0.3840 0.8374	0.3450 0.5808	0.5617 0.8786	0.8356 0.9994	1.6470 0.6794	1.1770 0.4653
STD. DEVIATIONS	0.2395 0.6709	0.1097 0.3562	0.3162 0.6070	0.7921 0.8074	1.5970 0.6109	0.7865 0.3907
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 8						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

Page 7

	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 7						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 9						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 10						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 11						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 13						
TOTALS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3

Page 8

AVERAGES	0.0007 0.0010	0.0007 0.0010	0.0010 0.0012	0.0010 0.0018	0.0040 0.0017	0.0021 0.0008
STD. DEVIATIONS	0.0004 0.0005	0.0004 0.0006	0.0005 0.0011	0.0002 0.0020	0.0116 0.0011	0.0014 0.0007
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 10						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 12						
AVERAGES	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
STD. DEVIATIONS	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES		CU. FEET		PERCENT	
PRECIPITATION	36.34	(6.245)	110447.25	0	100.00	
RUNOFF	0.815	(0.0496)	5352.04	0.049		
EVAPOTRANSPIRATION	21.091	(3.8916)	7655078.50	69.506		
LATERAL DRAINAGE COLLECTED FROM LAYER 2	9.2396	(2.0765)	335160.500	30.4207		
Page 9						

PERCOLATION/LEAKAGE THROUGH LAYER 4	0.0000 (0.0000)	1.474	0.0001	
AVERAGE HEAD ON TOP OF LAYER 3	0.002 (0.001)			
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.0000 (0.0000)	1.430	0.0001	
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.0000 (0.0000)	0.844	0.0000	
AVERAGE HEAD ON TOP OF LAYER 7	0.000 (0.000)			
LATERAL DRAINAGE COLLECTED FROM LAYER 9	0.0000 (0.0000)	0.000	0.0000	
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.0000 (0.0000)	0.004	0.0000	
AVERAGE HEAD ON TOP OF LAYER 10	0.000 (0.000)			
LATERAL DRAINAGE COLLECTED FROM LAYER 11	0.0000 (0.0000)	0.023	0.0000	
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.0000 (0.0000)	0.021	0.0000	
AVERAGE HEAD ON TOP OF LAYER 12	0.000 (0.000)			
CHANGE IN WATER STORAGE	0.005 (0.0357)	1852.60	0.017	

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100				
	INCHES		CU. FT.	
PRECIPITATION	5.36	194500.000		
Page 10				

RUNOFF	0.343	2219.7217
DRAINAGE COLLECTED FROM LAYER 2	2.4500	88575.75000
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000	0.77051
AVERAGE HEAD ON TOP OF LAYER 3	1.014	
MAXIMUM HEAD ON TOP OF LAYER 3	1.138	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	10.2 FEET	
DRAINAGE COLLECTED FROM LAYER 6	0.00000	0.72551
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000	0.00077
AVERAGE HEAD ON TOP OF LAYER 7	0.000	
MAXIMUM HEAD ON TOP OF LAYER 7	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 9	0.00000	0.00003
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00000	0.00004
AVERAGE HEAD ON TOP OF LAYER 10	0.000	
MAXIMUM HEAD ON TOP OF LAYER 10	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 11	0.00000	0.00004
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.00000	0.00010
AVERAGE HEAD ON TOP OF LAYER 12	0.000	
MAXIMUM HEAD ON TOP OF LAYER 12	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 11 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	2.31	837825.8138
Page 11		

MAXIMUM VEG. SOIL WATER (VOL/VOL)	0.200	
MINIMUM VEG. SOIL WATER (VOL/VOL)	0.010	
*** Maximum heads are computed using McEnroe's equations. ***		
Reference: Maximum Saturated Depth over Landfill Liner by Bruce M. McEnroe, University of Kansas ASCE Journal of Environmental Engineering Vol. 119, No. 2, March 1993, pp. 262-270.		

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)
1	7.3172	0.0644
2	0.0021	0.0105
3	0.0000	0.0000
4	10.7480	0.4270
5	140.1600	0.2920
6	0.0020	0.0100
7	0.0000	0.0000
8	1.0000	0.0450
9	0.0020	0.0100
10	0.0000	0.0000
11	0.0020	0.0100
12	0.0000	0.0000
Page 12		

CELLS P-2 TO P-4

INTERMEDIATE COVER

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT SOIL DATA BASE USING SOIL TEXTURE #18 WITH A FAIR STAND OF GRASS, A SURFACE SLOPE OF 1.1% AND A SLOPE LENGTH OF 50. FEET.

SCS RUNOFF CURVE NUMBER	=	59.58
FRACTION OF AREA ALLOWING RUNOFF	=	100.00 PERCENT
AREA PROTECTED ON HORIZONTAL PLANE	=	100.0000 ACRES
EVAPORATIVE ZONE DEPTH	=	22.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE	=	3.539 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE	=	14.752 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	1.654 INCHES
INITIAL SNOW WATER	=	0.000 INCHES
INITIAL WATER IN LAYER MATERIALS	=	189.376 INCHES
TOTAL INITIAL WATER	=	189.276 INCHES
TOTAL SUBSURFACE INFLOW	=	0.00 INCHES/YEAR

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.89	1.36	2.12	3.11	4.91	4.07
STD. DEVIATIONS	0.68	0.33	1.20	1.91	3.01	2.86
	1.76	1.61	1.84	1.95	1.28	0.91
RUNOFF						
TOTALS	0.001	0.011	0.000	0.000	0.007	0.001
STD. DEVIATIONS	0.000	0.000	0.000	0.001	0.000	0.002
	0.000	0.000	0.000	0.004	0.037	0.011

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0.000 0.000 0.002 0.000 0.000 0.016

EVAPOTRANSPIRATION

TOTALS	1.128	1.319	2.052	2.586	4.910	4.910
STD. DEVIATIONS	0.407	0.468	0.768	1.265	1.379	1.956
	1.802	1.558	1.028	0.676	0.420	0.372

LATERAL DRAINAGE COLLECTED FROM LAYER 2

TOTALS	0.1302	0.0645	0.0583	0.0046	0.2198	0.2752
STD. DEVIATIONS	0.1545	0.0110	0.0000	0.1089	0.2007	0.2195
	0.3288	0.1603	0.1362	0.2385	0.4675	0.7597
	0.5527	0.0655	0.0382	0.3350	0.4753	0.5362

PERCOLATION/LEAKAGE THROUGH LAYER 3

TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0001
STD. DEVIATIONS	0.0001	0.0000	0.0000	0.0000	0.0001	0.0001
	0.0001	0.0000	0.0000	0.0001	0.0001	0.0001

LATERAL DRAINAGE COLLECTED FROM LAYER 5

TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0001	0.0000	0.0001	0.0001	0.0000

PERCOLATION/LEAKAGE THROUGH LAYER 6

TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

LATERAL DRAINAGE COLLECTED FROM LAYER 7

TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0001
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

Page 6

PERCOLATION/LEAKAGE THROUGH LAYER 9

TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3

AVERAGES	0.0001	0.0000	0.0000	0.0000	0.0001	0.0002
STD. DEVIATIONS	0.0001	0.0001	0.0001	0.0001	0.0002	0.0004
	0.0002	0.0000	0.0000	0.0001	0.0002	0.0002

DAILY AVERAGE HEAD ON TOP OF LAYER 5

AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

DAILY AVERAGE HEAD ON TOP OF LAYER 8

AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	INCHES	CU. FEET	PERCENT
PRECIPITATION	5.36	1345880,800	
RUNOFF	0.349	90220,3516	

Page 7

PRECIPITATION 30.34 (0.745) 11014725.0 100.00

RUNOFF 0.024 (0.057) 8700.25 0.075

EVAPOTRANSPIRATION 20.697 (4.6018) 10417129.00 94.575

LATERAL DRAINAGE COLLECTED FROM LAYER 2 1.62975 (1.90959) 591589.375 5.37990

PERCOLATION/LEAKAGE THROUGH LAYER 3 0.00003 (0.00006) 119.544 0.00109

AVERAGE HEAD ON TOP OF LAYER 3 0.000 (0.000)

LATERAL DRAINAGE COLLECTED FROM LAYER 5 0.00020 (0.00012) 71.001 0.00065

PERCOLATION/LEAKAGE THROUGH LAYER 6 0.00015 (0.00013) 42.063 0.00044

AVERAGE HEAD ON TOP OF LAYER 6 0.000 (0.000)

LATERAL DRAINAGE COLLECTED FROM LAYER 7 0.00013 (0.00013) 48.430 0.00044

PERCOLATION/LEAKAGE THROUGH LAYER 9 0.00000 (0.00000) 0.033 0.00000

AVERAGE HEAD ON TOP OF LAYER 8 0.000 (0.000)

CHANGE IN WATER STORAGE -0.000 (-2.0754) -2027.00 -0.020

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	5.36	1345880,800
RUNOFF	0.349	90220,3516

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DRAINAGE COLLECTED FROM LAYER 2	0.16029	58173.95228
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.000027	0.64625
AVERAGE HEAD ON TOP OF LAYER 3	0.002	
MAXIMUM HEAD ON TOP OF LAYER 3	0.004	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	2.0 FEET	
DRAINAGE COLLECTED FROM LAYER 5	0.00001	3.50925
PERCOLATION/LEAKAGE THROUGH LAYER 6	0.000007	2.50565
AVERAGE HEAD ON TOP OF LAYER 6	0.000	
MAXIMUM HEAD ON TOP OF LAYER 6	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 7	0.00001	2.50486
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000000	0.00113
AVERAGE HEAD ON TOP OF LAYER 8	0.000	
MAXIMUM HEAD ON TOP OF LAYER 8	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 7 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	2.31	837825.8128
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.4649
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0778

*** Maximum heads are computed using McEnroe's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner
by Bruce M. McEnroe, University of Kansas
ASCE Journal of Environmental Engineering
Vol. 118, No. 3, March 1993, pp. 362-376.

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL/VOL)
1	171.5269	0.2859
2	0.0020	0.0100
3	0.0000	0.0000
4	1.0000	0.0050
5	0.0020	0.0100
6	0.0000	0.0000
7	0.0020	0.0100
8	0.0000	0.0000
9	15.0344	0.4276
SNOW WATER	0.000	

FINAL COVER

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#
#*****
#
# HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
# HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
# DEVELOPED BY ENVIRONMENTAL LABORATORY
# USAC WATERWAYS EXPERIMENT STATION
# FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
#*****
#
PRECIPITATION DATA FILE: C:\M\C\DATA1.D4
TEMPERATURE DATA FILE: C:\M\C\DATA1.D7
SOLAR RADIATION DATA FILE: C:\M\C\DATA3.D33
EVAPORATION DATA FILE: C:\M\C\DATA3.D33
SOIL AND DESIGN DATA FILE: C:\M\C\DATA10.D10
OUTPUT DATA FILE: C:\M\C\OUT.OUT

TIME: 11:48 DATE: 4/26/2016

-----
TITLE: P3 with cover
-----

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER BEFC
      COMPUTED AS NEARLY STEADY-STATE VALUE BY THE PROGRAM.

-----
LAYER 1
-----
TYPE 1 - VERTICAL PENETRATION LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 36.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0459 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0459 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.00999978000E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
      FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

-----
LAYER 2
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34
THICKNESS = 0.20 INCHES
Page 1

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WILTING POINT = 0.0010 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.000000000E-12 CM/SEC
SLOPE = 7.00 PERCENT
DRAINAGE LENGTH = 50.0 FEET

-----
LAYER 7
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 33
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999000E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 3 - GOOD
FML PLACEMENT QUALITY = 3 - GOOD

-----
LAYER 8
-----
TYPE 1 - VERTICAL PENETRATION LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 24.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0459 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0459 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC

-----
LAYER 9
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34
THICKNESS = 0.20 INCHES
POROSITY = 0.8100 VOL/VOL
FIELD CAPACITY = 0.0010 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0180 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.000000000E-12 CM/SEC
SLOPE = 7.00 PERCENT
DRAINAGE LENGTH = 50.0 FEET

-----
LAYER 10
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 33
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
Page 5

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POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0200 VOL/VOL
WILTING POINT = 0.0450 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0216 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.000000000E-12 CM/SEC
SLOPE = 0.10 PERCENT
DRAINAGE LENGTH = 50.0 FEET

-----
LAYER 3
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 33
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999000E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00
FML PLACEMENT QUALITY = 3 - GOOD

-----
LAYER 4
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 17
THICKNESS = 0.20 INCHES
POROSITY = 0.7500 VOL/VOL
FIELD CAPACITY = 0.7400 VOL/VOL
WILTING POINT = 0.8000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.7500 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 3.00000000000E-09 CM/SEC

-----
LAYER 5
-----
TYPE 1 - VERTICAL PENETRATION LAYER
MATERIAL TEXTURE NUMBER 18
THICKNESS = 600.00 INCHES
POROSITY = 0.6710 VOL/VOL
FIELD CAPACITY = 0.7400 VOL/VOL
WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2920 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

-----
LAYER 6
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34
THICKNESS = 0.20 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
Page 2

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WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999000E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 3 - GOOD

-----
LAYER 11
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34
THICKNESS = 0.20 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
WILTING POINT = 0.0100 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.000000000E-12 CM/SEC
SLOPE = 7.00 PERCENT
DRAINAGE LENGTH = 50.0 FEET

-----
LAYER 12
-----
TYPE 1 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 33
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999000E-12 CM/SEC
FML PINHOLE DENSITY = 2.00 HOLES/ACRE
FML INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FML PLACEMENT QUALITY = 3 - GOOD

-----
LAYER 13
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 17
THICKNESS = 0.20 INCHES
POROSITY = 0.4210 VOL/VOL
FIELD CAPACITY = 0.4180 VOL/VOL
WILTING POINT = 0.4620 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4210 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-09 CM/SEC

-----
GENERAL DESIGN AND EVAPORATIVE ZONE DATA
NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
      SOIL DATA BASE USING SOIL TEXTURE # 1 WITH A
      FAIR STAND OF GRASS, A SURFACE SLOPE OF 1.8
Page 4

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AND A SLOPE LENGTH OF 50 FEET.

SCE RUNOFF CURVE NUMBER	=	46.40
FRACTION OF AREA ALLOWING RUNOFF	=	100.0 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE	=	100.000 ACRES
EVAPORATIVE ZONE DEPTH	=	21.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE	=	0.194 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE	=	0.174 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	0.164 INCHES
INITIAL SNOW WATER	=	0.000 INCHES
INITIAL WATER IN LAYER MATERIALS	=	194.118 INCHES
TOTAL INITIAL WATER	=	194.118 INCHES
TOTAL SURFACE INFLOW	=	0.00 INCHES/YEAR

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.05	1.36	2.12	3.13	4.91	4.02
STD. DEVIATIONS	0.68	0.85	1.20	1.91	3.02	2.08
	1.76	1.62	1.84	1.93	1.78	0.91
RUNOFF						
TOTALS	0.001	0.012	0.000	0.000	0.010	0.002
STD. DEVIATIONS	0.000	0.004	0.000	0.000	0.004	0.002
EVAPOTRANSPIRATION						
TOTALS	0.780	0.808	1.480	1.982	2.281	2.107
STD. DEVIATIONS	0.437	0.489	0.734	0.843	1.308	1.419
	1.111	1.300	1.008	0.906	0.566	0.499
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	0.3840	0.3458	0.5617	0.8358	1.0638	1.1770
STD. DEVIATIONS	0.8374	0.5808	0.6986	0.9994	0.6794	0.4815
	0.2305	0.1897	0.1162	0.7961	1.1970	0.7865
	0.9709	0.3162	0.6070	0.8874	0.6389	0.3887
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 5						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

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STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 7						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 9						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 10						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 11						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 12						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	0.9037	0.9007	0.9020	0.9016	0.9018	0.9021
STD. DEVIATIONS	0.0026	0.0026	0.0026	0.0026	0.0026	0.0026
	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 10						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

Page 6

AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 12						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.34 (0.245)	11084725.0	100.00
RUNOFF	0.013 (0.0490)	5312.84	0.049
EVAPOTRANSPIRATION	21.091 (1.9916)	7653876.30	69.106
LATERAL DRAINAGE COLLECTED FROM LAYER 2	9.2399 (2.8753)	3351601.750	30.42897
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.0000 (0.0000)	0.899	0.00001
AVERAGE HEAD ON TOP OF LAYER 3	0.902 (0.001)		
LATERAL DRAINAGE COLLECTED FROM LAYER 5	0.0000 (0.0000)	0.857	0.00001
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.0000 (0.0000)	0.042	0.00000
AVERAGE HEAD ON TOP OF LAYER 7	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 9	0.0000 (0.0000)	0.900	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.0000 (0.0000)	0.041	0.00000
AVERAGE HEAD ON TOP OF LAYER 10	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 11	0.0000 (0.0000)	0.001	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 12	0.0000 (0.0000)	0.020	0.00000
AVERAGE HEAD ON TOP OF LAYER 12	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.001 (0.8367)	1802.53	0.017

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PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	5.56	194580.000
RUNOFF	0.243	881.70,7832
DRAINAGE COLLECTED FROM LAYER 2	2.45990	889675.43700
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000001	0.22518
AVERAGE HEAD ON TOP OF LAYER 3	1.118	
MAXIMUM HEAD ON TOP OF LAYER 3	1.138	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	19.7 FEET	
DRAINAGE COLLECTED FROM LAYER 5	0.00000	0.22467
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000	0.00044
AVERAGE HEAD ON TOP OF LAYER 7	0.000	
MAXIMUM HEAD ON TOP OF LAYER 7	0.003	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 9	0.00000	0.00001
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000	0.00045
AVERAGE HEAD ON TOP OF LAYER 10	0.000	
MAXIMUM HEAD ON TOP OF LAYER 10	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 11	0.00000	0.00036
PERCOLATION/LEAKAGE THROUGH LAYER 12	0.000000	0.00009
AVERAGE HEAD ON TOP OF LAYER 12	0.000	
MAXIMUM HEAD ON TOP OF LAYER 12	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 11 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	2.31	837825.8120
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.0203
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0280

*** Maximum heads are computed using McCornea's equations. ***
Reference: Maximum Saturated Depth over Landfill Liner by Bruce M. McCornea, University of Kansas

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 #

FINAL WATER STORAGE AT END OF YEAR 100

YEAR	(INCHES)	(VOL./VOL.)
1	2.3172	0.0644
2	0.0021	0.0105
3	0.0000	0.0000
4	0.1500	0.7300
5	175.2000	0.2920
6	0.0020	0.0100
7	0.0000	0.0000
8	1.0800	0.0450
9	0.0000	0.0100
10	0.0000	0.0000
11	0.0020	0.0100
12	0.0000	0.0000
13	15.9844	0.4270
SNOW WATER	0.000	

*page *

LANDFILL Y

CELL Y-1

INTERMEDIATE COVER

LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.08804 (0.19933)	395.178	0.58286
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.01175 (0.02893)	66.249	0.10066
AVERAGE HEAD ON TOP OF LAYER 3	0.016 (0.048)		
LATERAL DRAINAGE COLLECTED FROM LAYER 4	0.01175 (0.02881)	96.253	0.30069
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.00000 (0.00000)	0.048	0.00001
AVERAGE HEAD ON TOP OF LAYER 5	0.003 (0.007)		
CHANGE IN WATER STORAGE	0.001 (1.2113)	4.89	0.007

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	1.08	8757.439
RUNOFF	0.547	3176.8064
DRAINAGE COLLECTED FROM LAYER 2	0.02888	168.19578
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.002846	16.33049
AVERAGE HEAD ON TOP OF LAYER 3	2.380	
MAXIMUM HEAD ON TOP OF LAYER 3	2.920	
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	21.3 FEET	
DRAINAGE COLLECTED FROM LAYER 4	0.00147	9.22515
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.000000	0.00095
AVERAGE HEAD ON TOP OF LAYER 5	0.123	
MAXIMUM HEAD ON TOP OF LAYER 5	0.223	
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	4.9 FEET	
SNOW WATER	3.10	17978.0039
MAXIMUM VEG. SOIL WATER (VOL./VOL.)		0.2879
MINIMUM VEG. SOIL WATER (VOL./VOL.)		0.0370

*** Maximum heads are computed using McEnroe's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner
 by Bruce M. McEnroe, University of Kansas
 ASCE Journal of Environmental Engineering
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Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL./VOL.)
1	187.3020	0.2838
2	0.5400	0.0450
3	0.0000	0.0000
4	0.5461	0.0450
5	0.0000	0.0000
6	15.3720	0.4270
SNOW WATER	0.000	

FINAL COVER

STD. DEVIATIONS	0.3997	0.3643	0.3736	0.4477	0.4310	0.3970
	0.2297	0.2028	0.1975	0.4028	0.3345	0.3343
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 7						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 8						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 10						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	0.0009	0.0022	0.0012	0.0042	0.0036	0.0022
	0.0013	0.0012	0.0018	0.0028	0.0046	0.0034
STD. DEVIATIONS	0.0017	0.0027	0.0025	0.0029	0.0029	0.0018
	0.0010	0.0013	0.0013	0.0017	0.0024	0.0023
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 9						

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AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES		CU. FEET	PERCENT		
PRECIPITATION	11.68	(2.092)	423866.0	100.00		
RUNOFF	6.176	(0.2638)	63937.73	15.368		
EVAPOTRANSPIRATION	3.699	(0.8479)	1242197.37	31.672		
LATERAL DRAINAGE COLLECTED FROM LAYER 2	7.80342	(1.53341)	7832640.100	86.8281		
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000	(0.00000)		0.817	0.0000	
AVERAGE HEAD ON TOP OF LAYER 3	0.003	(0.001)				
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00000	(0.00000)		0.004	0.0000	
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000	(0.00000)		0.813	0.0000	
AVERAGE HEAD ON TOP OF LAYER 7	0.000	(0.000)				
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.00000	(0.00000)		0.058	0.0000	
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00000	(0.00000)		0.755	0.0000	
AVERAGE HEAD ON TOP OF LAYER 9	0.000	(0.000)				
CHANGE IN WATER STORAGE	-0.001	(0.5099)	-197.19	-0.005		

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	1.68	606840.000
RUNOFF	0.672	235919.8870
DRAINAGE COLLECTED FROM LAYER 2	1.83384	665683.02500
PERCOLATION/LEAKAGE THROUGH LAYER 4		0.00000
AVERAGE HEAD ON TOP OF LAYER 3	0.046	

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MAXIMUM HEAD ON TOP OF LAYER 3	0.892
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	3.0 FEET
DRAINAGE COLLECTED FROM LAYER 6	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000
AVERAGE HEAD ON TOP OF LAYER 7	0.000
MAXIMUM HEAD ON TOP OF LAYER 7	0.000
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET
DRAINAGE COLLECTED FROM LAYER 8	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000
AVERAGE HEAD ON TOP OF LAYER 9	0.000
MAXIMUM HEAD ON TOP OF LAYER 9	0.000
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET
SNOW WATER	3.10 1123625.2500
MAXIMUM VEG. SOIL WATER (VOL./VOL.)	0.1807
MINIMUM VEG. SOIL WATER (VOL./VOL.)	0.0129
NOTE: Maximum heads are computed using McInroe's equations.	
Reference: Maximum saturated depth over Landfill liner by Bruce M. McInroe, University of Kansas ASCE Journal of Environmental Engineering Vol. 119, No. 2, March 1993, pp. 260-270.	
FINAL WATER STORAGE AT END OF YEAR 100	
LAYER	(INCHES) (VOL./VOL.)
1	1.2849 0.0428
2	0.0108 0.0433
3	0.0000 0.0000
4	0.1673 0.7500
5	192.7200 0.2920
6	0.5400 0.0450
7	0.0000 0.0000
8	0.5400 0.0450

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9	0.0000	0.0000
10	13.3720	0.4270
SNOW WATER	0.000	

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CELLS Y-2 AND Y-3

INTERMEDIATE COVER

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#
#*****
#
# HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
# HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
# DEVELOPED BY ENVIRONMENTAL LABORATORY
# USEA WATERWAYS EXPERIMENT STATION
# FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
#*****
#
PRECIPITATION DATA FILE: C:\0V2NC\DATA1.D4
TEMPERATURE DATA FILE: C:\0V2NC\DATA7.D7
SOLAR RADIATION DATA FILE: C:\0V2NC\DATA11.D13
EVAPOTRANSPIRATION DATA: C:\0V2NC\DATA10.D10
SOIL AND DESIGN DATA FILE: C:\0V2NC\DATA10.D10
OUTPUT DATA FILE: C:\0V2NC\OUT\OUT

TIME: 09: 3 DATE: 8/27/2019

-----
TITLE: r3-without cover
#*****

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE
      COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

      LAYER 1
      TYPE 1 - VERTICAL PERCOLATION LAYER
      MATERIAL TEXTURE NUMBER 18
      THICKNESS = 660.00 INCHES
      POROSITY = 0.3710 VOL/VOL
      FIELD CAPACITY = 0.2920 VOL/VOL
      WILTING POINT = 0.0770 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.3957 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 0.10000005000E-02 CM/SEC
      NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.443
            FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

      LAYER 2
      TYPE 3 - LATERAL DRAINAGE LAYER
      MATERIAL TEXTURE NUMBER 1
      Page 1
  
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      TYPE 3 - BARRIER SOIL LAYER
      MATERIAL TEXTURE NUMBER 10
      THICKNESS = 36.00 INCHES
      POROSITY = 0.4270 VOL/VOL
      FIELD CAPACITY = 0.4150 VOL/VOL
      WILTING POINT = 0.3670 VOL/VOL
      INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
      EFFECTIVE SAT. HYD. COND. = 0.10000005000E-02 CM/SEC

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
      SOIL DATA BASE USING SOIL TEXTURE #18 WITH A
      PAIR SLOPE OF 0.4355, A SURFACE SLOPE OF 4.3%
      AND A SLOPE LENGTH OF 30. FEET.

SCS RUNOFF CURVE NUMBER = 62.30
FRACTION OF AREA ALLOWED RUNOFF = 100.00 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE = 1.910 ACRES
EVAPORATIVE ZONE DEPTH = 31.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 3.843 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 21.472 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE = 2.464 INCHES
INITIAL SNOW WATER = 0.000 INCHES
INITIAL WATER IN LAYER MATERIALS = 703.670 INCHES
TOTAL INITIAL WATER = 703.670 INCHES
TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

-----
AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100

PRECIPITATION
TOTALS
STD. DEVIATIONS
RUNOFF
TOTALS
STD. DEVIATIONS
EVAPOTRANSPIRATION
TOTALS
STD. DEVIATIONS
LATERAL DRAINAGE COLLECTED FROM LAYER 2
Page 3
  
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THICKNESS = 12.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4150 VOL/VOL
WILTING POINT = 0.3670 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4150 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.99999997800E-02 CM/SEC
SLOPE = 1.00 PERCENT
DRAINAGE LENGTH = 30.0 FEET

LAYER 3
TYPE 4 - FLEXIBLE MEMBRANE LAYER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999600E-12 CM/SEC
PML PIMPLE DENSITY = 1.00 HOLES/ACRE
PML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
PML PLACEMENT QUALITY = 5 GOOD

LAYER 4
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.4150 VOL/VOL
WILTING POINT = 0.3670 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4150 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.99999997800E-02 CM/SEC
SLOPE = 1.00 PERCENT
DRAINAGE LENGTH = 30.0 FEET

LAYER 5
TYPE 4 - FLEXIBLE MEMBRANE LAYER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999999600E-12 CM/SEC
PML PIMPLE DENSITY = 1.00 HOLES/ACRE
PML INSTALLATION DEFECTS = 1.00 HOLES/ACRE
PML PLACEMENT QUALITY = 5 GOOD

LAYER 6
Page 2
  
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TOTALS
STD. DEVIATIONS
PERCOLATION/LEAKAGE THROUGH LAYER 3
TOTALS
STD. DEVIATIONS
LATERAL DRAINAGE COLLECTED FROM LAYER 4
TOTALS
STD. DEVIATIONS
PERCOLATION/LEAKAGE THROUGH LAYER 6
TOTALS
STD. DEVIATIONS

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AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3
AVERAGES
STD. DEVIATIONS
DAILY AVERAGE HEAD ON TOP OF LAYER 5
AVERAGES
STD. DEVIATIONS

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AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

PRECIPITATION
RUNOFF
EVAPOTRANSPIRATION
Page 4
  
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LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.08804 (0.19933)	473.744	0.58286
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.01175 (0.02891)	51.491	0.10066
AVERAGE HEAD ON TOP OF LAYER 3	0.016 (0.048)		
LATERAL DRAINAGE COLLECTED FROM LAYER 4	0.01175 (0.02881)	81.477	0.30069
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.00000 (0.00000)	0.022	0.00001
AVERAGE HEAD ON TOP OF LAYER 5	0.009 (0.009)		
CHANGE IN WATER STORAGE	0.001 (1.2113)	0.84	0.007

	PEAK DAILY VALUES FOR YEARS 1 THROUGH 100	
	(INCHES)	(CU. FT.)
PRECIPITATION	1.08	11947.943
RUNOFF	0.547	3792.3125
DRAINAGE COLLECTED FROM LAYER 2	0.02888	209.90379
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.002846	15.79282
AVERAGE HEAD ON TOP OF LAYER 3	2.380	
MAXIMUM HEAD ON TOP OF LAYER 3	2.920	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	21.3 FEET	
DRAINAGE COLLECTED FROM LAYER 4	0.00347	9.81861
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.000000	0.00109
AVERAGE HEAD ON TOP OF LAYER 5	0.123	
MAXIMUM HEAD ON TOP OF LAYER 5	0.223	
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	4.9 FEET	
SNOW WATER	3.10	21461.2422
MAXIMUM VEG. SOIL WATER (VOL./VOL.)		0.2879
MINIMUM VEG. SOIL WATER (VOL./VOL.)		0.0370

*** Maximum heads are computed using McEnroe's equations. ***

Reference: Maximum Saturated Depth over Landfill Liner
 by Bruce M. McEnroe, University of Kansas
 ASCE Journal of Environmental Engineering
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Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL./VOL.)
1	187.3020	0.2838
2	0.5400	0.0450
3	0.0000	0.0000
4	0.5461	0.0450
5	0.0000	0.0000
6	15.3720	0.4270
SNOW WATER	0.000	

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FINAL COVER

STD. DEVIATIONS	0.3997	0.3643	0.3736	0.4477	0.4310	0.3970
	0.2297	0.2028	0.1975	0.4028	0.3345	0.3343
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 7						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 8						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 10						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	0.0009	0.0022	0.0012	0.0042	0.0036	0.0022
	0.0013	0.0012	0.0018	0.0028	0.0046	0.0034
STD. DEVIATIONS	0.0017	0.0027	0.0025	0.0029	0.0029	0.0018
	0.0010	0.0013	0.0013	0.0017	0.0024	0.0023
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 9						

AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES		CU. FEET	PERCENT		
PRECIPITATION	11.68	(2.092)	4238966.0	100.00		
RUNOFF	6.178	(0.2938)	63937.73	1.508		
EVAPOTRANSPIRATION	3.699	(0.8479)	1242197.37	31.672		
LATERAL DRAINAGE COLLECTED FROM LAYER 2	7.80342	(1.53341)	7832640.100	96.82881		
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000	(0.00000)		0.817	0.00000	
AVERAGE HEAD ON TOP OF LAYER 3	0.003	(0.001)				
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00000	(0.00000)		0.004	0.00000	
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000	(0.00000)		0.813	0.00000	
AVERAGE HEAD ON TOP OF LAYER 7	0.000	(0.000)				
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.00000	(0.00000)		0.058	0.00000	
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00000	(0.00000)		0.755	0.00000	
AVERAGE HEAD ON TOP OF LAYER 9	0.000	(0.000)				
CHANGE IN WATER STORAGE	-0.001	(0.5099)	-197.19	-0.005		

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)		(CU. FT.)
PRECIPITATION	1.68		606840.000
RUNOFF	0.672		235919.8870
DRAINAGE COLLECTED FROM LAYER 2	1.83384		665683.02500
PERCOLATION/LEAKAGE THROUGH LAYER 4			0.00000
AVERAGE HEAD ON TOP OF LAYER 3	0.046		

MAXIMUM HEAD ON TOP OF LAYER 3	0.892
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	3.0 FEET
DRAINAGE COLLECTED FROM LAYER 6	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000
AVERAGE HEAD ON TOP OF LAYER 7	0.000
MAXIMUM HEAD ON TOP OF LAYER 7	0.000
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET
DRAINAGE COLLECTED FROM LAYER 8	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000
AVERAGE HEAD ON TOP OF LAYER 9	0.000
MAXIMUM HEAD ON TOP OF LAYER 9	0.000
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET
SNOW WATER	3.10 1123625.2500
MAXIMUM VEG. SOIL WATER (VOL./VOL.)	0.1807
MINIMUM VEG. SOIL WATER (VOL./VOL.)	0.0129
NOTE: Maximum heads are computed using McInroe's equations.	
Reference: Maximum saturated depth over Landfill liner by Bruce M. McInroe, University of Kansas ASCE Journal of Environmental Engineering Vol. 119, No. 2, March 1993, pp. 260-270.	
FINAL WATER STORAGE AT END OF YEAR 100	
LAYER	(INCHES) (VOL./VOL.)
1	1.2849 0.0428
2	0.0108 0.0433
3	0.0000 0.0000
4	0.1873 0.7500
5	192.7200 0.2920
6	0.5400 0.0450
7	0.0000 0.0000
8	0.5400 0.0450

9	0.0000	0.0000
10	13.3720	0.4270
SNOW WATER	0.000	

30-YEAR DURATION

DRAINAGE COLLECTED FROM LAYER 4	0.0000	2.246	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.468	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
CHANGE IN WATER STORAGE	1.499	544101.562	14.45
SOIL WATER AT START OF YEAR	205.414	7383992.000	
SOIL WATER AT END OF YEAR	205.745	73959264.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	1.169	424227.944	11.27
ANNUAL WATER BUDGET BALANCE	0.0000	-4.819	0.00

ANNUAL TOTALS FOR YEAR 4			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.09	4021670.750	100.00
RUNOFF	0.999	352484.344	9.00
EVAPOTRANSPIRATION	11.185	4659512.000	109.84
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.563	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000004	1.391	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	-1.119	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.272	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	-1.092	-396931.094	-9.85
SOIL WATER AT START OF YEAR	205.745	73959264.000	
SOIL WATER AT END OF YEAR	204.747	73960112.000	
SNOW WATER AT START OF YEAR	1.169	424227.944	10.54
SNOW WATER AT END OF YEAR	0.075	27054.846	0.67
ANNUAL WATER BUDGET BALANCE	0.0000	3.583	0.00

ANNUAL TOTALS FOR YEAR 5			
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	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.81	1924031.350	100.00
RUNOFF	0.079	28726.516	0.75
EVAPOTRANSPIRATION	9.864	3390331.500	91.25
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.002	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000000	0.012	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	0.001	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000000	0.020	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	0.867	314750.969	8.00
SOIL WATER AT START OF YEAR	204.747	73960112.000	
SOIL WATER AT END OF YEAR	204.595	74268136.000	
SNOW WATER AT START OF YEAR	0.075	27054.846	0.69
SNOW WATER AT END OF YEAR	0.093	33779.789	0.86
ANNUAL WATER BUDGET BALANCE	0.0000	2.238	0.00

ANNUAL TOTALS FOR YEAR 6			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	17.72	4617960.000	100.00
RUNOFF	0.154	55743.414	1.21
EVAPOTRANSPIRATION	11.271	4817264.500	104.33
DRAINAGE COLLECTED FROM LAYER 2	0.0943	34246.254	0.74
PERC./LEAKAGE THROUGH LAYER 3	0.022457	8151.960	0.19
AVG. HEAD ON TOP OF LAYER 3	0.0237		
DRAINAGE COLLECTED FROM LAYER 4	0.0243	8125.165	0.19
PERC./LEAKAGE THROUGH LAYER 5	0.000005	1.743	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0054		
CHANGE IN WATER STORAGE	-0.823	-298020.468	-8.45
SOIL WATER AT START OF YEAR	204.595	74268136.000	
SOIL WATER AT END OF YEAR	203.867	74405896.000	

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SNOW WATER AT START OF YEAR	0.093	33779.789	0.73
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.265	0.00

ANNUAL TOTALS FOR YEAR 7			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.14	4759871.000	100.00
RUNOFF	0.000	11632.699	0.45
EVAPOTRANSPIRATION	10.359	4486355.500	94.06
DRAINAGE COLLECTED FROM LAYER 2	0.0000	12.541	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000043	15.519	0.33
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0001	39.005	0.06
PERC./LEAKAGE THROUGH LAYER 5	0.000002	0.823	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	0.771	261781.466	5.49
SOIL WATER AT START OF YEAR	204.589	74003896.000	
SOIL WATER AT END OF YEAR	204.589	74265672.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.810	0.00

ANNUAL TOTALS FOR YEAR 8			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.65	4228951.000	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	11.917	4321939.500	102.29
DRAINAGE COLLECTED FROM LAYER 2	0.0494	37913.570	0.42

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PERC./LEAKAGE THROUGH LAYER 3	0.011978	1075.911	0.12
AVG. HEAD ON TOP OF LAYER 3	0.0119		
DRAINAGE COLLECTED FROM LAYER 4	0.0189	1057.207	0.12
PERC./LEAKAGE THROUGH LAYER 5	0.000004	1.545	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0093		
CHANGE IN WATER STORAGE	-0.130	-119962.475	-2.84
SOIL WATER AT START OF YEAR	204.589	74265672.000	
SOIL WATER AT END OF YEAR	204.238	74145712.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	4.819	0.00

ANNUAL TOTALS FOR YEAR 9			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.89	4243870.500	100.00
RUNOFF	0.421	225447.582	5.32
EVAPOTRANSPIRATION	12.103	4393121.000	103.54
DRAINAGE COLLECTED FROM LAYER 2	0.0268	978.867	0.23
PERC./LEAKAGE THROUGH LAYER 3	0.005118	2946.788	0.07
AVG. HEAD ON TOP OF LAYER 3	0.0053		
DRAINAGE COLLECTED FROM LAYER 4	0.0081	3952.831	0.09
PERC./LEAKAGE THROUGH LAYER 5	0.000004	1.316	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0020		
CHANGE IN WATER STORAGE	-1.070	-388279.719	-9.13
SOIL WATER AT START OF YEAR	204.238	74145712.000	
SOIL WATER AT END OF YEAR	203.189	73757432.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-1.937	0.00

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ANNUAL TOTALS FOR YEAR 10

	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.07	4744410.500	100.00
RUNOFF	0.085	31009.810	0.65
EVAPOTRANSPIRATION	0.873	3581730.000	75.54
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.001	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000000	0.015	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	12.621	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.422	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	5.112	1119847.110	23.81
SOIL WATER AT START OF YEAR	205.189	73757432.000	
SOIL WATER AT END OF YEAR	205.169	74470304.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	1.132	410775.781	8.66
ANNUAL WATER BUDGET BALANCE	0.0000	4.520	0.00

ANNUAL TOTALS FOR YEAR 11

	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.79	4275771.000	100.00
RUNOFF	0.416	150909.594	3.53
EVAPOTRANSPIRATION	12.844	4962245.000	108.84
DRAINAGE COLLECTED FROM LAYER 2	0.2920	106696.281	2.49
PERC./LEAKAGE THROUGH LAYER 3	0.05044	20670.934	0.48
AVG. HEAD ON TOP OF LAYER 3	0.0708		
DRAINAGE COLLECTED FROM LAYER 4	0.0567	20587.648	0.48
PERC./LEAKAGE THROUGH LAYER 5	0.000009	3.181	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0238		
CHANGE IN WATER STORAGE	-1.870	-660661.050	-15.44

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SOIL WATER AT START OF YEAR	205.169	74470304.000	
SOIL WATER AT END OF YEAR	204.369	74186080.000	
SNOW WATER AT START OF YEAR	1.132	410775.781	9.80
SNOW WATER AT END OF YEAR	0.111	40334.402	0.94
ANNUAL WATER BUDGET BALANCE	0.0000	-5.346	0.00

ANNUAL TOTALS FOR YEAR 12

	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.87	3945811.000	100.00
RUNOFF	0.118	42802.867	1.08
EVAPOTRANSPIRATION	10.939	3870880.500	100.64
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.487	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000003	1.225	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0002	80.549	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000002	0.378	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0001		
CHANGE IN WATER STORAGE	-0.187	-87962.641	-1.72
SOIL WATER AT START OF YEAR	204.369	74186080.000	
SOIL WATER AT END OF YEAR	204.282	74158456.000	
SNOW WATER AT START OF YEAR	0.111	40334.402	1.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	8.474	0.00

ANNUAL TOTALS FOR YEAR 13

	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.94	3608220.500	100.00
RUNOFF	0.090	32889.732	0.91

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EVAPOTRANSPIRATION	10.408	3776891.250	104.86
DRAINAGE COLLECTED FROM LAYER 2	0.0002	84.191	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000132	51.292	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0002	54.610	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.332	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	-0.334	-200986.000	-3.37
SOIL WATER AT START OF YEAR	204.293	74158456.000	
SOIL WATER AT END OF YEAR	203.740	73957864.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-2.173	0.00

ANNUAL TOTALS FOR YEAR 14

	INCHES	CU. FEET	PERCENT
PRECIPITATION	7.60	2758800.750	100.00
RUNOFF	0.126	45904.789	1.66
EVAPOTRANSPIRATION	7.857	2852091.250	103.38
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.133	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000001	0.454	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	0.324	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.241	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	0.383	-139599.109	-5.05
SOIL WATER AT START OF YEAR	205.740	73957464.000	
SOIL WATER AT END OF YEAR	205.356	73836264.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-3.134	0.00

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ANNUAL TOTALS FOR YEAR 15

	INCHES	CU. FEET	PERCENT
PRECIPITATION	21.59	4207170.500	100.00
RUNOFF	0.095	34380.339	0.82
EVAPOTRANSPIRATION	9.928	3240857.000	77.03
DRAINAGE COLLECTED FROM LAYER 2	0.0000	5.398	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000022	7.909	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	7.074	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.243	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	2.567	931744.312	22.15
SOIL WATER AT START OF YEAR	203.356	73828264.000	
SOIL WATER AT END OF YEAR	205.384	74558104.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.539	195707.687	4.65
ANNUAL WATER BUDGET BALANCE	0.0000	-8.837	0.00

ANNUAL TOTALS FOR YEAR 16

	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.93	3967590.750	100.00
RUNOFF	0.038	13731.185	0.35
EVAPOTRANSPIRATION	11.661	4232905.000	106.69
DRAINAGE COLLECTED FROM LAYER 2	0.0000	1.420	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000008	2.785	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	2.831	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.432	0.00

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AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	-0.769	-279057.856	-7.03
SOIL WATER AT START OF YEAR	205.154	7455494.000	
SOIL WATER AT END OF YEAR	205.154	74470912.000	
SNOW WATER AT START OF YEAR	0.000	194707.987	4.93
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	5.560	0.00

ANNUAL TOTALS FOR YEAR 17			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.70	3907881.000	100.00
RUNOFF	0.508	206012.922	5.37
EVAPOTRANSPIRATION	10.605	3849732.250	98.56
DRAINAGE COLLECTED FROM LAYER 2	0.0665	24335.768	0.62
PERC./LEAKAGE THROUGH LAYER 3	0.016553	6009.398	0.15
AVG. HEAD ON TOP OF LAYER 3	0.0160		
DRAINAGE COLLECTED FROM LAYER 4	0.0285	1992.232	0.13
PERC./LEAKAGE THROUGH LAYER 5	0.000004	1.510	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0040		
CHANGE IN WATER STORAGE	-0.490	-179966.953	-4.61
SOIL WATER AT START OF YEAR	205.154	74470912.000	
SOIL WATER AT END OF YEAR	204.658	74290912.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-2.550	0.00

ANNUAL TOTALS FOR YEAR 18			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.00	3267000.130	100.00
RUNOFF	0.026	9268.788	0.28
EVAPOTRANSPIRATION	9.843	3572275.500	109.34
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.804	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000005	1.807	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	1.506	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000005	0.801	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	-0.867	-314545.214	-9.83
SOIL WATER AT START OF YEAR	204.470	74224920.000	
SOIL WATER AT END OF YEAR	203.865	74005128.000	
SNOW WATER AT START OF YEAR	0.256	92752.898	2.94

PRECIPITATION	10.93	3967590.000	100.00
RUNOFF	0.118	42763.738	1.08
EVAPOTRANSPIRATION	10.739	3898088.250	98.23
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	15.326	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000001	0.431	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	0.074	26717.652	0.67
SOIL WATER AT START OF YEAR	204.658	74290912.000	
SOIL WATER AT END OF YEAR	204.476	74244920.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.259	92752.898	2.84
ANNUAL WATER BUDGET BALANCE	0.0000	4.705	0.00

ANNUAL TOTALS FOR YEAR 19

	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.00	3267000.130	100.00
RUNOFF	0.026	9268.788	0.28
EVAPOTRANSPIRATION	9.843	3572275.500	109.34
DRAINAGE COLLECTED FROM LAYER 2	0.0000	0.804	0.00
PERC./LEAKAGE THROUGH LAYER 3	0.000005	1.807	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
DRAINAGE COLLECTED FROM LAYER 4	0.0000	1.506	0.00
PERC./LEAKAGE THROUGH LAYER 5	0.000005	0.801	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	-0.867	-314545.214	-9.83
SOIL WATER AT START OF YEAR	204.470	74224920.000	
SOIL WATER AT END OF YEAR	203.865	74005128.000	
SNOW WATER AT START OF YEAR	0.256	92752.898	2.94

SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-3.490	0.00

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 19							
	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	SUM/DEC	
PRECIPITATION							
TOTALS	1.80	1.20	0.88	0.95	0.95	0.94	0.918
STD. DEVIATIONS	0.19	0.49	0.48	0.64	1.25	1.35	
RUNOFF							
TOTALS	0.083	0.136	0.012	0.000	0.000	0.000	0.000
STD. DEVIATIONS	0.170	0.204	0.029	0.000	0.000	0.000	0.000
EVAPOTRANSPIRATION							
TOTALS	0.479	0.534	1.494	1.692	1.229	2.454	0.985
STD. DEVIATIONS	0.272	0.465	0.792	0.610	0.596	0.585	
LATERAL DRAINAGE COLLECTED FROM LAYER 2							
TOTALS	0.0000	0.0000	0.0030	0.0067	0.0082	0.0094	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0010	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 3							
TOTALS	0.0000	0.0000	0.0005	0.0014	0.0038	0.0014	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0001	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 4							
TOTALS	0.0000	0.0000	0.0001	0.0004	0.0034	0.0034	0.0030
STD. DEVIATIONS	0.0000	0.0000	0.0001	0.0001	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 5							
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
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AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)							
DAILY AVERAGE HEAD ON TOP OF LAYER 3							
AVERAGES	0.0000	0.0000	0.0085	0.0198	0.0224	0.0187	0.0187
STD. DEVIATIONS	0.0075	0.0027	0.0068	0.0061	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0389	0.0350	0.0473	0.0443	0.0443
	0.0039	0.0057	0.0036	0.0004	0.0001	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 5							
AVERAGES	0.0001	0.0000	0.0001	0.0020	0.0037	0.0043	0.0043
STD. DEVIATIONS	0.0037	0.0021	0.0021	0.0007	0.0002	0.0001	0.0001
STD. DEVIATIONS	0.0001	0.0000	0.0014	0.0068	0.0081	0.0088	0.0088
	0.0074	0.0045	0.0004	0.0012	0.0005	0.0002	0.0002

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 19			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.02	(1.447)	3999314.1
RUNOFF	0.781	(0.2945)	84013.42
EVAPOTRANSPIRATION	10.769	(1.3446)	3909267.23
LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.0296	(0.0699)	10149.312
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.0023	(0.0140)	2262.412
AVERAGE HEAD ON TOP OF LAYER 3	0.007	(0.017)	
LATERAL DRAINAGE COLLECTED FROM LAYER 4	0.0024	(0.0139)	2264.297
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.0000	(0.0000)	0.811
AVERAGE HEAD ON TOP OF LAYER 5	0.001	(0.003)	
CHANGE IN WATER STORAGE	-0.618	(1.2755)	-6382.30

PEAK DAILY VALUES FOR YEARS 1 THROUGH 19		
	(INCHES)	(CU. FT.)

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PRECIPITATION          1.58      608846.000
RUNOFF                 0.540      106579.5470
DRAINAGE COLLECTED FROM LAYER 2      0.00426      149.09363
PERCOLATION/LEAKAGE THROUGH LAYER 3  0.000879      286.55217
AVERAGE HEAD ON TOP OF LAYER 5      0.376
MAXIMUM HEAD ON TOP OF LAYER 3      0.611
LOCATION OF MAXIMUM HEAD IN LAYER 2
(DISTANCE FROM DRAIN)      9.1 FEET
DRAINAGE COLLECTED FROM LAYER 4      0.00039      149.31868
PERCOLATION/LEAKAGE THROUGH LAYER 6  0.000000      0.00000
AVERAGE HEAD ON TOP OF LAYER 5      0.034
MAXIMUM HEAD ON TOP OF LAYER 3      0.065
LOCATION OF MAXIMUM HEAD IN LAYER 4
(DISTANCE FROM DRAIN)      1.0 FEET
SNOW WATER             7.62      999789.3370
MAXIMUM VEG. SOIL WATER (VOL/VOL)    0.2509
MINIMUM VEG. SOIL WATER (VOL/VOL)    0.0770
    
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*** Maximum heads are computed using mcbrine's equations. ***
 Reference: Maximum Saturated Depth over Landfill Liner
 by Bruce M. McBrine, University of Kansas
 ASCE JOURNAL OF Environmental Engineering
 Vol. 119, No. 1, March 1993, pp. 202-210.

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FINAL WATER STORAGE AT END OF YEAR 10
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LAYER	(INCHES)	(VOL/VOL)
1	186.8734	0.2851
2	0.3400	0.0450
3	0.0000	0.0000
4	0.3400	0.0450
5	0.0000	0.0000
6	35.3770	0.4370
SNOW WATER	0.000	

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SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.856	0.00

ANNUAL TOTALS FOR YEAR 2			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	13.50	4537500.500	100.00
RUNOFF	0.910	297119.531	6.55
EVAPOTRANSPIRATION	4.299	1559389.500	34.37
DRAINAGE COLLECTED FROM LAYER 2	7.6734	2785431.250	61.39
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.790	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0028		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.288	-104409.023	-2.30
SOIL WATER AT START OF YEAR	205.378	74552064.000	
SOIL WATER AT END OF YEAR	205.090	74447656.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.542	0.00

ANNUAL TOTALS FOR YEAR 3			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.37	3704310.250	100.00

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RUNOFF	0.041	14761.194	0.39
EVAPOTRANSPIRATION	3.221	1165986.120	31.09
DRAINAGE COLLECTED FROM LAYER 2	5.3848	1947352.120	51.73
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.807	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0020		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.742	692309.187	16.60
SOIL WATER AT START OF YEAR	205.090	74447656.000	
SOIL WATER AT END OF YEAR	205.863	74655756.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	1.169	424227.844	11.27
ANNUAL WATER BUDGET BALANCE	0.0000	1.283	0.00

ANNUAL TOTALS FOR YEAR 4			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.09	4025670.750	100.00
RUNOFF	1.161	421472.125	10.47
EVAPOTRANSPIRATION	4.378	1589300.000	39.48
DRAINAGE COLLECTED FROM LAYER 2	6.8002	2399434.100	59.33
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.805	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0025		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00

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AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.051	-381514.437	-8.48
SOIL WATER AT START OF YEAR	205.863	74655756.000	
SOIL WATER AT END OF YEAR	205.706	74671400.000	
SNOW WATER AT START OF YEAR	1.169	424227.844	10.54
SNOW WATER AT END OF YEAR	0.075	27054.846	0.67
ANNUAL WATER BUDGET BALANCE	0.0000	-1.363	0.00

ANNUAL TOTALS FOR YEAR 5			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.81	3924031.250	100.00
RUNOFF	0.124	49331.922	1.25
EVAPOTRANSPIRATION	4.658	1690916.000	42.06
DRAINAGE COLLECTED FROM LAYER 2	6.3431	2302539.750	58.68
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.791	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0021		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.316	-124539.078	-3.97
SOIL WATER AT START OF YEAR	205.706	74671400.000	
SOIL WATER AT END OF YEAR	205.372	74550336.000	
SNOW WATER AT START OF YEAR	0.075	27054.846	0.67
SNOW WATER AT END OF YEAR	0.093	33779.789	0.86
ANNUAL WATER BUDGET BALANCE	0.0000	2.199	0.00

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ANNUAL TOTALS FOR YEAR 6			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	12.72	4617360.000	100.00
RUNOFF	0.174	53849.484	1.21
EVAPOTRANSPIRATION	4.103	1489544.620	32.26
DRAINAGE COLLECTED FROM LAYER 2	9.9362	3207142.500	69.47
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.841	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0017		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.378	-135574.437	-3.64
SOIL WATER AT START OF YEAR	205.372	74550336.000	
SOIL WATER AT END OF YEAR	205.092	74448336.000	
SNOW WATER AT START OF YEAR	0.093	33779.789	0.78
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-2.710	0.00

ANNUAL TOTALS FOR YEAR 7			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	17.14	4769821.000	100.00
RUNOFF	0.066	23981.318	0.30
EVAPOTRANSPIRATION	4.138	1501959.000	31.49
DRAINAGE COLLECTED FROM LAYER 2	8.3011	3195389.250	67.00
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.805	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0012		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00

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AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.159	48777.406	1.00
SOIL WATER AT START OF YEAR	205.092	7444830.000	
SOIL WATER AT END OF YEAR	205.225	74496616.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	4.038	0.00

ANNUAL TOTALS FOR YEAR 8

	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.65	4228951.000	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	3.266	822665.687	19.45
DRAINAGE COLLECTED FROM LAYER 2	8.9531	324987.750	76.85
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0033		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.431	136297.828	3.20
SOIL WATER AT START OF YEAR	205.225	74496616.000	
SOIL WATER AT END OF YEAR	205.055	74052902.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.087	0.00

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ANNUAL TOTALS FOR YEAR 9

	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.69	4243470.500	100.00
RUNOFF	0.797	289415.406	6.82
EVAPOTRANSPIRATION	3.777	1370959.370	32.31
DRAINAGE COLLECTED FROM LAYER 2	7.7501	2812283.750	66.30
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.775	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0028		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.634	-230197.185	-5.42
SOIL WATER AT START OF YEAR	205.055	74052778.000	
SOIL WATER AT END OF YEAR	205.021	74422778.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.736	0.00

ANNUAL TOTALS FOR YEAR 10

	INCHES	CU. FEET	PERCENT
PRECIPITATION	13.07	4744410.100	100.00
RUNOFF	0.088	37097.275	0.68
EVAPOTRANSPIRATION	3.464	127361.120	2.65
DRAINAGE COLLECTED FROM LAYER 2	8.1215	2948089.500	62.14

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PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.809	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0030		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.796	506059.781	10.48
SOIL WATER AT START OF YEAR	205.021	74422778.000	
SOIL WATER AT END OF YEAR	203.286	74518808.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	1.132	410775.781	8.66
ANNUAL WATER BUDGET BALANCE	0.0000	2.840	0.00

ANNUAL TOTALS FOR YEAR 11

	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.79	4279771.000	100.00
RUNOFF	0.435	157860.141	3.69
EVAPOTRANSPIRATION	4.419	1604037.370	37.48
DRAINAGE COLLECTED FROM LAYER 2	8.9703	2929521.250	68.45
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.827	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0030		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.134	-411651.125	-8.62
SOIL WATER AT START OF YEAR	205.289	74518808.000	

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SOIL WATER AT END OF YEAR	205.172	74477660.000	
SNOW WATER AT START OF YEAR	1.132	410775.781	8.66
SNOW WATER AT END OF YEAR	0.111	40334.402	0.94
ANNUAL WATER BUDGET BALANCE	0.0000	3.289	0.00

ANNUAL TOTALS FOR YEAR 12

	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.87	3945811.000	100.00
RUNOFF	0.114	41563.867	1.05
EVAPOTRANSPIRATION	3.604	1308192.170	33.16
DRAINAGE COLLECTED FROM LAYER 2	7.1959	2684697.750	68.64
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.811	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0027		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.245	-88844.445	-2.23
SOIL WATER AT START OF YEAR	205.172	74477660.000	
SOIL WATER AT END OF YEAR	205.039	74429088.000	
SNOW WATER AT START OF YEAR	0.111	40334.402	1.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	1.977	0.00

ANNUAL TOTALS FOR YEAR 13

	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.94	3608220.500	100.00

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RUNOFF	0.112	40553.973	1.12
EVAPOTRANSPIRATION	4.076	1461300.370	40.50
DRAINAGE COLLECTED FROM LAYER 2	5.8309	2044011.500	56.55
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.766	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0021		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.172	62357.301	1.73
SOIL WATER AT START OF YEAR	205.059	7442988.000	
SOIL WATER AT END OF YEAR	205.211	74491448.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.699	0.00

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ANNUAL TOTALS FOR YEAR 14			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	7.60	2738600.750	100.00
RUNOFF	0.129	46736.363	1.69
EVAPOTRANSPIRATION	3.214	1166800.620	42.25
DRAINAGE COLLECTED FROM LAYER 2	4.8899	1592186.500	57.75
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.767	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0016		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00

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AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.132	-47028.457	-1.74
SOIL WATER AT START OF YEAR	205.211	74491448.000	
SOIL WATER AT END OF YEAR	205.079	74443120.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	5.304	0.00

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ANNUAL TOTALS FOR YEAR 15			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.39	4207170.100	100.00
RUNOFF	0.114	41884.808	0.99
EVAPOTRANSPIRATION	3.227	1182131.060	28.10
DRAINAGE COLLECTED FROM LAYER 2	7.1702	2607284.750	61.87
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.802	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0026		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.048	580420.281	9.04
SOIL WATER AT START OF YEAR	205.079	74443120.000	
SOIL WATER AT END OF YEAR	205.182	74628232.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.139	195707.687	4.61
ANNUAL WATER BUDGET BALANCE	0.0000	-0.018	0.00

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PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.086	-31206.491	-0.80
SOIL WATER AT START OF YEAR	205.115	74156584.000	
SOIL WATER AT END OF YEAR	205.029	74223376.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	1.801	0.00

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ANNUAL TOTALS FOR YEAR 16			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.93	3967590.750	100.00
RUNOFF	0.043	15181.319	0.39
EVAPOTRANSPIRATION	3.384	1228952.870	30.96
DRAINAGE COLLECTED FROM LAYER 2	0.3152	1091007.000	27.50
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.813	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0031		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.052	-367351.966	-9.26
SOIL WATER AT START OF YEAR	205.182	74628232.000	
SOIL WATER AT END OF YEAR	205.113	74416584.000	
SNOW WATER AT START OF YEAR	0.139	191707.687	4.80
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	3.485	0.00

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ANNUAL TOTALS FOR YEAR 17			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.76	3905881.000	100.00
RUNOFF	0.064	219321.906	5.60
EVAPOTRANSPIRATION	3.644	1322710.370	33.86
DRAINAGE COLLECTED FROM LAYER 2	0.5985	2091253.250	53.32
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.853	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0024		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00

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DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.086	-31206.491	-0.80
SOIL WATER AT START OF YEAR	205.115	74156584.000	
SOIL WATER AT END OF YEAR	205.029	74223376.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	1.801	0.00

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ANNUAL TOTALS FOR YEAR 18			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.93	3967590.000	100.00
RUNOFF	0.127	46126.987	1.16
EVAPOTRANSPIRATION	3.011	1310854.370	33.03
DRAINAGE COLLECTED FROM LAYER 2	6.5577	2380456.000	60.00
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.751	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0024		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.035	210395.362	5.31
SOIL WATER AT START OF YEAR	201.029	74259376.000	
SOIL WATER AT END OF YEAR	205.108	74563024.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.256	92752.898	2.34

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ANNUAL WATER BUDGET BALANCE			
	INCHES	CU. FEET	PERCENT
ANNUAL WATER BUDGET BALANCE	0.0000	-2.878	0.00

ANNUAL TOTALS FOR YEAR 19			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	9.00	3257000.250	100.00
RUNOFF	0.077	5906.840	0.18
EVAPOTRANSPIRATION	2.921	1050297.370	32.45
DRAINAGE COLLECTED FROM LAYER 2	4.4033	2924425.250	72.15
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.758	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0024		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.151	-127570.828	-3.90
SOIL WATER AT START OF YEAR	205.408	7456024.000	
SOIL WATER AT END OF YEAR	205.312	7452826.000	
SNOW WATER AT START OF YEAR	0.250	92752.898	2.84
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	3.186	0.00

ANNUAL TOTALS FOR YEAR 20			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	13.60	4916800.500	100.00
RUNOFF	0.056	20227.537	0.41
EVAPOTRANSPIRATION	4.198	1523979.870	30.87
DRAINAGE COLLECTED FROM LAYER 2	6.4667	3437513.750	69.85

PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.957	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0015		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.124	-44920.809	-0.91
SOIL WATER AT START OF YEAR	205.312	7452826.000	
SOIL WATER AT END OF YEAR	205.188	7448336.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.019	0.00

ANNUAL TOTALS FOR YEAR 21			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	10.42	3786090.500	100.00
RUNOFF	0.004	1479.972	0.04
EVAPOTRANSPIRATION	2.757	1000663.810	26.43
DRAINAGE COLLECTED FROM LAYER 2	7.7027	2790081.250	73.85
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.384	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0028		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.078	-12102.185	-0.32
SOIL WATER AT START OF YEAR	205.188	7448336.000	

SOIL WATER AT END OF YEAR	205.155	7447132.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-2.113	0.00

ANNUAL TOTALS FOR YEAR 22			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	17.79	4661270.000	100.00
RUNOFF	0.004	1283.439	0.03
EVAPOTRANSPIRATION	3.425	1243428.000	27.87
DRAINAGE COLLECTED FROM LAYER 2	8.5298	3090324.750	69.40
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.958	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0031		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.331	120231.141	2.69
SOIL WATER AT START OF YEAR	205.155	7447132.000	
SOIL WATER AT END OF YEAR	205.189	74483720.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.297	107746.367	2.82
ANNUAL WATER BUDGET BALANCE	0.0000	2.190	0.00

ANNUAL TOTALS FOR YEAR 23			
	INCHES	CU. FEET	PERCENT

PRECIPITATION	11.76	4276140.000	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	3.938	1211679.750	28.34
DRAINAGE COLLECTED FROM LAYER 2	8.6862	3153073.750	73.74
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.872	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0032		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.244	-86614.025	-0.07
SOIL WATER AT START OF YEAR	205.189	74483720.000	
SOIL WATER AT END OF YEAR	205.055	74434936.000	
SNOW WATER AT START OF YEAR	0.297	107746.367	2.52
SNOW WATER AT END OF YEAR	0.187	67913.789	1.59
ANNUAL WATER BUDGET BALANCE	0.0000	0.498	0.00

ANNUAL TOTALS FOR YEAR 24			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	13.23	4802495.000	100.00
RUNOFF	0.155	16225.254	1.17
EVAPOTRANSPIRATION	4.158	1509450.620	31.43
DRAINAGE COLLECTED FROM LAYER 2	8.7728	3184510.250	66.31
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.824	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0032		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00

PERC./LEAKAGE THROUGH LAYER 10	0.00000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.144	52319.988	1.09
SOIL WATER AT START OF YEAR	205.055	74824936.000	
SOIL WATER AT END OF YEAR	205.886	74533168.000	
SNOW WATER AT START OF YEAR	0.187	67013.799	1.41
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.000	0.00

ANNUAL TOTALS FOR YEAR 25			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.39	4134570.500	100.00
RUNOFF	0.375	133974.062	3.25
EVAPOTRANSPIRATION	4.162	1510924.120	36.34
DRAINAGE COLLECTED FROM LAYER 2	0.6188	2402625.750	58.11
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.756	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0024		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.234	81010.339	2.05
SOIL WATER AT START OF YEAR	205.896	74553256.000	
SOIL WATER AT END OF YEAR	205.247	74504896.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.375	133974.062	3.28
ANNUAL WATER BUDGET BALANCE	0.0000	-0.866	0.00

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PRECIPITATION	11.66	4232580.000	100.00
RUNOFF	0.136	41945.816	0.99
EVAPOTRANSPIRATION	3.692	1346081.000	31.66
DRAINAGE COLLECTED FROM LAYER 2	6.4308	2460273.750	72.31
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.866	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0031		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.578	-209820.703	-4.89
SOIL WATER AT START OF YEAR	205.247	74504896.000	
SOIL WATER AT END OF YEAR	204.642	74480400.000	
SNOW WATER AT START OF YEAR	0.373	133521.359	3.20
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.169	0.00

ANNUAL TOTALS FOR YEAR 27			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	13.52	4607760.500	100.00
RUNOFF	0.000	0.000	0.00
EVAPOTRANSPIRATION	2.642	95202.562	19.54
DRAINAGE COLLECTED FROM LAYER 2	10.7889	3919981.250	79.87
PERC./LEAKAGE THROUGH LAYER 4	0.000003	0.935	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0040		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.000	0.00

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PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.079	28575.393	0.19
SOIL WATER AT START OF YEAR	205.042	74430400.000	
SOIL WATER AT END OF YEAR	205.121	74458976.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-0.692	0.00

ANNUAL TOTALS FOR YEAR 28			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	16.06	5829780.000	100.00
RUNOFF	0.006	2292.627	0.04
EVAPOTRANSPIRATION	4.253	1543786.250	26.48
DRAINAGE COLLECTED FROM LAYER 2	10.1482	3828994.000	65.68
PERC./LEAKAGE THROUGH LAYER 4	0.000002	0.877	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0039		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.253	454701.201	7.80
SOIL WATER AT START OF YEAR	205.121	74458976.000	
SOIL WATER AT END OF YEAR	205.905	74433512.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.460	170165.891	2.92

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ANNUAL WATER BUDGET BALANCE	0.0000	5.168	0.00
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ANNUAL TOTALS FOR YEAR 29			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	12.88	4673440.500	100.00
RUNOFF	0.558	202473.906	4.33
EVAPOTRANSPIRATION	4.527	1643201.120	35.15
DRAINAGE COLLECTED FROM LAYER 2	8.8153	3214474.230	68.75
PERC./LEAKAGE THROUGH LAYER 4	0.000000	0.793	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0032		
DRAINAGE COLLECTED FROM LAYER 5	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	0.000	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.050	-384661.062	-8.23
SOIL WATER AT START OF YEAR	205.905	74433512.000	
SOIL WATER AT END OF YEAR	205.077	74442608.000	
SNOW WATER AT START OF YEAR	0.469	170165.891	3.64
SNOW WATER AT END OF YEAR	0.222	80208.275	1.84
ANNUAL WATER BUDGET BALANCE	0.0000	0.114	0.00

ANNUAL TOTALS FOR YEAR 30			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	8.89	3227070.500	100.00
RUNOFF	0.022	7892.003	0.24
EVAPOTRANSPIRATION	3.769	1373341.500	42.62

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8	0.5400	0.0450
9	0.0000	0.0000
10	15.3720	0.4270
SNOW WATER	0.207	

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LANDFILL M

INTERMEDIATE COVER

PERCOLATION/LEAKAGE THROUGH LAYER 9						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 5						
AVERAGES	0.0001	0.0001	0.0001	0.0001	0.0001	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0001
STD. DEVIATIONS	0.0001	0.0002	0.0002	0.0002	0.0001	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0001

DAILY AVERAGE HEAD ON TOP OF LAYER 8						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	INCHES	CU. FEET	PERCENT
PRECIPITATION	11.19 (1.997)	438894.0	100.00
RUNOFF	0.096 (0.2109)	31317.74	0.687
EVAPOTRANSPIRATION	10.731 (1.3385)	389326.00	85.453
LATERAL DRAINAGE COLLECTED FROM LAYER 4	1.72878 (1.11502)	627475.687	14.2647
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.00000 (0.00000)	0.000	0.00007
AVERAGE HEAD ON TOP OF LAYER 5	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 7	0.00000 (0.00000)	0.000	0.00002
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.00000 (0.00000)	0.000	0.00000
AVERAGE HEAD ON TOP OF LAYER 8	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.012 (1.3364)	4342.97	0.095

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	1.15	417450.000
RUNOFF	1.567	586071.6670
DRAINAGE COLLECTED FROM LAYER 4	0.16126	58137.49310
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000000	0.00010
AVERAGE HEAD ON TOP OF LAYER 5	0.002	
MAXIMUM HEAD ON TOP OF LAYER 5	0.004	
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	2.2 FEET	
DRAINAGE COLLECTED FROM LAYER 7	0.00000	0.00500
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000000	0.00010
AVERAGE HEAD ON TOP OF LAYER 8	0.000	
MAXIMUM HEAD ON TOP OF LAYER 8	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 7 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	2.58	93137.4370
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.2436
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0438

*** Maximum heads are computed using McEnroe's Equation. ***

Reference: Maximum Saturated Depth over Landfill Liner by Bruce M. McEnroe, University of Kansas, ASCE Journal of Environmental Engineering Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100

LAYER	(INCHES)	(VOL/VOL)
1	2.9002	0.1611
2	382.8594	0.2900
5	0.8019	0.0869
4	0.0000	0.0000
5	0.0000	0.0000
6	7.0860	0.4270

7	0.0020	0.0100
8	0.0000	0.0000
9	11.3770	0.4270
SNOW WATER	0.000	

FINAL COVER

```

#
#*****
#
# HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
# HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
# DEVELOPED BY ENVIRONMENTAL LABORATORY
# USAC WATERWAYS EXPERIMENT STATION
# FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
#*****
#
PRECIPITATION DATA FILE: C:\MC\DATA1.D4
TEMPERATURE DATA FILE: C:\MC\DATA7.D7
SOLAR RADIATION DATA FILE: C:\MC\DATA13.DL3
EVAPORATION DATA FILE: C:\MC\DATA10.DL0
SOIL AND DESIGN DATA FILE: C:\MC\DATA10.DL0
OUTPUT DATA FILE: C:\MC\OUT.DOUT

TIME: 12: 7 DATE: 7/22/2016

-----
TITLE: Landfill Infiltration Cover
-----

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER BEING
      COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.

LAYER 1
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 13
THICKNESS = 36.00 INCHES
POROSITY = 0.4750 VOL/VOL
FIELD CAPACITY = 0.2780 VOL/VOL
WILTING POINT = 0.2650 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2289 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.17000000000E-08 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 2.00
      FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

LAYER 2
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 18.00 INCHES
Page 1
    
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FILL PIMMOLE DENSITY = 2.00 HOLES/ACRE
FILL INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FILL PLACEMENT QUALITY = 3 - GOOD

LAYER 3
-----
TYPE 2 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 15
THICKNESS = 18.00 INCHES
POROSITY = 0.5940 VOL/VOL
FIELD CAPACITY = 0.4280 VOL/VOL
WILTING POINT = 0.3670 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-06 CM/SEC

LAYER 4
-----
TYPE 3 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34
THICKNESS = 0.20 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
WILTING POINT = 0.0050 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.0000000000E-08 CM/SEC
DRAINAGE LENGTH = 50.0 FEET

LAYER 5
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-12 CM/SEC
FILL PIMMOLE DENSITY = 2.00 HOLES/ACRE
FILL INSTALLATION DEFECTS = 2.00 HOLES/ACRE
FILL PLACEMENT QUALITY = 3 - GOOD

LAYER 10
-----
TYPE 3 - BARRIER SOIL LAYER
MATERIAL TEXTURE NUMBER 16
THICKNESS = 36.00 INCHES
POROSITY = 0.4270 VOL/VOL
FIELD CAPACITY = 0.2670 VOL/VOL
WILTING POINT = 0.2670 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
Page 5
    
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POROSITY = 0.4370 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
WILTING POINT = 0.0350 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0670 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC

LAYER 3
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 13
THICKNESS = 1320.00 INCHES
POROSITY = 0.6710 VOL/VOL
FIELD CAPACITY = 0.2930 VOL/VOL
WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.1300 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

LAYER 4
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.8170 VOL/VOL
FIELD CAPACITY = 0.0410 VOL/VOL
WILTING POINT = 0.0180 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0319 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.999999978000E-02 CM/SEC

LAYER 5
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34
THICKNESS = 0.30 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0050 VOL/VOL
WILTING POINT = 0.0050 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0100 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.0000000000E-08 CM/SEC
DRAINAGE LENGTH = 50.0 FEET

LAYER 6
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.06 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-12 CM/SEC
Page 2
    
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EFFECTIVE SAT. HYD. COND. = 0.10000000000E-06 CM/SEC

GENERAL DESIGN AND EVAPORATIVE ZONE DATA
-----
NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT
      SOIL DATA BASE USING SOIL TEXTURE #15 WITH A
      FAIR STAND OF GRASS, A SURFACE SLOPE OF 1%
      AND A SLOPE LENGTH OF 50 FEET.

SCS RUNOFF CURVE NUMBER = 91.00
FRACTION OF AREA ALLOWING RUNOFF = 100.00 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE = 100.000 ACRES
EVAPORATIVE ZONE DEPTH = 36.00 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 13.841 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 17.100 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE = 9.440 INCHES
INITIAL SNOW WATER = 0.000 INCHES
INITIAL WATER IN LAYER MATERIALS = 422.108 INCHES
TOTAL INITIAL WATER = 422.108 INCHES
TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

-----
AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100
-----
PRECIPITATION
TOTALS JAN/JUL FEB/AUG MAR/SEP APR/OCT MAY/NOV JUN/DEC
0.92 1.12 1.06 1.08 1.01 0.79
0.92 0.96 0.96 0.96 1.07 0.68

STO. DEVIATIONS
0.70 0.58 0.47 0.52 0.55 0.51
0.81 0.42 0.44 0.62 0.58 0.61

RUNOFF
TOTALS 0.208 0.228 0.042 0.002 0.005 0.000
0.009 0.001 0.000 0.000 0.000 0.000

STO. DEVIATIONS
0.260 0.400 0.152 0.000 0.014 0.000
0.002 0.000 0.000 0.000 0.000 0.000

EVAPORATION
TOTALS 0.840 0.729 1.785 1.384 1.276 2.113
0.875 0.505 0.400 0.471 0.589 0.656

STO. DEVIATIONS
0.134 0.350 0.267 0.267 0.540 0.374
0.586 0.393 0.306 0.295 0.160 0.171

LATERAL DRAINAGE COLLECTED FROM LAYER 1
TOTALS 0.0134 0.0106 0.0810 0.0407 0.0174 0.0063
0.0246 0.0220 0.0189 0.0275 0.0254 0.0341

STO. DEVIATIONS
0.0100 0.0707 0.1627 0.1011 0.0392 0.0491
0.0926 0.0211 0.0196 0.0161 0.0132 0.0311

PERCOLATION/LEAKAGE THROUGH LAYER 1
-----
Page 4
    
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TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 8						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 10						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 5						
AVERAGES	0.0000	0.0000	0.0001	0.0001	0.0001	0.0001
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0001	0.0001
DAILY AVERAGE HEAD ON TOP OF LAYER 9						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES	CU. FEET	PERCENT			
PRECIPITATION	12.56 (1.887)	4558664.0	100.00			
RUNOFF	0.585 (0.6272)	197981.64	4.343			
EVAPOTRANSPIRATION	11.717 (1.4322)	4253096.00	94.297			
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.29436 (0.45884)	106854.381	2.34398			
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000 (0.00000)	0.000	0.00000			
AVERAGE HEAD ON TOP OF LAYER 5	0.000 (0.000)					

LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.00000 (0.00000)	0.075	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00000 (0.00000)	0.028	0.00000
AVERAGE HEAD ON TOP OF LAYER 9	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.002 (1.2019)	751.64	0.016

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
PRECIPITATION	1.15	417450.000
RUNOFF	1.251	817192.8870
DRAINAGE COLLECTED FROM LAYER 8	0.09834	34245.99610
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000	0.00214
AVERAGE HEAD ON TOP OF LAYER 8	0.000	
MAXIMUM HEAD ON TOP OF LAYER 8	0.003	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 8	0.00000	0.00203
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000	0.00011
AVERAGE HEAD ON TOP OF LAYER 9	0.000	
MAXIMUM HEAD ON TOP OF LAYER 9	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 9 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	1.58	93517.4370
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.4249
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.2658

*** Maximum Heads are computed using McCorr's equations. ***
 Reference: Maximum saturated depth over Landfill Liner
 by Bruce H. McCorr, University of Kansas
 ASCE Journal of Environmental Engineering
 Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100

LAYER	(INCHES)	(VOL/VOL)
1	32.0277	0.5341
2	1.0219	0.0570
3	305.4400	0.2920
4	0.7581	0.0652
5	0.0010	0.0101
6	0.0000	0.0000
7	7.6860	0.4270
8	0.0070	0.0100
9	0.0000	0.0000
10	11.3720	0.4270
SNOW WATER	0.000	

LANDFILL D

INTERMEDIATE COVER

AVERAGE(S) OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 4

AVERAGES	0.0001	0.0001	0.0000	0.0002	0.0001	0.0001
STD. DEVIATIONS	0.0001	0.0001	0.0000	0.0000	0.0001	0.0001

DAILY AVERAGE HEAD ON TOP OF LAYER 6

AVERAGES	0.0001	0.0001	0.0000	0.0001	0.0002	0.0001
STD. DEVIATIONS	0.0001	0.0000	0.0000	0.0001	0.0000	0.0001

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100

	INCHES	CU. FEET	PERCENT
PRECIPITATION	36.72 (4.350)	1312900.0	100.00
RUNOFF	3.236 (1.7185)	1174655.37	8.812
EVAPOTRANSPIRATION	26.512 (2.9560)	9623941.00	73.198
LATERAL DRAINAGE COLLECTED FROM LAYER 3	6.77766 (2.01145)	2456897.500	18.44602
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.20695 (0.03887)	751321.352	0.56358
AVERAGE HEAD ON TOP OF LAYER 4	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 1	0.20695 (0.03887)	751321.328	0.56355
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00000 (0.00000)	0.019	0.00000
AVERAGE HEAD ON TOP OF LAYER 6	0.000 (0.000)		
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.00729 (0.01549)	2646.523	0.01985
CHANGE IN WATER STORAGE	-0.051 (1.0081)	-1704.97	-0.049

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100

	(INCHES)	(CU. FT.)
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PRECIPITATION	4.44	1611720.000
RUNOFF	1.620	1317758.2100
DRAINAGE COLLECTED FROM LAYER 3	0.17254	62652.38970
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.003417	877.45713
AVERAGE HEAD ON TOP OF LAYER 4	0.001	
MAXIMUM HEAD ON TOP OF LAYER 4	0.002	
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	2.0 FEET	
DRAINAGE COLLECTED FROM LAYER 1	0.00242	877.45703
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000000	0.00000
AVERAGE HEAD ON TOP OF LAYER 6	0.000	
MAXIMUM HEAD ON TOP OF LAYER 6	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 1 (DISTANCE FROM DRAIN)	0.0 FEET	
PERCOLATION/LEAKAGE THROUGH LAYER 8	0.000553	200.85625
SNOW WATER	5.36	1947431.0200
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.5603
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0770

*** Maximum heads are computed using McCorr's equations. ***
 Reference: Maximum saturated depth over Landfill Liner
 by Bruce H. McCorr, University of Kansas
 ASCE Journal of Environmental Engineering
 vol. 119, No. 1, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100

LAYER	(INCHES)	(VOL/VOL)
1	331.7234	0.2931
2	1.9172	0.1598
3	0.0010	0.0100
4	0.0000	0.0000
5	0.0000	0.0000
6	0.0000	0.0000
7	0.1800	0.7500

8	1.3985	0.2832
SNOW WATER	0.015	

FINAL COVER

EVAPORATIVE ZONE DEPTH	=	20.0	INCHES
INITIAL WATER IN EVAPORATIVE ZONE	=	1,249	INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	0.340	INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	0.760	INCHES
INITIAL SNOW WATER	=	0.000	INCHES
INITIAL WATER IN LAYER MATERIALS	=	362.364	INCHES
TOTAL INITIAL WATER	=	162.584	INCHES
TOTAL SUBSURFACE INFLOW	=	0.00	INCHES/YEAR

AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 100							
	JAN/FEB	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC	
PRECIPITATION							
TOTALS	2.89	2.32	2.65	3.47	3.44	3.20	
STD. DEVIATIONS	1.10	0.89	1.09	1.29	1.58	1.28	
1.68	1.77						
RUNOFF							
TOTALS	0.318	0.811	1.858	0.168	0.080	0.000	
STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000	
0.333	0.811	1.221	0.400	0.000	0.000	0.000	
0.000	0.000	0.000	0.000	0.000	0.000	0.213	
EVAPOTRANSPIRATION							
TOTALS	0.349	0.407	0.398	2.493	1.136	0.837	
STD. DEVIATIONS	1.099	0.956	0.981	0.443	0.366	0.491	
0.100	0.134	0.575	0.723	0.738	0.513	0.113	
0.659	0.739	0.568	0.352	0.169	0.181		
LATERAL DRAINAGE COLLECTED FROM LAYER 2							
TOTALS	0.2939	0.0853	1.9791	2.8601	2.0027	2.4473	
STD. DEVIATIONS	2.7951	2.4298	2.0838	2.1793	1.9664	2.3399	
0.4573	0.2333	1.4011	1.3411	1.1023	0.5083	0.5083	
1.0519	1.0511	1.0028	0.9198	0.7328	0.9063		
PERCOLATION/LEAKAGE THROUGH LAYER 4							
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
LATERAL DRAINAGE COLLECTED FROM LAYER 8							
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
PERCOLATION/LEAKAGE THROUGH LAYER 9							

Page 5

TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 10							
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 12							
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 13							
TOTALS	0.0007	0.0006	0.0007	0.0006	0.0006	0.0006	0.0006
STD. DEVIATIONS	0.0017	0.0014	0.0015	0.0014	0.0013	0.0013	0.0013
0.0017	0.0017	0.0011	0.0011	0.0010	0.0010	0.0010	0.0010

AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)			
DAILY AVERAGE HEAD ON TOP OF LAYER 2			
AVERAGES	0.0001	0.0000	0.0003
STD. DEVIATIONS	0.0003	0.0003	0.0004
0.0003	0.0004	0.0004	0.0004
0.0003	0.0003	0.0003	0.0002
0.0002	0.0002	0.0002	0.0002
DAILY AVERAGE HEAD ON TOP OF LAYER 8			
AVERAGES	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 11			
AVERAGES	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000
0.0000	0.0000	0.0000	0.0000

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100		
INCHES	CU. FEET	PERCENT

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PRECIPITATION	36.72	(4.356)	1332903.0	100.00
RUNOFF	3.189	(1.7079)	1157730.00	8.885
EVAPOTRANSPIRATION	10.416	(1.7401)	3780964.25	28.665
LATERAL DRAINAGE COLLECTED FROM LAYER 2	23.09707	(3.1494)	8984236.000	62.89783
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00000	(0.00000)	1.629	0.00002
AVERAGE HEAD ON TOP OF LAYER 5	0.000	(0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.00000	(0.00000)	0.751	0.00000
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.00000	(0.00000)	-1.377	0.00001
AVERAGE HEAD ON TOP OF LAYER 9	0.000	(0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 10	0.00000	(0.00000)	-1.317	0.00001
PERCOLATION/LEAKAGE THROUGH LAYER 12	0.00000	(0.00000)	0.020	0.00000
AVERAGE HEAD ON TOP OF LAYER 11	0.000	(0.000)		
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.00715	(0.01495)	2597.128	0.01948
CHANGE IN WATER STORAGE	0.032	(1.2126)	4371.39	0.033

PEAK DAILY VALUES FOR YEARS 1 THROUGH 100		
	(INCHES)	(CU. FT.)
PRECIPITATION	4.44	1611730.000
RUNOFF	7.596	1305284.1200
DRAINAGE COLLECTED FROM LAYER 2	5.78134	1881364.75000
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000000	0.01178
AVERAGE HEAD ON TOP OF LAYER 5	0.040	
MAXIMUM HEAD ON TOP OF LAYER 5	0.040	
LOCATION OF MAXIMUM HEAD IN LAYER 5 (DISTANCE FROM DRAIN)	0.1 FEET	
DRAINAGE COLLECTED FROM LAYER 8	0.00000	0.00214
PERCOLATION/LEAKAGE THROUGH LAYER 9	0.000000	0.00792
AVERAGE HEAD ON TOP OF LAYER 9	0.000	

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MAXIMUM HEAD ON TOP OF LAYER 9	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET	
DRAINAGE COLLECTED FROM LAYER 10	0.00000	0.00784
PERCOLATION/LEAKAGE THROUGH LAYER 12	0.000000	0.00006
AVERAGE HEAD ON TOP OF LAYER 11	0.000	
MAXIMUM HEAD ON TOP OF LAYER 11	0.000	
LOCATION OF MAXIMUM HEAD IN LAYER 10 (DISTANCE FROM DRAIN)	0.0 FEET	
PERCOLATION/LEAKAGE THROUGH LAYER 13	0.000498	177.79572
SNOW WATER	5.36	194741.6200
MAXIMUM VOL. SOIL WATER (VOL./VOL.)		0.3428
MINIMUM VOL. SOIL WATER (VOL./VOL.)		0.0180

*** Maximum heads are computed using McEnroe's equations. ***
 Reference: Maximum saturated depth over landfill liner
 by Bruce H. McEnroe, University of Kansas
 ASCE Journal of Environmental Engineering
 Vol. 110, No. 2, March 1989, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL./VOL.)
1	3.4843	0.1452
2	0.0030	0.0100
3	0.0000	0.0000
4	0.1800	0.7500
5	4.5860	0.5780
6	350.2999	0.2920
7	1.5720	0.1310
8	0.0030	0.0100
9	0.0000	0.0000
10	0.0030	0.0100
11	0.0000	0.0000
12	0.1800	0.7500

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13	1.3816	0.2826
SNOW WATER	0.015	

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LANDFILL F

INTERMEDIATE COVER

FROM LAYER 2			
PERCOLATION/LEAKAGE THROUGH LAYER 2	0.2327 (0.06097)	77417.992	0.59230
AVERAGE HEAD ON TOP OF LAYER 3	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.2327 (0.06097)	77417.898	0.59250
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.0000 (0.00000)	0.002	0.00000
AVERAGE HEAD ON TOP OF LAYER 5	0.000 (0.000)		
CHANGE IN WATER STORAGE	0.007 (2.7107)	7430.83	0.019
=====			
PEAK DAILY VALUES FOR YEAR 1 THROUGH 100			
	(INCHES)	(CU. FT.)	
PRECIPITATION	4.85	1764180.000	
RUNOFF	4.412	1601418.5000	
DRAINAGE COLLECTED FROM LAYER 2	0.18017	65402.45700	
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.003450	1266.87085	
AVERAGE HEAD ON TOP OF LAYER 3	0.002		
MAXIMUM HEAD ON TOP OF LAYER 3	0.005		
LOCATION OF MAXIMUM HEAD IN LAYER 3 (DISTANCE FROM DRAIN)	1.3 FEET		
DRAINAGE COLLECTED FROM LAYER 4	0.00349	1268.87061	
PERCOLATION/LEAKAGE THROUGH LAYER 6	0.000000	0.00132	
AVERAGE HEAD ON TOP OF LAYER 5	0.000		
MAXIMUM HEAD ON TOP OF LAYER 5	0.001		
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	0.0 FEET		
SNOW WATER	30.70	3884699.2500	
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.5587	
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0770	
*** Maximum heads are computed using McWhorter's equations. ***			
Reference: Maximum Saturated Depth over Landfill Liner by Bruce M. McWhorter, University of Kansas ASCE Journal of Environmental Engineering Vol. 119, No. 2, March 1993, pp. 262-270.			
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FINAL WATER STORAGE AT END OF YEAR 100		
LAYER	(INCHES)	(VOL./VOL.)
1	174.0037	0.2382
2	0.0020	0.0100
3	0.0000	0.0000
4	0.0020	0.0100
5	0.0000	0.0000
6	15.3720	0.4270
SNOW WATER	3.651	

FINAL COVER

STD. DEVIATIONS	0.0742	0.0000	1.1663	1.0387	0.7115	0.3302
	0.2884	0.3377	0.9086	1.4462	1.1085	0.3696
PERCOLATION/LEAKAGE THROUGH LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 4						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 7						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 8						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 10						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)						
DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	0.0069	0.0000	0.2264	1.1872	0.1759	0.1508
	0.0087	0.0099	0.2173	0.3679	0.4043	0.2923
STD. DEVIATIONS	0.0513	0.0000	0.4492	0.4915	0.2045	0.1574
	0.0823	0.0930	0.2696	0.4261	0.3291	0.2879
DAILY AVERAGE HEAD ON TOP OF LAYER 7						
AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
DAILY AVERAGE HEAD ON TOP OF LAYER 9						

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AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 100						
	INCHES		CU. FEET	PERCENT		
PRECIPITATION	36.08	(4.859)	8307055.0	100.00		
RUNOFF	4.994	(2.4105)	11812754.62	13.859		
EVAPOTRANSPIRATION	20.330	(2.2028)	7307047.00	15.904		
LATERAL DRAINAGE COLLECTED FROM LAYER 2	10.86420	(2.80777)	3943703.000	10.1721		
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00004	(0.00001)	21.896	0.0001		
AVERAGE HEAD ON TOP OF LAYER 3	0.283	(0.098)				
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.00007	(0.00001)	8.427	0.00006		
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00004	(0.00001)	13.265	0.00011		
AVERAGE HEAD ON TOP OF LAYER 7	0.000	(0.000)				
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.00004	(0.00001)	13.234	0.00010		
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00000	(0.00000)	0.033	0.00000		
AVERAGE HEAD ON TOP OF LAYER 9	0.000	(0.000)				
CHANGE IN WATER STORAGE	0.020	(1.9579)	7124.30	0.055		
PEAK DAILY VALUES FOR YEARS 1 THROUGH 100						
	(INCHES)		(CU. FT.)			
PRECIPITATION	4.86		1764180.000			
RUNOFF	4.507		1096223.0000			
DRAINAGE COLLECTED FROM LAYER 2	1.48375		538022.00000			
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000010		2.80175			
AVERAGE HEAD ON TOP OF LAYER 3	11.135					

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MAXIMUM HEAD ON TOP OF LAYER 3	17.141
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	16.5 FEET
DRAINAGE COLLECTED FROM LAYER 6	0.00001 2.47269
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000004 1.35270
AVERAGE HEAD ON TOP OF LAYER 7	0.000
MAXIMUM HEAD ON TOP OF LAYER 7	0.001
LOCATION OF MAXIMUM HEAD IN LAYER 6 (DISTANCE FROM DRAIN)	0.0 FEET
DRAINAGE COLLECTED FROM LAYER 8	0.00000 1.33265
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000 0.00011
AVERAGE HEAD ON TOP OF LAYER 9	0.000
MAXIMUM HEAD ON TOP OF LAYER 9	0.008
LOCATION OF MAXIMUM HEAD IN LAYER 8 (DISTANCE FROM DRAIN)	0.0 FEET
SNOW WATER	10.70 3894699.2500
MAXIMUM VEG. SOIL WATER (VOL./VOL.)	0.2495
MINIMUM VEG. SOIL WATER (VOL./VOL.)	0.0360
NOTE: Maximum heads are computed using McInroe's equations.	
Reference: Maximum saturated depth over Landfill liner by Bruce M. McInroe, University of Kansas ASCE Journal of Environmental Engineering Vol. 119, No. 2, March 1993, pp. 260-270.	
FINAL WATER STORAGE AT END OF YEAR 100	
LAYER	(INCHES) (VOL./VOL.)
1	2.0464 0.1705
2	0.1019 0.0468
3	0.0000 0.0000
4	0.1800 0.7500
5	175.2000 0.2920
6	0.0020 0.0100
7	0.0000 0.0000
8	0.0020 0.0100

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9	0.0000	0.0000
10	13.3720	0.4270
SNOW WATER	3.651	

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30-YEAR DURATION

DRAINAGE COLLECTED FROM LAYER 4	0.3265	118516.258	0.72
PERC./LEAKAGE THROUGH LAYER 6	0.000000	0.073	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0001		
CHANGE IN WATER STORAGE	-0.116	-53177.798	-0.32
SOIL WATER AT START OF YEAR	191.822	69867864.000	
SOIL WATER AT END OF YEAR	196.166	69050712.000	
SNOW WATER AT START OF YEAR	0.804	291892.700	1.76
SNOW WATER AT END OF YEAR	2.314	839867.375	5.07
ANNUAL WATER BUDGET BALANCE	0.0000	0.490	0.00

ANNUAL TOTALS FOR YEAR 4			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	35.34	11739421.000	100.00
RUNOFF	5.433	1972777.120	16.80
EVAPOTRANSPIRATION	19.079	6830003.000	59.46
DRAINAGE COLLECTED FROM LAYER 2	0.8429	2493965.750	21.16
PERC./LEAKAGE THROUGH LAYER 3	0.191291	70890.594	0.60
AVG. HEAD ON TOP OF LAYER 3	0.0002		
DRAINAGE COLLECTED FROM LAYER 4	0.1933	70890.381	0.60
PERC./LEAKAGE THROUGH LAYER 6	0.000000	0.063	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0001		
CHANGE IN WATER STORAGE	0.619	231776.687	1.97
SOIL WATER AT START OF YEAR	190.769	69056712.000	
SOIL WATER AT END OF YEAR	192.880	6952576.000	
SNOW WATER AT START OF YEAR	2.314	839867.375	7.15
SNOW WATER AT END OF YEAR	1.338	493781.250	4.14
ANNUAL WATER BUDGET BALANCE	0.0000	6.282	0.00

ANNUAL TOTALS FOR YEAR 5			
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	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.14	10404810.000	100.00
RUNOFF	3.065	1330323.230	12.10
EVAPOTRANSPIRATION	23.674	8593752.000	78.55
DRAINAGE COLLECTED FROM LAYER 2	5.6595	2054412.870	18.78
PERC./LEAKAGE THROUGH LAYER 3	0.133775	55820.101	0.51
AVG. HEAD ON TOP OF LAYER 3	0.0002		
DRAINAGE COLLECTED FROM LAYER 4	0.1338	55820.258	0.51
PERC./LEAKAGE THROUGH LAYER 6	0.000000	0.051	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0000		
CHANGE IN WATER STORAGE	-3.012	-1093486.500	-9.99
SOIL WATER AT START OF YEAR	191.880	69652576.000	
SOIL WATER AT END OF YEAR	189.857	68858584.000	
SNOW WATER AT START OF YEAR	1.338	485781.250	4.44
SNOW WATER AT END OF YEAR	0.569	204485.797	1.89
ANNUAL WATER BUDGET BALANCE	0.0000	-2.825	0.00

ANNUAL TOTALS FOR YEAR 6			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	31.88	13024440.000	100.00
RUNOFF	4.459	1633250.500	12.54
EVAPOTRANSPIRATION	22.806	8278609.000	63.56
DRAINAGE COLLECTED FROM LAYER 2	7.7172	2801826.750	21.51
PERC./LEAKAGE THROUGH LAYER 3	0.228489	82940.102	0.64
AVG. HEAD ON TOP OF LAYER 3	0.0003		
DRAINAGE COLLECTED FROM LAYER 4	0.2285	82940.047	0.64
PERC./LEAKAGE THROUGH LAYER 6	0.000000	0.062	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0001		
CHANGE IN WATER STORAGE	0.629	228335.935	1.75
SOIL WATER AT START OF YEAR	189.627	68828284.000	
SOIL WATER AT END OF YEAR	189.654	68916864.000	

ANNUAL TOTALS FOR YEAR 7			
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SNOW WATER AT START OF YEAR	0.569	204485.797	1.89
SNOW WATER AT END OF YEAR	0.802	310540.300	2.74
ANNUAL WATER BUDGET BALANCE	0.0000	-1.463	0.00

ANNUAL TOTALS FOR YEAR 7			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	42.17	15307713.000	100.00
RUNOFF	2.261	82702.312	0.59
EVAPOTRANSPIRATION	78.790	10269233.000	67.09
DRAINAGE COLLECTED FROM LAYER 2	8.1079	3300184.250	21.60
PERC./LEAKAGE THROUGH LAYER 3	0.285992	103816.953	0.68
AVG. HEAD ON TOP OF LAYER 3	0.0003		
DRAINAGE COLLECTED FROM LAYER 4	0.2860	103816.891	0.68
PERC./LEAKAGE THROUGH LAYER 6	0.000000	0.081	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0001		
CHANGE IN WATER STORAGE	2.271	807777.062	5.28
SOIL WATER AT START OF YEAR	189.834	68916864.000	
SOIL WATER AT END OF YEAR	190.806	69262576.000	
SNOW WATER AT START OF YEAR	0.982	350340.700	2.33
SNOW WATER AT END OF YEAR	2.255	81804.697	0.35
ANNUAL WATER BUDGET BALANCE	0.0000	-0.048	0.00

ANNUAL TOTALS FOR YEAR 8			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	40.45	1468346.000	100.00
RUNOFF	3.262	1194259.000	8.07
EVAPOTRANSPIRATION	26.223	951088.000	64.83
DRAINAGE COLLECTED FROM LAYER 2	8.5039	3086916.500	21.02

ANNUAL TOTALS FOR YEAR 9			
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PERC./LEAKAGE THROUGH LAYER 3	0.283386	86334.102	0.50
AVG. HEAD ON TOP OF LAYER 3	0.0003		
DRAINAGE COLLECTED FROM LAYER 4	0.2834	86334.016	0.50
PERC./LEAKAGE THROUGH LAYER 6	0.000000	0.073	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0001		
CHANGE IN WATER STORAGE	2.222	806591.062	5.49
SOIL WATER AT START OF YEAR	190.804	69262576.000	
SOIL WATER AT END OF YEAR	194.331	70341976.000	
SNOW WATER AT START OF YEAR	2.255	81804.697	5.57
SNOW WATER AT END OF YEAR	0.932	345000.187	2.35
ANNUAL WATER BUDGET BALANCE	0.0000	-10.735	0.00

ANNUAL TOTALS FOR YEAR 9			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.33	11735792.000	100.00
RUNOFF	2.517	913644.750	7.79
EVAPOTRANSPIRATION	24.078	8958360.000	76.33
DRAINAGE COLLECTED FROM LAYER 2	9.3429	3391477.000	28.90
PERC./LEAKAGE THROUGH LAYER 3	0.260325	94498.070	0.81
AVG. HEAD ON TOP OF LAYER 3	0.0001		
DRAINAGE COLLECTED FROM LAYER 4	0.2603	94497.977	0.81
PERC./LEAKAGE THROUGH LAYER 6	0.000000	0.075	0.00
AVG. HEAD ON TOP OF LAYER 3	0.0001		
CHANGE IN WATER STORAGE	-4.489	-1621201.120	-13.82
SOIL WATER AT START OF YEAR	194.331	70341976.000	
SOIL WATER AT END OF YEAR	189.975	68966896.000	
SNOW WATER AT START OF YEAR	0.932	345000.187	2.94
SNOW WATER AT END OF YEAR	0.839	304476.133	2.39
ANNUAL WATER BUDGET BALANCE	0.0000	13.125	0.00

ANNUAL TOTALS FOR YEAR 10			
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ANNUAL TOTALS FOR YEAR 10

	INCHES	CU. FEET	PERCENT
PRECIPITATION	21.62	11478085.000	100.00
RUNOFF	1.002	581706.125	5.07
EVAPOTRANSPIRATION	24.803	9003426.000	78.44
DRAINAGE COLLECTED FROM LAYER 2	3.2992	1197622.370	10.43
PERC./LEAKAGE THROUGH LAYER 3	0.097550	33410.762	0.31
AVG. HEAD ON TOP OF LAYER 3	0.0001		
DRAINAGE COLLECTED FROM LAYER 4	0.0976	33410.711	0.31
PERC./LEAKAGE THROUGH LAYER 5	0.000000	0.041	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	1,818	659901.700	5.75
SOIL WATER AT START OF YEAR	189.973	68960896.000	
SOIL WATER AT END OF YEAR	188.879	68563024.000	
SNOW WATER AT START OF YEAR	0.839	304476.331	2.55
SNOW WATER AT END OF YEAR	1.753	1362247.620	11.87
ANNUAL WATER BUDGET BALANCE	0.0000	0.283	0.00

ANNUAL TOTALS FOR YEAR 11

	INCHES	CU. FEET	PERCENT
PRECIPITATION	26.18	9503841.000	100.00
RUNOFF	3.033	737964.437	7.77
EVAPOTRANSPIRATION	19.691	7147924.000	75.21
DRAINAGE COLLECTED FROM LAYER 2	5.3231	1927290.120	20.33
PERC./LEAKAGE THROUGH LAYER 3	0.136371	57490.504	0.60
AVG. HEAD ON TOP OF LAYER 3	0.0002		
DRAINAGE COLLECTED FROM LAYER 4	0.1384	57486.853	0.60
PERC./LEAKAGE THROUGH LAYER 5	0.000000	0.059	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	-1.070	-377336.312	-3.92

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SOIL WATER AT START OF YEAR	189.879	68563024.000	
SOIL WATER AT END OF YEAR	191.129	69379896.000	
SNOW WATER AT START OF YEAR	3.753	1392247.620	14.33
SNOW WATER AT END OF YEAR	0.477	173037.435	1.82
ANNUAL WATER BUDGET BALANCE	0.0000	7.076	0.00

ANNUAL TOTALS FOR YEAR 12

	INCHES	CU. FEET	PERCENT
PRECIPITATION	27.74	10699622.000	100.00
RUNOFF	3.230	1172659.500	11.65
EVAPOTRANSPIRATION	20.890	7582919.500	73.30
DRAINAGE COLLECTED FROM LAYER 2	5.1579	1844932.210	19.31
PERC./LEAKAGE THROUGH LAYER 3	0.149812	54381.910	0.54
AVG. HEAD ON TOP OF LAYER 3	0.0002		
DRAINAGE COLLECTED FROM LAYER 4	0.1498	54381.887	0.54
PERC./LEAKAGE THROUGH LAYER 5	0.000000	0.052	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	-1.888	-685260.125	-6.81
SOIL WATER AT START OF YEAR	191.129	69379896.000	
SOIL WATER AT END OF YEAR	189.010	68610690.000	
SNOW WATER AT START OF YEAR	0.477	173037.435	1.72
SNOW WATER AT END OF YEAR	0.709	256993.094	2.55
ANNUAL WATER BUDGET BALANCE	0.0000	-2.070	0.00

ANNUAL TOTALS FOR YEAR 13

	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.02	10897762.000	100.00
RUNOFF	3.166	1149337.620	10.55

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EVAPOTRANSPIRATION	11.992	7993247.000	73.26
DRAINAGE COLLECTED FROM LAYER 2	4.0594	1084988.620	15.35
PERC./LEAKAGE THROUGH LAYER 3	0.129928	46074.844	0.42
AVG. HEAD ON TOP OF LAYER 3	0.0002		
DRAINAGE COLLECTED FROM LAYER 4	0.1269	46074.816	0.42
PERC./LEAKAGE THROUGH LAYER 5	0.000000	0.035	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	0.063	23391.926	0.22
SOIL WATER AT START OF YEAR	189.010	68610690.000	
SOIL WATER AT END OF YEAR	189.783	68891256.000	
SNOW WATER AT START OF YEAR	0.708	256993.094	2.39
SNOW WATER AT END OF YEAR	0.600	0.600	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	1.409	0.00

ANNUAL TOTALS FOR YEAR 14

	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.63	11844692.000	100.00
RUNOFF	1.856	673743.312	5.69
EVAPOTRANSPIRATION	25.181	8411967.000	71.05
DRAINAGE COLLECTED FROM LAYER 2	6.0291	2108564.000	18.42
PERC./LEAKAGE THROUGH LAYER 3	0.171358	64306.305	0.54
AVG. HEAD ON TOP OF LAYER 3	0.0002		
DRAINAGE COLLECTED FROM LAYER 4	0.1772	64306.246	0.54
PERC./LEAKAGE THROUGH LAYER 5	0.000000	0.062	0.00
AVG. HEAD ON TOP OF LAYER 5	0.0000		
CHANGE IN WATER STORAGE	1.383	502688.687	4.14
SOIL WATER AT START OF YEAR	189.783	68891256.000	
SOIL WATER AT END OF YEAR	191.051	69328000.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.112	40541.383	0.34
ANNUAL WATER BUDGET BALANCE	0.0000	0.227	0.00

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AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 14

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	1.98	2.60	2.08	1.10	1.28	3.00
STD. DEVIATIONS	2.82	2.77	2.97	2.89	3.42	2.50
RUNOFF						
TOTALS	0.92	0.91	1.26	1.39	1.38	1.17
STD. DEVIATIONS	1.11	0.97	1.72	1.42	1.50	1.25
EVAPOTRANSPIRATION						
TOTALS	0.129	0.142	1.132	0.308	0.000	0.000
STD. DEVIATIONS	0.000	0.000	0.000	0.000	0.000	0.000
LATERAL DRAINAGE COLLECTED FROM LAYER 2						
TOTALS	0.6280	0.0388	0.0186	1.2177	2.5491	0.9101
STD. DEVIATIONS	0.2358	0.0013	0.0381	0.0991	0.4177	1.0066
PERCOLATION/LEAKAGE THROUGH LAYER 3						
TOTALS	0.0160	0.0008	0.0020	0.0358	0.0678	0.0258
STD. DEVIATIONS	0.0093	0.0001	0.0023	0.0039	0.0145	0.0311
LATERAL DRAINAGE COLLECTED FROM LAYER 4						
TOTALS	0.0160	0.0008	0.0020	0.0358	0.0678	0.0258
STD. DEVIATIONS	0.0093	0.0001	0.0023	0.0039	0.0145	0.0311
PERCOLATION/LEAKAGE THROUGH LAYER 5						
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

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AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3						
AVERAGES	0.0003	0.0000	0.0000	0.0005	0.0011	0.0004
	0.0001	0.0000	0.0000	0.0000	0.0002	0.0004
STD. DEVIATIONS	0.0004	0.0001	0.0001	0.0003	0.0003	0.0003
	0.0003	0.0000	0.0001	0.0001	0.0002	0.0003

DAILY AVERAGE HEAD ON TOP OF LAYER 5						
AVERAGES	0.0001	0.0000	0.0000	0.0001	0.0002	0.0001
	0.0000	0.0000	0.0000	0.0000	0.0000	0.0001
STD. DEVIATIONS	0.0001	0.0000	0.0000	0.0001	0.0001	0.0001
	0.0001	0.0000	0.0000	0.0000	0.0001	0.0001

AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 14

	INCHES		CU. FEET	PERCENT
PRECIPITATION	34.42 (1.650)		12493425.0	100.00
RUNOFF	3.023 (1.6225)		1313076.75	10.526
EVAPOTRANSPIRATION	23.488 (2.6689)		8526259.00	68.246
LATERAL DRAINAGE COLLECTED FROM LAYER 2	7.2527 (2.39503)		2682572.500	21.07347
PERCOLATION/LEAKAGE THROUGH LAYER 3	0.19934 (0.09661)		74902.602	0.59954
AVERAGE HEAD ON TOP OF LAYER 3	0.000 (0.000)			
LATERAL DRAINAGE COLLECTED FROM LAYER 4	0.70694 (0.06602)		74902.589	0.59954
PERCOLATION/LEAKAGE THROUGH LAYER 5	0.00000 (0.00000)		0.002	0.00000
AVERAGE HEAD ON TOP OF LAYER 5	0.000 (0.000)			
CHANGE IN WATER STORAGE	-0.153 (1.9543)		-53990.96	-0.443

PEAK DAILY VALUES FOR YEARS 1 THROUGH 14

	(INCHES)	(CU. FT.)
PRECIPITATION	7.80	1016400.000
RUNOFF	2.293	831121.5920

DRAINAGE COLLECTED FROM LAYER 2	0.17344	62957.89060
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.003435	1247.01677
AVERAGE HEAD ON TOP OF LAYER 5	0.002	
MAXIMUM HEAD ON TOP OF LAYER 3	0.004	
LOCATION OF MAXIMUM HEAD IN LAYER 2 (DISTANCE FROM DRAIN)	2.9 FEET	
DRAINAGE COLLECTED FROM LAYER 4	0.00344	1247.01611
PERCOLATION/LEAKAGE THROUGH LAYER 6	0.000000	0.00046
AVERAGE HEAD ON TOP OF LAYER 3	0.000	
MAXIMUM HEAD ON TOP OF LAYER 5	0.001	
LOCATION OF MAXIMUM HEAD IN LAYER 4 (DISTANCE FROM DRAIN)	0.0 FEET	
SNOW WATER	5.97	1047561.0000
MAXIMUM VEG. SOIL WATER (VOL/VOL)		0.3147
MINIMUM VEG. SOIL WATER (VOL/VOL)		0.0776

*** Maximum heads are computed using McInroe's equations. ***
 Reference: Maximum saturated depth over Landfill liner
 by Bruce M. McInroe, University of Kansas
 ASCE Journal of Environmental Engineering
 Vol. 119, No. 2, March 1993, pp. 262-270.

FINAL WATER STORAGE AT END OF YEAR 14

LAYER	(INCHES)	(VOL./VOL.)
1	179.0750	0.2628
2	0.0029	0.0146
3	0.0000	0.0000
4	0.0020	0.0100
5	0.0000	0.0000
6	15.9720	0.4270
IMPV WATER	0.112	

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*****
HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
HELP MODEL VERSION 3.07 (1 NOVEMBER 1997)
DEVELOPED BY ENVIRONMENTAL LABORATORY
USAE WATERWAYS EXPERIMENT STATION
FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
*****

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PRECIPITATION DATA FILE: C:\VC\DATA1.D
TEMPERATURE DATA FILE: C:\VC\DATA7.D
SOLAR RADIATION DATA FILE: C:\VC\DATA13.DL3
EVAPOTRANSPIRATION DATA: C:\VC\DATA13.DL3
SOIL AND DESIGN DATA FILE: C:\VC\DATA10.DL0
OUTPUT DATA FILE: C:\VC\OUT\OUT

```

TIME: 18:46 DATE: 9/ 2/2019

TITLE: R Post-Closure

NOTE: INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE SPECIFIED BY THE USER.

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-----
LAYER 1
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0459 VOL/VOL
WILTING POINT = 0.0189 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2673 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.00999978000E-02 CM/SEC
NOTE: SATURATED HYDRAULIC CONDUCTIVITY IS MULTIPLIED BY 3.00
FOR ROOT CHANNELS IN TOP HALF OF EVAPORATIVE ZONE.

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LAYER 2
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 1
THICKNESS = 12.00 INCHES
Page 1

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WILTING POINT = 0.0050 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0185 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.000000000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 50.0 FEET

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LAYER 3
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999998000E-12 CM/SEC
PML PINHOLE DENSITY = 2.00 HOLES/ACRE
PML INSTALLATION DEFECTS = 2.00
PML PLACEMENT QUALITY = 3 - 9000

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LAYER 4
-----
TYPE 3 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 34
THICKNESS = 0.20 INCHES
POROSITY = 0.8900 VOL/VOL
FIELD CAPACITY = 0.0180 VOL/VOL
WILTING POINT = 0.0050 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0400 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 33.000000000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 50.0 FEET

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-----
LAYER 5
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 31
THICKNESS = 0.08 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.19999998000E-12 CM/SEC
PML PINHOLE DENSITY = 2.00 HOLES/ACRE
PML INSTALLATION DEFECTS = 2.00
PML PLACEMENT QUALITY = 3 - 9000

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LAYER 6
-----
TYPE 2 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 16
Page 3

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POROSITY = 0.4170 VOL/VOL
FIELD CAPACITY = 0.0459 VOL/VOL
WILTING POINT = 0.0189 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.00999978000E-02 CM/SEC
SLOPE = 2.00 PERCENT
DRAINAGE LENGTH = 50.0 FEET

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LAYER 3
-----
TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 30
THICKNESS = 0.04 INCHES
POROSITY = 0.0000 VOL/VOL
FIELD CAPACITY = 0.0000 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.39999998000E-12 CM/SEC
PML PINHOLE DENSITY = 2.00 HOLES/ACRE
PML INSTALLATION DEFECTS = 2.00
PML PLACEMENT QUALITY = 3 - 9000

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LAYER 4
-----
TYPE 3 - BARRIER SOIL LINER
MATERIAL TEXTURE NUMBER 17
THICKNESS = 0.24 INCHES
POROSITY = 0.7500 VOL/VOL
FIELD CAPACITY = 0.0400 VOL/VOL
WILTING POINT = 0.0000 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.0000 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.30000000000E-08 CM/SEC

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LAYER 5
-----
TYPE 1 - VERTICAL PERCOLATION LAYER
MATERIAL TEXTURE NUMBER 1B
THICKNESS = 600.00 INCHES
POROSITY = 0.6710 VOL/VOL
FIELD CAPACITY = 0.0740 VOL/VOL
WILTING POINT = 0.0770 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.2928 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-02 CM/SEC

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-----
LAYER 6
-----
TYPE 2 - LATERAL DRAINAGE LAYER
MATERIAL TEXTURE NUMBER 14
THICKNESS = 0.20 INCHES
POROSITY = 0.8500 VOL/VOL
FIELD CAPACITY = 0.0100 VOL/VOL
Page 2

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THICKNESS = 30.00 INCHES
POROSITY = 0.3170 VOL/VOL
FIELD CAPACITY = 0.4280 VOL/VOL
WILTING POINT = 0.3070 VOL/VOL
INITIAL SOIL WATER CONTENT = 0.4270 VOL/VOL
EFFECTIVE SAT. HYD. COND. = 0.10000000000E-06 CM/SEC

```

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT SOIL DATA BASE USING SOIL TEXTURE # 1 WITH A FAIR STAND OF GRASS, A SURFACE SLOPE OF 10.8 AND A SLOPE LENGTH OF 30. FEET.

```

SCS RUNOFF CURVE NUMBER = 13.80
FRACTION OF AREA ALLOWING RUNOFF = 100.0 PERCENT
AREA PROJECTED ON HORIZONTAL PLANE = 100.000 ACRES
EVAPORATIVE ZONE DEPTH = 20.0 INCHES
INITIAL WATER IN EVAPORATIVE ZONE = 3.94 INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE = 8.340 INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE = 0.580 INCHES
INITIAL SNOW WATER = 0.000 INCHES
INITIAL WATER IN LAYER MATERIALS = 105.524 INCHES
TOTAL INITIAL WATER = 109.524 INCHES
TOTAL SUBSURFACE INFLOW = 0.00 INCHES/YEAR

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	ANNUAL TOTALS FOR YEAR 1		
	INCHES	CU. FEET	PERCENT
PRECIPITATION	36.09	1310075.000	100.00
RUNOFF	4.779	1733901.250	19.24
EVAPOTRANSPIRATION	20.160	7318011.500	55.86
DRAINAGE COLLECTED FROM LAYER 2	11.1520	4049161.250	30.50
PERC./LEAKAGE THROUGH LAYER 4	0.000042	92.530	0.00
AVG. HEAD ON TOP OF LAYER 5	0.2728		
DRAINAGE COLLECTED FROM LAYER 6	0.4774	173395.422	1.39
PERC./LEAKAGE THROUGH LAYER 7	0.003281	1231.002	0.02
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0084	1291.077	0.08
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.036	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.489	-174000.185	-0.12
SOIL WATER AT START OF YEAR	101.537	7097928.000	
SOIL WATER AT END OF YEAR	101.048	70805928.000	

SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.000	0.00

ANNUAL TOTALS FOR YEAR 2			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	39.59	1400873.000	100.00
RUNOFF	7.358	2671035.250	19.07
EVAPOTRANSPIRATION	19.419	669715.500	47.73
DRAINAGE COLLECTED FROM LAYER 2	14.3040	5192364.500	37.07
PERC./LEAKAGE THROUGH LAYER 4	0.000077	28.973	0.00
AVG. HEAD ON TOP OF LAYER 3	0.3500		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	10.600	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000047	17.170	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	17.136	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.034	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.490	-540970.312	-3.88
SOIL WATER AT START OF YEAR	195.058	70801928.000	
SOIL WATER AT END OF YEAR	190.763	69973077.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.804	291892.500	2.08
ANNUAL WATER BUDGET BALANCE	0.0000	0.402	0.00

ANNUAL TOTALS FOR YEAR 3			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	45.66	16574580.000	100.00

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RUNOFF	6.850	2486712.000	15.00
EVAPOTRANSPIRATION	22.109	8170741.500	49.30
DRAINAGE COLLECTED FROM LAYER 2	14.4116	5232129.500	31.57
PERC./LEAKAGE THROUGH LAYER 4	0.000077	27.990	0.00
AVG. HEAD ON TOP OF LAYER 3	0.3523		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	10.550	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000048	17.435	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	17.403	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.032	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.997	684964.000	4.13
SOIL WATER AT START OF YEAR	193.768	69973072.000	
SOIL WATER AT END OF YEAR	199.143	70110056.000	
SNOW WATER AT START OF YEAR	0.804	291892.500	1.70
SNOW WATER AT END OF YEAR	2.314	829867.375	5.07
ANNUAL WATER BUDGET BALANCE	0.0000	4.831	0.00

ANNUAL TOTALS FOR YEAR 4			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.34	11739421.000	100.00
RUNOFF	5.981	2170961.000	18.49
EVAPOTRANSPIRATION	16.116	5850067.500	49.83
DRAINAGE COLLECTED FROM LAYER 2	9.9821	3523487.250	30.87
PERC./LEAKAGE THROUGH LAYER 4	0.000050	20.504	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2456		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	6.446	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000051	13.030	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	17.028	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.031	0.00

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AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.262	94889.320	0.81
SOIL WATER AT START OF YEAR	193.141	70110056.000	
SOIL WATER AT END OF YEAR	194.378	70550032.000	
SNOW WATER AT START OF YEAR	2.314	829867.375	7.15
SNOW WATER AT END OF YEAR	1.338	481791.250	4.14
ANNUAL WATER BUDGET BALANCE	0.0000	-4.234	0.00

ANNUAL TOTALS FOR YEAR 5			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.14	10940819.000	100.00
RUNOFF	4.054	1457351.000	13.32
EVAPOTRANSPIRATION	10.601	3747920.000	35.33
DRAINAGE COLLECTED FROM LAYER 2	8.7504	3176283.750	29.02
PERC./LEAKAGE THROUGH LAYER 4	0.000049	17.932	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2344		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	7.099	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000030	10.833	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	10.802	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.031	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-2.326	-840644.875	-7.60
SOIL WATER AT START OF YEAR	194.378	70550032.000	
SOIL WATER AT END OF YEAR	190.821	69997680.000	
SNOW WATER AT START OF YEAR	1.338	481791.250	4.44
SNOW WATER AT END OF YEAR	0.569	206485.797	1.89
ANNUAL WATER BUDGET BALANCE	0.0000	-8.823	0.00

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ANNUAL TOTALS FOR YEAR 6			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	35.88	13024440.000	100.00
RUNOFF	5.110	1871631.120	14.37
EVAPOTRANSPIRATION	19.109	6926499.500	53.26
DRAINAGE COLLECTED FROM LAYER 2	11.0050	3949044.250	30.37
PERC./LEAKAGE THROUGH LAYER 4	0.000082	22.472	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2695		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	9.018	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000037	13.434	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	13.420	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.034	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.610	221467.672	1.70
SOIL WATER AT START OF YEAR	192.821	69997080.000	
SOIL WATER AT END OF YEAR	193.079	70069296.000	
SNOW WATER AT START OF YEAR	0.569	206485.797	1.59
SNOW WATER AT END OF YEAR	0.982	356340.500	2.74
ANNUAL WATER BUDGET BALANCE	0.0000	4.158	0.00

ANNUAL TOTALS FOR YEAR 7			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	42.17	15307713.000	100.00
RUNOFF	2.552	927632.375	6.08
EVAPOTRANSPIRATION	24.759	8983311.000	59.70
DRAINAGE COLLECTED FROM LAYER 2	13.0269	4801360.000	31.37
PERC./LEAKAGE THROUGH LAYER 4	0.000088	24.944	0.00
AVG. HEAD ON TOP OF LAYER 3	0.3242		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	8.599	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000045	16.245	0.00

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AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	36.210	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.093	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.035	59386.875	3.88
SOIL WATER AT START OF YEAR	193.026	7005296.000	
SOIL WATER AT END OF YEAR	193.390	7020066.000	
SNOW WATER AT START OF YEAR	0.952	350340.500	2.33
SNOW WATER AT END OF YEAR	2.255	818404.687	5.35
ANNUAL WATER BUDGET BALANCE	0.0000	-1.953	0.00

ANNUAL TOTALS FOR YEAR 8

	INCHES	CU. FEET	PERCENT
PRECIPITATION	40.45	1469346.000	100.00
RUNOFF	5.703	1344239.500	9.15
EVAPOTRANSPIRATION	23.100	828342.000	57.13
DRAINAGE COLLECTED FROM LAYER 2	15.4420	5603446.000	38.18
PERC./LEAKAGE THROUGH LAYER 4	0.000084	30.316	0.00
AVG. HEAD ON TOP OF LAYER 3	0.3782		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	12.023	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000050	18.291	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0001	16.257	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.093	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.799	-65187.500	-4.44
SOIL WATER AT START OF YEAR	193.390	7020066.000	
SOIL WATER AT END OF YEAR	192.897	7021608.000	
SNOW WATER AT START OF YEAR	2.255	818404.687	5.37
SNOW WATER AT END OF YEAR	0.952	343600.187	2.85
ANNUAL WATER BUDGET BALANCE	0.0000	-0.569	0.00

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ANNUAL TOTALS FOR YEAR 9

	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.33	1173792.000	100.00
RUNOFF	2.378	836225.312	7.98
EVAPOTRANSPIRATION	21.763	7809934.000	67.31
DRAINAGE COLLECTED FROM LAYER 2	7.8120	2833759.000	24.16
PERC./LEAKAGE THROUGH LAYER 4	0.000043	15.184	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1921		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	3.692	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000077	9.892	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	9.819	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.093	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.176	63852.220	0.54
SOIL WATER AT START OF YEAR	192.897	7021608.000	
SOIL WATER AT END OF YEAR	193.184	7016584.000	
SNOW WATER AT START OF YEAR	0.952	343600.187	2.94
SNOW WATER AT END OF YEAR	0.839	304878.531	2.59
ANNUAL WATER BUDGET BALANCE	0.0000	5.944	0.00

ANNUAL TOTALS FOR YEAR 10

	INCHES	CU. FEET	PERCENT
PRECIPITATION	51.62	11476043.000	100.00
RUNOFF	1.962	712141.875	6.20
EVAPOTRANSPIRATION	21.613	7843508.500	68.35
DRAINAGE COLLECTED FROM LAYER 2	7.7661	2693106.000	23.24

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PERC./LEAKAGE THROUGH LAYER 4	0.000034	12.386	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1389		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	4.509	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000021	7.484	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	7.450	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.094	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-2.779	-827299.375	-7.23
SOIL WATER AT START OF YEAR	193.184	7012658.000	
SOIL WATER AT END OF YEAR	192.352	6989612.000	
SNOW WATER AT START OF YEAR	0.839	304878.531	2.85
SNOW WATER AT END OF YEAR	3.753	1362247.620	11.87
ANNUAL WATER BUDGET BALANCE	0.0000	-2.132	0.00

ANNUAL TOTALS FOR YEAR 11

	INCHES	CU. FEET	PERCENT
PRECIPITATION	76.18	9503541.000	100.00
RUNOFF	7.458	892432.500	9.39
EVAPOTRANSPIRATION	13.519	672496.000	7.04
DRAINAGE COLLECTED FROM LAYER 2	9.0572	2924777.000	30.78
PERC./LEAKAGE THROUGH LAYER 4	0.000046	16.651	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1937		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	6.407	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000078	10.245	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	10.211	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.094	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.855	-1036379.750	-10.93
SOIL WATER AT START OF YEAR	192.352	6989612.000	

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SOIL WATER AT END OF YEAR	192.073	7084894.000	
SNOW WATER AT START OF YEAR	3.751	1362247.620	14.33
SNOW WATER AT END OF YEAR	0.477	173037.455	1.82
ANNUAL WATER BUDGET BALANCE	0.0000	4.293	0.00

ANNUAL TOTALS FOR YEAR 12

	INCHES	CU. FEET	PERCENT
PRECIPITATION	27.74	10069622.000	100.00
RUNOFF	3.755	1363050.370	13.54
EVAPOTRANSPIRATION	12.435	6328927.500	62.85
DRAINAGE COLLECTED FROM LAYER 2	7.2183	2624762.250	26.17
PERC./LEAKAGE THROUGH LAYER 4	0.000043	15.445	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1781		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	6.307	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000075	9.138	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	9.105	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.093	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.768	-257114.609	-2.55
SOIL WATER AT START OF YEAR	192.072	7084894.000	
SOIL WATER AT END OF YEAR	192.023	6970780.000	
SNOW WATER AT START OF YEAR	0.477	173037.455	1.72
SNOW WATER AT END OF YEAR	0.788	288983.094	2.93
ANNUAL WATER BUDGET BALANCE	0.0000	1.076	0.00

ANNUAL TOTALS FOR YEAR 13

	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.02	10897262.000	100.00

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RUNOFF	3,388	1302482.000	11.95
EVAPOTRANSPIRATION	10,466	6701861.000	61.51
DRAINAGE COLLECTED FROM LAYER 2	7,7160	2800922.500	25.70
PERC./LEAKAGE THROUGH LAYER 4	0.000044	16.131	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1871		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.398	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000027	9.753	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	9.719	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.034	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.250	90770.271	0.83
SOIL WATER AT START OF YEAR	192,034	6970780.000	
SOIL WATER AT END OF YEAR	192,991	7005540.000	
SNOW WATER AT START OF YEAR	0.708	25683.094	2.36
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	0.790	0.00

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ANNUAL TOTALS FOR YEAR 14			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.63	11844602.000	100.00
RUNOFF	2,182	792193.187	6.69
EVAPOTRANSPIRATION	18,583	6745509.500	56.95
DRAINAGE COLLECTED FROM LAYER 2	11,7337	4252074.000	35.90
PERC./LEAKAGE THROUGH LAYER 4	0.000062	22.162	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2866		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	7.805	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000040	14.557	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	14.527	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	-0.033	0.00

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AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.151	54991.324	0.46
SOIL WATER AT START OF YEAR	192,991	7005540.000	
SOIL WATER AT END OF YEAR	193,030	7006992.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.112	40345.383	0.34
ANNUAL WATER BUDGET BALANCE	0.0000	1.790	0.00

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ANNUAL TOTALS FOR YEAR 15			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.31	11002530.000	100.00
RUNOFF	2,311	818907.887	7.42
EVAPOTRANSPIRATION	18,146	6587124.500	59.87
DRAINAGE COLLECTED FROM LAYER 2	9,7693	3546251.250	32.23
PERC./LEAKAGE THROUGH LAYER 4	0.000053	10.168	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2387		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	6.064	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000034	12.203	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	12.169	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.034	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.083	10233.951	0.27
SOIL WATER AT START OF YEAR	193,030	7006992.000	
SOIL WATER AT END OF YEAR	193,023	70067432.000	
SNOW WATER AT START OF YEAR	0.112	40545.383	0.37
SNOW WATER AT END OF YEAR	0.202	73222.781	0.67
ANNUAL WATER BUDGET BALANCE	0.0000	7.831	0.00

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PERC./LEAKAGE THROUGH LAYER 7	0.000021	7.868	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	7.833	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.035	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-1.421	-513867.125	-1.46
SOIL WATER AT START OF YEAR	193,406	70206528.000	
SOIL WATER AT END OF YEAR	193,007	70061304.000	
SNOW WATER AT START OF YEAR	2,412	875492.625	8.95
SNOW WATER AT END OF YEAR	1,341	486846.062	4.98
ANNUAL WATER BUDGET BALANCE	0.0000	-1.429	0.00

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ANNUAL TOTALS FOR YEAR 16			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	43.34	15732416.000	100.00
RUNOFF	4,955	1791411.000	11.39
EVAPOTRANSPIRATION	22,138	8035990.500	51.08
DRAINAGE COLLECTED FROM LAYER 2	11,6742	4063731.500	25.85
PERC./LEAKAGE THROUGH LAYER 4	0.000074	26.739	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2327		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	10.222	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000046	16.318	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	16.483	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.034	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	3.592	641258.187	5.46
SOIL WATER AT START OF YEAR	193,023	70067432.000	
SOIL WATER AT END OF YEAR	193,406	70206528.000	
SNOW WATER AT START OF YEAR	0.202	73222.781	0.47
SNOW WATER AT END OF YEAR	2,412	875492.625	5.36
ANNUAL WATER BUDGET BALANCE	0.0000	-2.225	0.00

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ANNUAL TOTALS FOR YEAR 17			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	26.94	9779221.000	100.00
RUNOFF	5,436	1973401.250	20.18
EVAPOTRANSPIRATION	17,026	6180803.000	63.20
DRAINAGE COLLECTED FROM LAYER 2	5,9479	2159073.500	22.08
PERC./LEAKAGE THROUGH LAYER 4	0.000038	13.675	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1431		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	6.008	0.00

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PERC./LEAKAGE THROUGH LAYER 7	0.000038	13.815	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	13.784	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.041	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.993	360361.219	2.28
SOIL WATER AT START OF YEAR	193,007	70061304.000	
SOIL WATER AT END OF YEAR	193,749	70330744.000	
SNOW WATER AT START OF YEAR	1,341	486846.062	3.08
SNOW WATER AT END OF YEAR	1,592	577970.500	3.65

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ANNUAL TOTALS FOR YEAR 18			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	43.37	15815913.000	100.00
RUNOFF	7,974	2894512.250	18.30
EVAPOTRANSPIRATION	23,186	8416373.000	53.21
DRAINAGE COLLECTED FROM LAYER 2	11,4177	4144446.000	26.20
PERC./LEAKAGE THROUGH LAYER 4	0.000052	22.516	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2791		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	9.701	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000038	13.815	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	13.784	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.041	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.993	360361.219	2.28
SOIL WATER AT START OF YEAR	193,007	70061304.000	
SOIL WATER AT END OF YEAR	193,749	70330744.000	
SNOW WATER AT START OF YEAR	1,341	486846.062	3.08
SNOW WATER AT END OF YEAR	1,592	577970.500	3.65

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ANNUAL WATER BUDGET BALANCE			
	INCHES	CU. FEET	PERCENT
ANNUAL WATER BUDGET BALANCE	0.0000	-2.351	0.00

ANNUAL TOTALS FOR YEAR 19			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	16.23	13151488.000	100.00
RUNOFF	5.945	2165161.750	16.44
EVAPOTRANSPIRATION	-18.321	6722982.500	51.12
DRAINAGE COLLECTED FROM LAYER 2	-10.6949	3892148.500	29.52
PERC./LEAKAGE THROUGH LAYER 4	0.000039	21.580	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2609		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	8.317	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000036	13.063	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	13.030	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.033	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	3.050	301176.150	2.90
SOIL WATER AT START OF YEAR	199.749	7039744.000	
SOIL WATER AT END OF YEAR	196.628	70286980.000	
SNOW WATER AT START OF YEAR	1.192	-577970.500	4.39
SNOW WATER AT END OF YEAR	1.763	1002920.440	7.63
ANNUAL WATER BUDGET BALANCE	0.0000	-1.978	0.00

ANNUAL TOTALS FOR YEAR 20			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	15.93	13042587.000	100.00
RUNOFF	5.920	2149000.500	16.48
EVAPOTRANSPIRATION	-12.969	827278.500	63.93
DRAINAGE COLLECTED FROM LAYER 2	-10.5748	3838601.500	29.43

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PERC./LEAKAGE THROUGH LAYER 4	0.000058	20.930	0.00
AVG. HEAD ON TOP OF LAYER 7	0.2584		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	7.897	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000036	13.033	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	13.022	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.032	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-3.534	-1282676.120	-9.83
SOIL WATER AT START OF YEAR	193.928	70286950.000	
SOIL WATER AT END OF YEAR	192.857	7007208.000	
SNOW WATER AT START OF YEAR	2.768	1002926.440	7.60
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	-4.133	0.00

ANNUAL TOTALS FOR YEAR 21			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	39.18	34222343.000	100.00
RUNOFF	3.977	1448310.120	10.15
EVAPOTRANSPIRATION	-27.428	8181787.000	57.25
DRAINAGE COLLECTED FROM LAYER 2	-11.5850	4200447.000	29.58
PERC./LEAKAGE THROUGH LAYER 4	0.000062	21.407	0.00
AVG. HEAD ON TOP OF LAYER 5	0.2839		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	8.049	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000040	14.359	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	14.324	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.035	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.188	470027.781	3.03
SOIL WATER AT START OF YEAR	192.857	7007208.000	

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SOIL WATER AT END OF YEAR	192.857	70003996.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	1.190	432345.437	3.04
ANNUAL WATER BUDGET BALANCE	0.0000	-2.675	0.00

ANNUAL TOTALS FOR YEAR 22			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	39.89	14480071.000	100.00
RUNOFF	7.232	2625045.000	18.13
EVAPOTRANSPIRATION	-20.330	7379801.500	50.97
DRAINAGE COLLECTED FROM LAYER 2	9.6039	3480206.750	24.08
PERC./LEAKAGE THROUGH LAYER 4	0.000034	10.537	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2349		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	7.673	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000033	11.864	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	11.831	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.033	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	2.725	988990.000	6.82
SOIL WATER AT START OF YEAR	192.857	70003996.000	
SOIL WATER AT END OF YEAR	191.961	70044768.000	
SNOW WATER AT START OF YEAR	1.190	432345.437	3.04
SNOW WATER AT END OF YEAR	3.807	1382056.250	8.54
ANNUAL WATER BUDGET BALANCE	0.0000	-1.790	0.00

ANNUAL TOTALS FOR YEAR 23			
	INCHES	CU. FEET	PERCENT

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PRECIPITATION	31.85	11488951.000	100.00
RUNOFF	4.556	1653820.370	14.39
EVAPOTRANSPIRATION	-19.938	7237750.500	62.98
DRAINAGE COLLECTED FROM LAYER 2	10.2929	3736310.000	32.32
PERC./LEAKAGE THROUGH LAYER 4	0.000058	21.166	0.00
AVG. HEAD ON TOP OF LAYER 5	0.2513		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	8.827	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000054	12.340	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	12.308	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.032	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-5.132	-1136951.870	-9.90
SOIL WATER AT START OF YEAR	192.961	70044768.000	
SOIL WATER AT END OF YEAR	193.214	70136304.000	
SNOW WATER AT START OF YEAR	3.807	1382056.250	12.03
SNOW WATER AT END OF YEAR	0.473	153581.791	1.34
ANNUAL WATER BUDGET BALANCE	0.0000	0.634	0.00

ANNUAL TOTALS FOR YEAR 24			
	INCHES	CU. FEET	PERCENT
PRECIPITATION	39.30	34265000.000	100.00
RUNOFF	5.108	1853514.500	12.99
EVAPOTRANSPIRATION	-20.322	7378887.000	51.71
DRAINAGE COLLECTED FROM LAYER 2	11.7601	3849734.000	34.00
PERC./LEAKAGE THROUGH LAYER 4	0.000072	26.075	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2251		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	9.886	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000045	16.190	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	16.137	0.00

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PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.000	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	0.512	189734.250	1.30
SOIL WATER AT START OF YEAR	101.213	7013604.000	
SOIL WATER AT END OF YEAR	284.148	70479640.000	
SNOW WATER AT START OF YEAR	0.423	133583.391	1.00
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	3.859	0.00

ANNUAL TOTALS FOR YEAR 25

	INCHES	CU. FEET	PERCENT
PRECIPITATION	30.87	11205809.000	100.00
RUNOFF	6.311	2290773.250	20.44
EVAPOTRANSPIRATION	19.149	5801120.000	52.30
DRAINAGE COLLECTED FROM LAYER 2	0.9743	2531681.500	22.59
PERC./LEAKAGE THROUGH LAYER 4	0.000042	15.362	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1605		
DRAINAGE COLLECTED FROM LAYER 5	0.00000	0.856	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000023	8.303	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.00000	0.473	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.082	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.439	521216.406	4.66
SOIL WATER AT START OF YEAR	194.148	70479640.000	
SOIL WATER AT END OF YEAR	193.077	70086792.000	
SNOW WATER AT START OF YEAR	0.0000	0.000	0.00
SNOW WATER AT END OF YEAR	2.310	921003.133	8.18
ANNUAL WATER BUDGET BALANCE	0.0000	7.727	0.00

ANNUAL TOTALS FOR YEAR 26

	INCHES	CU. FEET	PERCENT
PRECIPITATION	38.33	13986391.000	100.00
RUNOFF	8.609	1910739.870	9.37
EVAPOTRANSPIRATION	21.375	7759058.000	55.48
DRAINAGE COLLECTED FROM LAYER 2	14.2441	3206906.000	37.23
PERC./LEAKAGE THROUGH LAYER 4	0.000075	27.310	0.00
AVG. HEAD ON TOP OF LAYER 3	0.3494		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	10.010	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000048	17.309	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	37.277	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.032	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.788	-289829.373	-2.07
SOIL WATER AT START OF YEAR	169.077	7006792.000	
SOIL WATER AT END OF YEAR	193.700	70313224.000	
SNOW WATER AT START OF YEAR	2.310	911061.125	8.51
SNOW WATER AT END OF YEAR	1.099	394800.844	2.82
ANNUAL WATER BUDGET BALANCE	0.0000	-1.918	0.00

ANNUAL TOTALS FOR YEAR 27

	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.55	12178651.000	100.00
RUNOFF	3.247	1178693.500	9.68
EVAPOTRANSPIRATION	17.895	6495875.000	53.34
DRAINAGE COLLECTED FROM LAYER 2	14.7917	3309370.500	44.09
PERC./LEAKAGE THROUGH LAYER 4	0.000079	28.879	0.00
AVG. HEAD ON TOP OF LAYER 3	0.3620		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	10.802	0.00

PERC./LEAKAGE THROUGH LAYER 7	0.000049	17.877	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	17.844	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.033	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-3.284	-985261.750	-7.10
SOIL WATER AT START OF YEAR	193.700	70313234.000	
SOIL WATER AT END OF YEAR	192.404	69847768.000	
SNOW WATER AT START OF YEAR	1.088	394800.844	3.24
SNOW WATER AT END OF YEAR	0.000	0.000	0.00
ANNUAL WATER BUDGET BALANCE	0.0000	3.616	0.00

ANNUAL TOTALS FOR YEAR 28

	INCHES	CU. FEET	PERCENT
PRECIPITATION	34.19	12410972.000	100.00
RUNOFF	3.314	1201028.120	9.69
EVAPOTRANSPIRATION	10.349	7386711.000	59.52
DRAINAGE COLLECTED FROM LAYER 2	9.1701	3310587.750	26.67
PERC./LEAKAGE THROUGH LAYER 4	0.000050	18.032	0.00
AVG. HEAD ON TOP OF LAYER 3	0.2239		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	6.580	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000032	11.471	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	11.838	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.034	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.407	510620.562	4.11
SOIL WATER AT START OF YEAR	192.404	69842768.000	
SOIL WATER AT END OF YEAR	192.128	70103960.000	
SNOW WATER AT START OF YEAR	0.000	0.000	0.00
SNOW WATER AT END OF YEAR	0.683	246024.953	2.00

ANNUAL WATER BUDGET BALANCE	0.0000	7.045	0.00
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ANNUAL TOTALS FOR YEAR 29

	INCHES	CU. FEET	PERCENT
PRECIPITATION	32.04	11630320.000	100.00
RUNOFF	1.858	674348.437	5.80
EVAPOTRANSPIRATION	20.776	7523051.000	64.69
DRAINAGE COLLECTED FROM LAYER 2	7.6310	2770051.330	23.82
PERC./LEAKAGE THROUGH LAYER 4	0.000040	14.578	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1843		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	4.868	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000027	9.910	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	9.876	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.034	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	1.825	662456.062	5.70
SOIL WATER AT START OF YEAR	193.128	70103960.000	
SOIL WATER AT END OF YEAR	192.857	70007048.000	
SNOW WATER AT START OF YEAR	0.683	248024.953	2.13
SNOW WATER AT END OF YEAR	2.779	1008797.100	8.67
ANNUAL WATER BUDGET BALANCE	0.0000	-0.795	0.00

ANNUAL TOTALS FOR YEAR 30

	INCHES	CU. FEET	PERCENT
PRECIPITATION	28.48	10388241.000	100.00
RUNOFF	3.206	1106406.120	11.57
EVAPOTRANSPIRATION	17.390	6212486.000	61.06

DRAINAGE COLLECTED FROM LAYER 2	8,1239	2048973.750	28.32
PERC./LEAKAGE THROUGH LAYER 4	0.000046	38.593	0.00
AVG. HEAD ON TOP OF LAYER 3	0.1978		
DRAINAGE COLLECTED FROM LAYER 6	0.0000	6.847	0.00
PERC./LEAKAGE THROUGH LAYER 7	0.000028	10.131	0.00
AVG. HEAD ON TOP OF LAYER 7	0.0000		
DRAINAGE COLLECTED FROM LAYER 8	0.0000	10.118	0.00
PERC./LEAKAGE THROUGH LAYER 10	0.000000	0.033	0.00
AVG. HEAD ON TOP OF LAYER 9	0.0000		
CHANGE IN WATER STORAGE	-0.330	-119641.109	-3.10
MOIST WATER AT START OF YEAR	192,857	7090708.000	
SOIL WATER AT END OF YEAR	195,092	7090729.600	
SNOW WATER AT START OF YEAR	2.779	1008797.190	9.79
SNOW WATER AT END OF YEAR	0.243	88269.394	0.85
ANNUAL WATER BUDGET BALANCE	0.0000	-0.842	0.00

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AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 30

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	2.05	2.59	3.23	2.98	3.28	2.98
STD. DEVIATIONS	0.84	1.09	1.32	1.19	1.22	1.19
ROHOFF	1.03	0.99	2.05	1.96	1.35	1.18
TOTALS	0.114	0.468	3.757	0.451	0.080	0.000
STD. DEVIATIONS	0.022	0.039	1.856	0.032	0.000	0.000
EVAPOTRANSPIRATION	0.568	0.508	0.593	2.131	2.700	2.457
STD. DEVIATIONS	2.798	2.587	2.943	1.472	1.078	0.348
LATERAL DRAINAGE COLLECTED FROM LAYER 2	0.006	0.081	0.283	0.001	0.751	0.340
STD. DEVIATIONS	0.008	0.021	0.726	0.001	0.183	0.117
TOTALS	0.0132	0.0000	0.8628	3.9262	0.5470	0.4404

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STD. DEVIATIONS	0.1757	0.1131	0.9005	1.1667	1.4379	0.8039
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.0754	0.0000	1.4215	1.7852	0.7144	0.5145
TOTALS	0.1632	0.1359	1.0685	2.1595	1.2192	0.5471
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
TOTALS	0.0159	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
TOTALS	0.0001	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.0001	0.0000	0.0000	0.0000	0.0000	0.0000
TOTALS	0.0001	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
TOTALS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

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AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)

DAILY AVERAGE HEAD ON TOP OF LAYER 3

AVERAGES	0.0028	0.0000	0.2470	1.2040	0.1372	0.1468
STD. DEVIATIONS	0.0001	0.0000	0.2678	0.3153	0.4270	0.2320

DAILY AVERAGE HEAD ON TOP OF LAYER 7

AVERAGES	0.0102	0.0000	0.4659	0.5263	0.2093	0.1126
STD. DEVIATIONS	0.0481	0.0190	0.2147	0.2132	0.2620	0.1172

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DAILY AVERAGE HEAD ON TOP OF LAYER 9

AVERAGES	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
STD. DEVIATIONS	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

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AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS 1 THROUGH 30

	INCHES	CU. FEET	PERCENT
PRECIPITATION	24.88 (5.2722)	12654865.0	100.00
ROHOFF	4.399 (1.7307)	1596782.25	12.618
EVAPOTRANSPIRATION	19.971 (2.7614)	7249296.00	57.296
LATERAL DRAINAGE COLLECTED FROM LAYER 2	10.48357 (2.78438)	3805536.750	30.07720
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.00006 (0.00003)	20.921	0.00017
AVERAGE HEAD ON TOP OF LAYER 3	0.196 (0.059)		
LATERAL DRAINAGE COLLECTED FROM LAYER 6	0.01594 (0.00739)	3786.589	0.04373
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.00015 (0.00001)	53.484	0.00042
AVERAGE HEAD ON TOP OF LAYER 7	0.000 (0.000)		
LATERAL DRAINAGE COLLECTED FROM LAYER 8	0.00015 (0.00001)	53.430	0.00042
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.00000 (0.00000)	0.033	0.00000
AVERAGE HEAD ON TOP OF LAYER 9	0.000 (0.000)		
CHANGE IN WATER STORAGE	-0.008 (1.7403)	-3791.01	-0.027

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PEAK DAILY VALUES FOR YEARS 1 THROUGH 30

	(INCHES)	(CU. FT.)
PRECIPITATION	3.20	1351600.000
ROHOFF	2.416	876998.0120
DRAINAGE COLLECTED FROM LAYER 2	1.78441	359889.56200
PERCOLATION/LEAKAGE THROUGH LAYER 4	0.000009	3.42337

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AVERAGE HEAD ON TOP OF LAYER 3

AVERAGE HEAD ON TOP OF LAYER 3	10.434	
MAXIMUM HEAD ON TOP OF LAYER 3 (DISTANCE FROM DRAIN)	15.432	
(LOCATION OF MAXIMUM HEAD IN LAYER 3)	15.4 FEET	
DRAINAGE COLLECTED FROM LAYER 6	0.13115	49058.23360
PERCOLATION/LEAKAGE THROUGH LAYER 7	0.000692	251.13902
AVERAGE HEAD ON TOP OF LAYER 7	0.009	
MAXIMUM HEAD ON TOP OF LAYER 7	0.004	
(LOCATION OF MAXIMUM HEAD IN LAYER 8)	0.0 FEET	
(DISTANCE FROM DRAIN)		
DRAINAGE COLLECTED FROM LAYER 8	0.00069	251.13890
PERCOLATION/LEAKAGE THROUGH LAYER 10	0.000000	0.00013
AVERAGE HEAD ON TOP OF LAYER 9	0.000	
MAXIMUM HEAD ON TOP OF LAYER 9	0.001	
(LOCATION OF MAXIMUM HEAD IN LAYER 8)	0.0 FEET	
(DISTANCE FROM DRAIN)		
SNOW WATER	5.59	2193710.2500
MAXIMUM VEG. SOIL WATER (VOL/VOL)	0.3376	
MINIMUM VEG. SOIL WATER (VOL/VOL)	0.0280	

*** Maximum heads are computed using McEnroe's equations. ***

Reference: Maximum saturated depth over Landfill liner by Bruce M. McEnroe, University of Kansas ASCE Journal of Environmental Engineering Vol. 119, No. 3, March 1993, pp. 262-270.

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FINAL WATER STORAGE AT END OF YEAR 30

LAYER	(INCHES)	(VOL/VOL)
1	3.2267	0.7888
2	1.0779	0.0898
3	0.0000	0.0000
4	0.1800	0.7500
5	175.2000	0.2920
6	0.0020	0.0100
7	0.0000	0.0000

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8	0.0020	0.0100
9	0.0000	0.0000
10	15.3720	0.4270
SNOW WATER	0.247	

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